Pink Elephant Proposed Design and Location

Certificate of Approval 7.26.2022



THE START

IN 1951 THE FIRST FULLY AUTOMATIC CAR WAS OPENED IN WASHINGTON STATE ON 4TH AVENUE AND LANDER STREET BY DEAN, ARCHIE, AND ELDON ANDERSON. "ELEPHANT CAR WASH" WAS A FIRST OF ITS KIND AND HOME BASED RIGHT HERE IN SEATTLE WHEN DEAN AND ELDON INVENTED THE HANDS FREE SYSTEM OF ROTATING PLASTIC BRISTLES THAT MADE A RELIABLE AND SAFE CLEANING PROCESS.

THE ICON

IN 1956 AFTER THE SUCCESS OF THE 4TH AVENUE AND LANDER STREET LOCATION, THE ANDERSON BROTHERS EXPANDED AND OPENED A NEW SPOT ON BATTERY STREET AND DENNY WAY IN DOWNTOWN SEATTLE. THE POPULARITY OF THIS LOCATION GREW AND ALONG WITH IT THE ICONIC STATUS OF THE ROTATING ELEPHANT SIGN. RIDING THE WAVES OF THIS SUCCESS, THE BOTHERS OPENED A THIRD LOCATION IN TACOMA IN 1963.

PASSING THE TORCH

IN 1982 THE ANDERSON BROTHERS SOLD THE THREE ELEPHANT CAR WASH LOCATIONS AND NAME TO BOB HANEY. BOB TRANSITIONED FROM THE PLASTIC BRISTLES TO A MORE HIGH-POWERED WATER WASH AND A CLOTH WRAPS. HE THEN WENT TO OPEN EIGHT MORE LOCATIONS THROUGHOUT THE PUGET SOUND.







PINK ELEPHANT

THE PINK ELEPHANT IS A REFURBISHED SCULPTURAL MONUMENT CELEBRATING THE ORIGINAL SIGN SITED AT THE INTERSECTION OF DENNY WAY AND DEXTER AVENUE IN PROXIMITY TO HIGHWAY 99. IT COMBINES AN ORIGINAL SIGN SHELL WITH NEW GLASS, ELECTRICAL COMPONENTS, INTERNAL STRUCTURE, AND HARDWARE WHILE INCORPORATING A DECORATIVE BASE REMINISCENT OF THE FIRST PINK ELEPHANT CAR WASH SIGN DESIGNED BY ARTIST BEATRICE HAVERFIELD.

THE SIGNAGE FOR THE PINK ELEPHANT CAR WASH CONSISTED OF NEON AND ARGON FILLED GLASS TUBING, PHOSPHORUS POWDER, GLASS, ALUMINUM, COPPER, PAINTED ENAMEL OVER SHEET METAL, SEVEN ELECTROMAGNETIC TRANSFORMERS, ANIMATOR, GTO-15 WIRING, CARBON STEEL, STAINLESS STEEL, AND MISC HARDWARE. MEASURING 122" X 24" X 110", MADE IN SEATTLE WA, 1956.

RESTORATION IS BEING DONE BY SEATTLE BASED LOCAL EXPERTS AT WESTERN NEON.

ARTIST: BEATRICE HAVERFIELD "QUEEN OF NEON"

BEATRICE HAVERFIELD (1913-1996) WAS AN UNPARALLELED ARTIST, DESIGNER, AND VISIONARY WHO TRANSFORMED THE SEATTLE URBAN LANDSCAPE. WHILE HER IMPACT CAN BE MARKED AND REMEMBERED, HER LEGACY IS IMMEASURABLE. BEATRICE WAS CREATIVE FROM AN EARLY AGE AND HONED HER ARTISTIC TALENTS WHILE SOAKING UP THE RAPIDLY CHANGING SEATTLE SKYLINE. FROM HER FIRST COMPANY WITH HIGH SCHOOL SWEETHEART ELDEN FISLER TO HER TIME WITH BURGEONING COMPANY CAMPBELL NEON, BEATRICE WAS A UNIQUE AND INSTRUMENTAL FORCE.

IN HER TIME, BEATRICE CREATED THE FAMOUS "IVAR'S" SIGN ON PIER 54, THE FUTURIST LOOK OF BLUE-COLLAR MAINSTAY "CHUBBY & TUBBY'S". AS WELL AS "THE DOG HOUSE", "DICK'S DRIVE-IN", AND LATER IN HER CAREER THE MULTI-COLORED, SHOW-STOPPING LANDMARK SIGN FOR CINERAMA MOVIE THEATER. IN 1956, BEATRICE WAS COMMISSIONED TO DESIGN A SIGN FOR ONE OF THE COUNTRY'S FIRST SEMI-AUTOMATED CAR WASH, "THE PINK ELEPHANT."



AN ORIGINAL SKETCH OF THE ELEPHANT CAR WASH SIGN (PHOTO AND SKETCH COURTESY KATHLEEN WOLLF)

PORTRAIT OF BEATRICE HAVERFIELD AS A YOUNG WOMAN



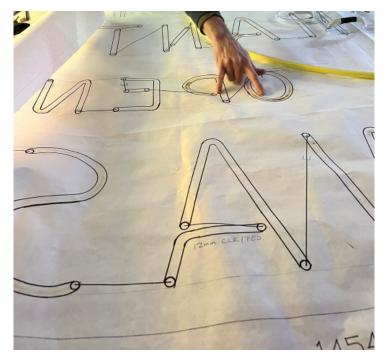
RESTORATION - PINK ELEPHANT AT WESTERN NEON

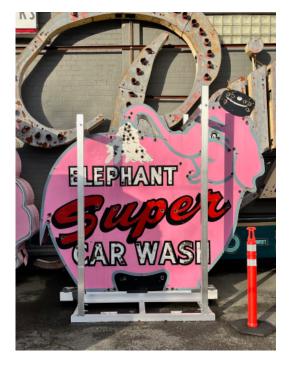




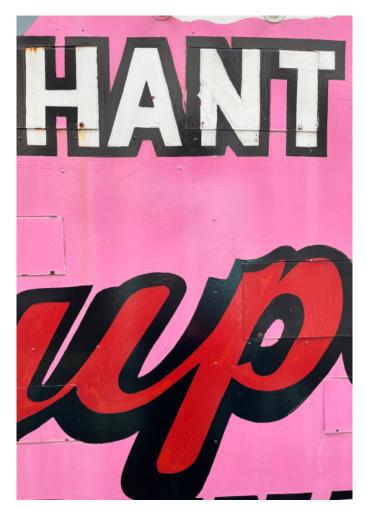






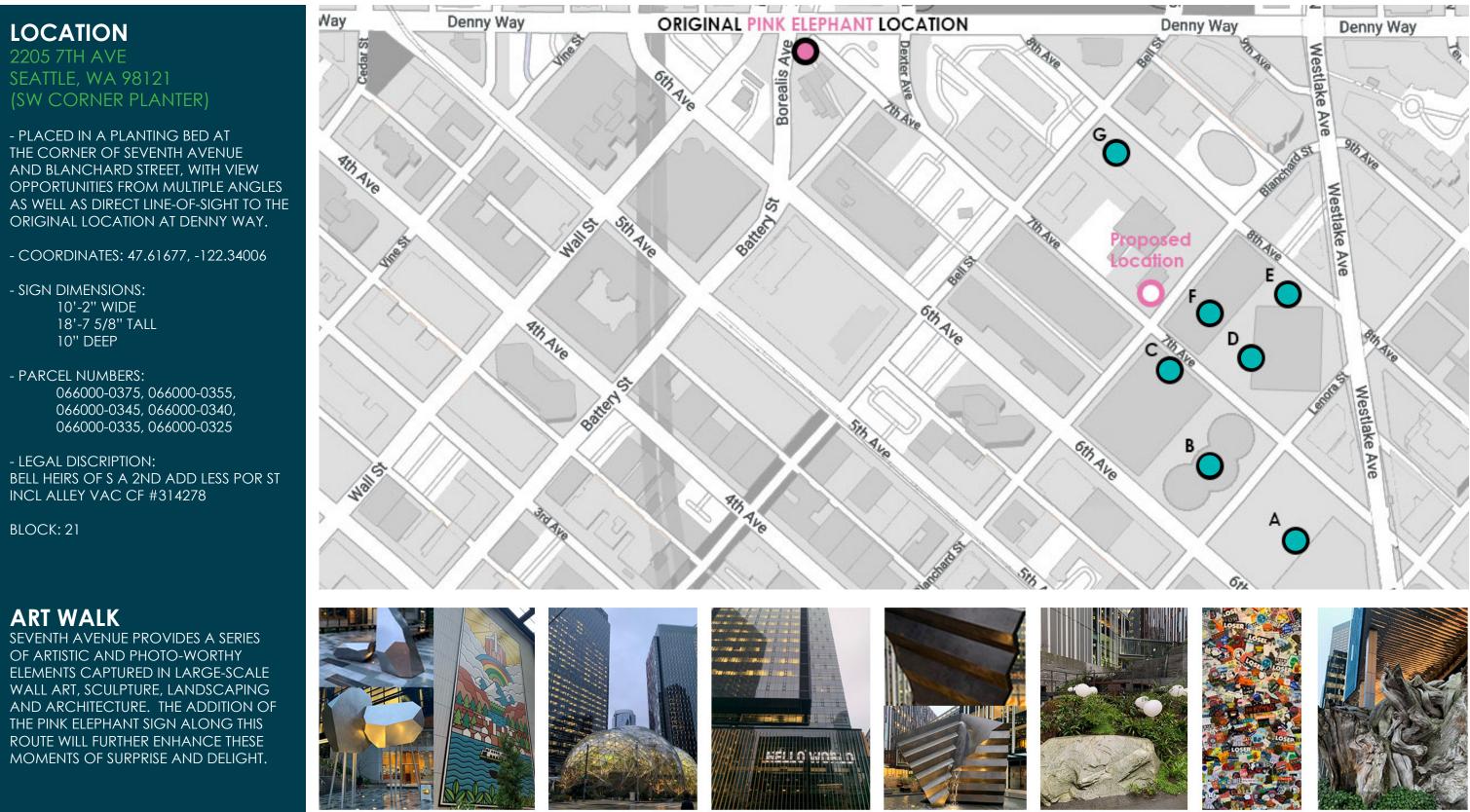








SITE PLAN



C. Hello World

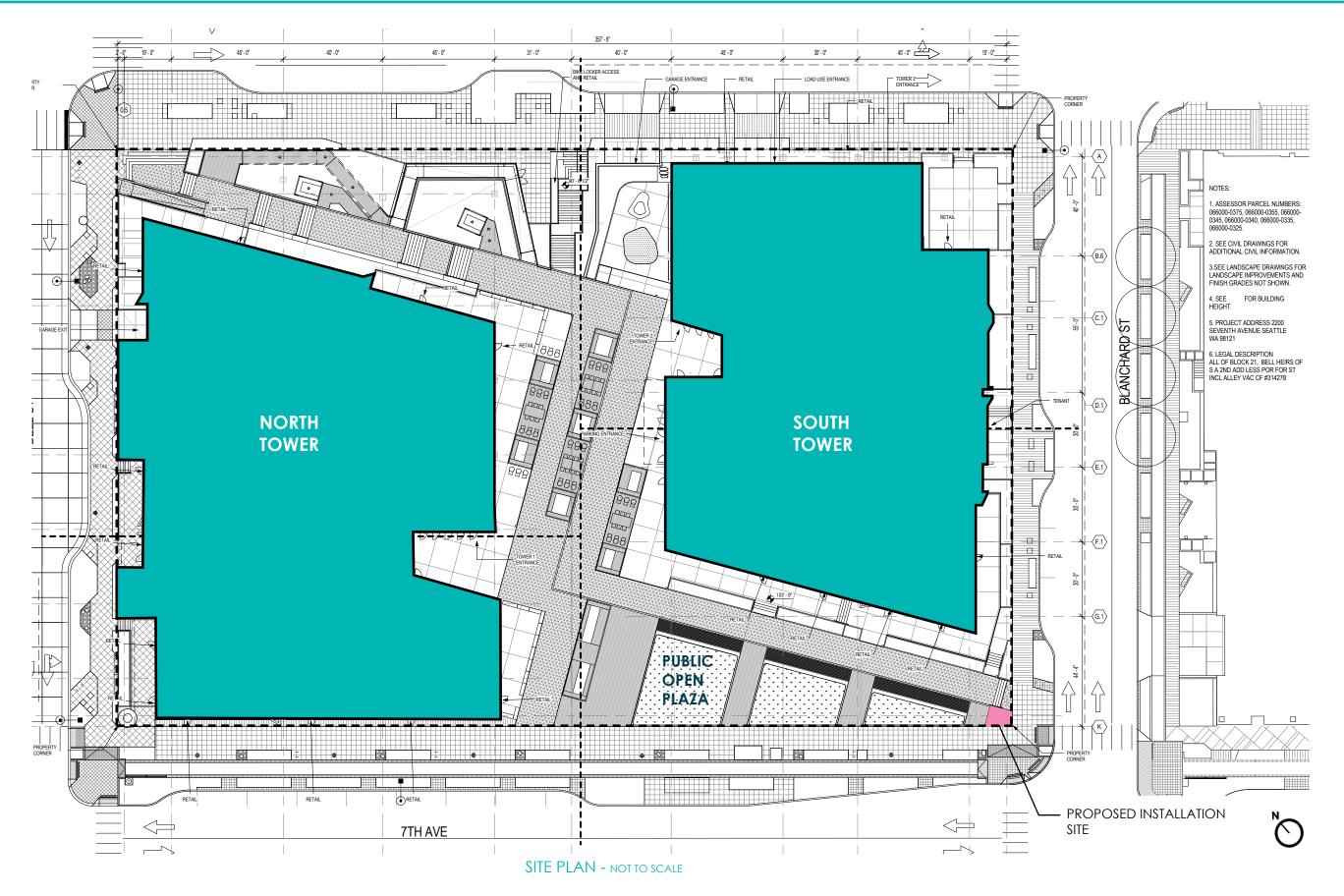
A. Petros Sculpture One Sky Above Us Mural B. The Spheres

D. Tonbi Fountain Sculpture

E. Urban Plaza and Hillclimb F. Sub Pop Wall

G. Urban Native Forest

PROPERTY PLAN - 2200 SEVENTH AVE, SEATTLE WA 98121

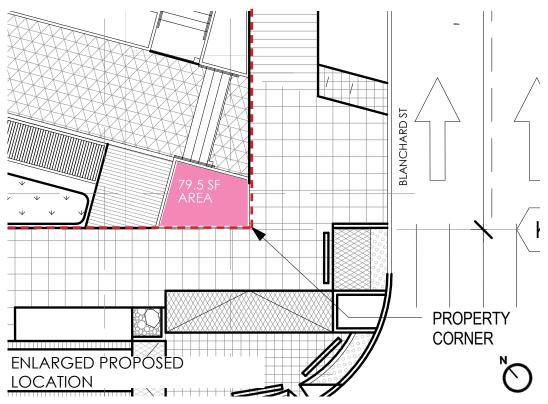


RENDERINGS - PINK ELEPHANT CAR WASH

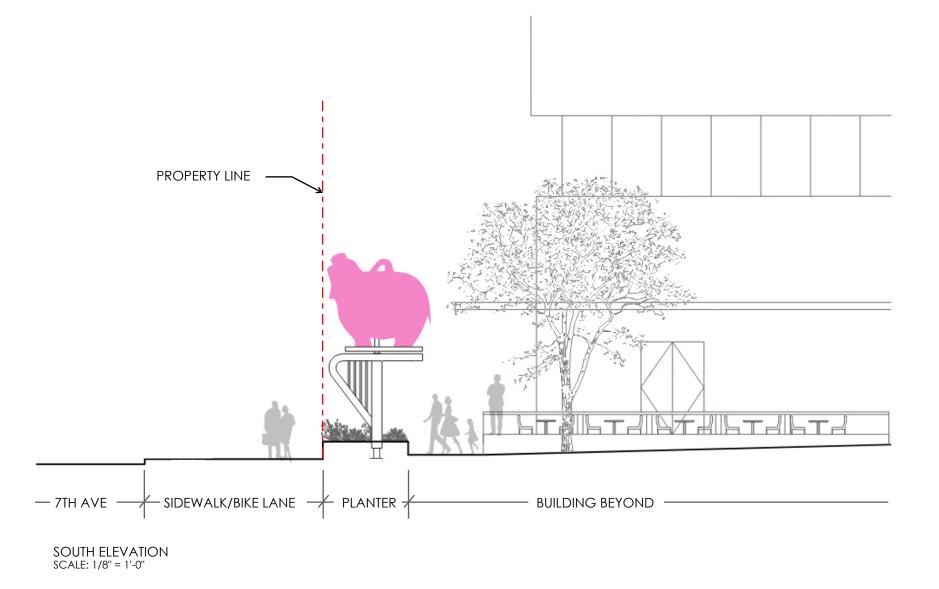




NON-LIGHTED SIDE FACING SOUTH

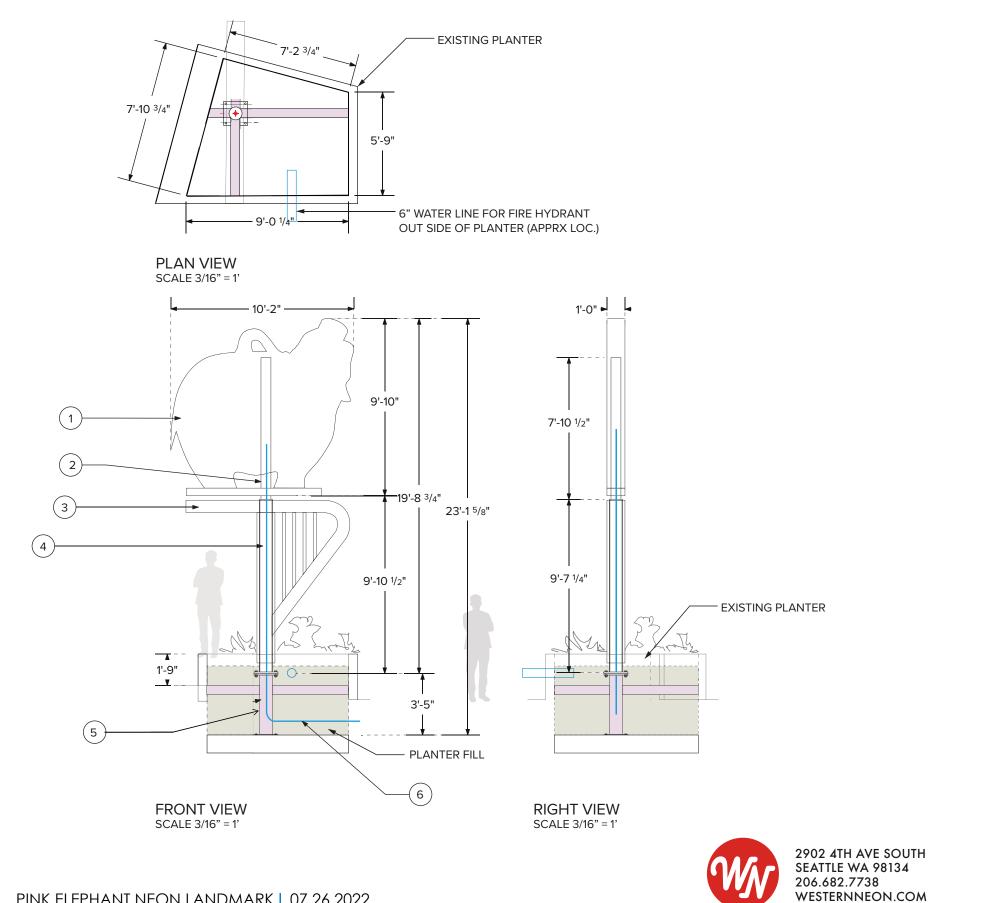


NEON LIGHTED SIDE FACING NORTH

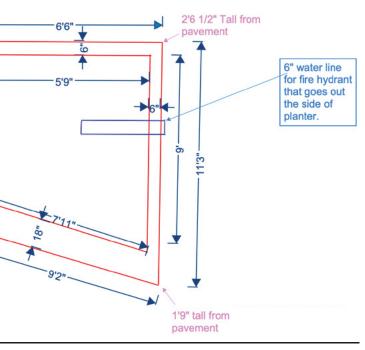




DETAILS - RESTORATION BY WESTERN NEON



- (1)REFUBISHED PINK ELEPHANT NEON SIGN CABINET. EXPOSED NEON TUBING MOUNTED TO CABINET. ALL TRANSFORMERS AND SECONDARY WIRING ARE HOUSED WITHIN SIGN CABINET. NOTE: NEON TUBING ON ONE SIDE OF SIGN ONLY.
- (2) 6" STEEL POLE INTO SIGN CABINET CONNECTS TO EXISTING STRUCTURE WITH MECHANICAL FASTENERS.
- (3) NEWLY FABRICATED ALUMINUM DECORATIVE POLE COVER, DESIGN TO EMULATE THE LOOK OF THE LARGE PINK ELEPHANT NEON SIGN POLE COVER. PAINT SATIN WHITE.
- (4) 8" STEEL POLE. WITH FABRICATED ALUMINUM POLE COVER. PAINT SATIN WHITE.
- (5) BASE PIPE, HSS BRACKET, MOUNTING PLATE (BY OTHERS). SEE ENGINEER STAMPED DRAWINGS.
- QTY (2) 120V/20A DEDICATED SIGN CIRCUITS IN CONDUIT. (4FT STUBOUT FROM FOOTING) PROVIDED BY OTHERS. (6)



PLANTER MEASUREMENTS

3' Tall from

9'2' 2

*

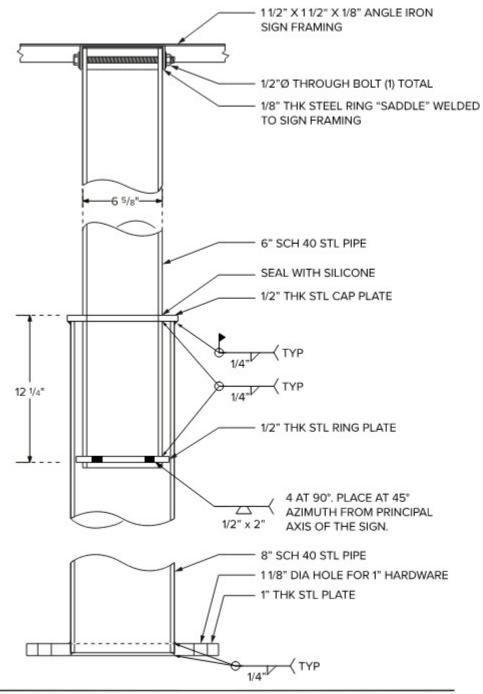
1'11" tall from pavement

SCALE NTS

¥

4

pavement



POLE DETAIL - SECTION SCALE 1 1/2" = 1'

I, Dane Jorgensen, have worked with Western Neon in the coordination, development, and review of the sign structure and connections depicted on this drawing sheet and attest to the integrity of the drawn details.

WASHI OF TRAL EN ONAL

8" SIGN POLE/PIPE -

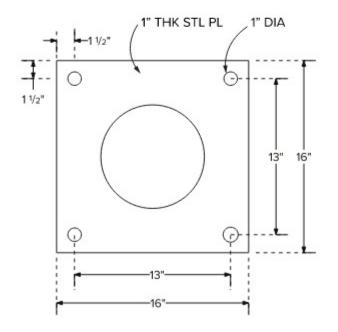
HOLE IN PLATE FOR ELECTRICAL.

06.02.2021

1"Ø HD GALVANIZED BOLT, WASHER,NUT (TYP). (4) TOTAL

MATCH PLATE BY OTHERS. SEE ATTACHED ENG DRAWING SHEET S101 AND ATTACHED ENG STAMPED CALCULATIONS

PIPE STRUCTURE, HSS BRACING AND MOUNTING PLATE BY OTHERS. SEE ATTACHED ENG DRAWING SHEET S101 AND ATTACHED ENG STAMPED CALCULATIONS

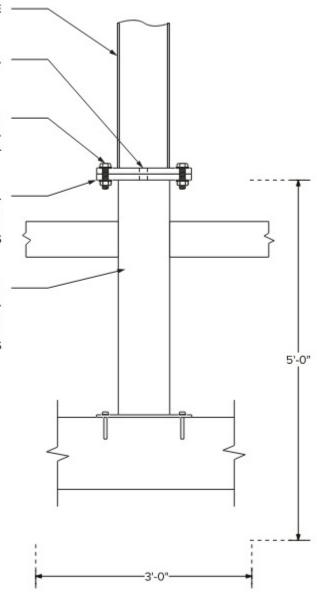


MOUNT PLATE DETAIL - PLAN SCALE 11/2" = 1'

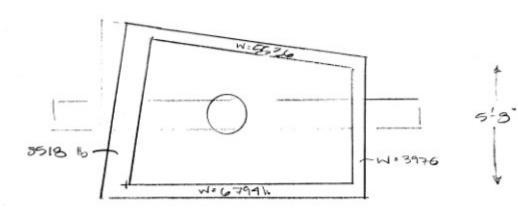
BASE CONNECTION DETAIL SCALE 11/2" = 1'



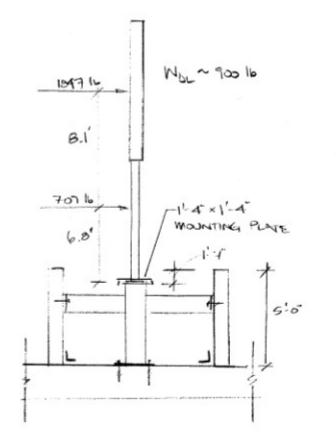
2902 4TH AVE SOUTH SEATTLE WA 98134 206.682.7738 WESTERNNEON.COM







SIGN IS FIXED ORIENTATION; NO ROTATION



Vu= 2556 13 Mu= =23-92 A.16

CHICK TAC

CHECK VORTIONS WEDERIT!

S" of STD RIRE Va-INT . 0.4 12

CHELK PLATE & BILT

1 \$ POLT IN TUREINS - 52 Kip Va

2.165% < 5%

the = 22, 342 Alb/ = 29, 354 /5 = 20 6.2

4 in 3 .)(-1) - 129.6 kin 25 = 75 kin < 129.6 V ...

SEE ATTACHED \$101

PINK ELEPHANT NEON LANDMARK | 07.26.2022

STRUCTURAL GENERAL NOTES EXISTING STRUCTURAL DESIGN BY COUGHLIN PORTER LUNDEEN; DATED 10/26/2017. A. GOVERNING CODES 1) SEATTLE BUILDING CODE (SBC), 2018 EDITION. 2) MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES, ASCE/SEI 7-16. 3) BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, ACI 318-14. 4) SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AISC 360-16. **B. DESIGN LOADS AND CRITERIA** 1) GRAVITY LOADS: a)ROOF LOADS: 1. N/A b)FLOOR LOADS: 1. EXISTING BIO RETENTION PLANTER a. SUPERIMPOSED DEAD LOAD: 515 psf b. LIVE LOAD: 28 psf 2)SNOW LOADS: a) FLAT ROOF SNOW LOAD: 25 psf UNIFORM 3) WIND CRITERIA: a)3-SEC PEAK GUST WIND SPEED = 97 mph b)RISK CATEGORY = II c) lw = 1.00 d)EXPOSURE = C 4) SEISMIC CRITERIA: a)SS = 1.35 g / S1 = 0.52 g MAPPED MCER VALUES b)RISK CATEGORY = II c) PROJECT SITE CLASS = C d) le = 1.00 e) SDS = 1.08 g / SD1 = 0.513 g DESIGN RESPONSE COEFFICIENT f) SEISMIC DESIGN CATEGORY = D g) ANALYSIS PROCEDURE: CHAPTER 13, SEISMIC DESIGN REQUIREMENTS FOR NONSTRUCTURAL COMPONENTS h)ARCHITECTURAL COMPONENT: SIGNS AND BILLBOARDS i) COMPONENT FACTOR: Ap = 2.5; Rp= 3; Ω = 2 C. MATERIALS SECTION 1) ANCHOR RODS: ASTM F1554 GRADE 36 W/ ASTM A563 GRADE DH HEAVY HEX NUTS 2) HYDRAULIC CEMENT GROUT: ASTM C1107, NON-METALLIC, NON-SHRINK, 3 DAY fc = 5000 psi 3) STRUCTURAL STEEL: a)W & WT SHAPES: ASTM A992, Fy = 50 ksi b)HP SHAPES: ASTM A572, Fy = 50 ksi c)HSS — RECTANGULAR: ASTM A500 GRADE B, Fy = 46 ksi d)HSS — ROUND: ASTM A500 GRADE B, Fy = 42 ksi e)STEEL PIPE: ASTM A53 GRADE B, Fy = 35 ksi f) CHANNEL & ANGLE SHAPES: ASTM A36, Fy = 36 ksi g)PLATES AND BARS: ASTM A36, Fy = 36 ksi 4) HIGH-STRENGTH BOLTS: ASTM A325-N TYPE 1 PLAIN; STEEL TO STEEL CONNECTIONS 5)NUTS: ASTM A563 GRADE DH PLAIN; STEEL TO STEEL CONNECTIONS 6) WASHERS: ASTM F436 TYPE 1 PLAIN; STEEL TO STEEL CONNECTIONS 7)BOLTS: ASTM A307 GRADE A; WOOD OR WOOD TO STEEL CONNECTIONS OR ERECTION ONLY 8)WELD FILLER METAL: FEXX = 70 ksi TENSILE STRENGTH 9)SCREW ANCHORS: a) CONCRETE: ASTM B633, CLASS SC1, TYPE III SUCH AS HILTI KWIK HUS-EZ ICC-ES REPORT ESR-3027 OR APPROVED EQUAL D. FOUNDATIONS 1) SIGN ANCHORAGE SHALL BE ATTACHED TO EXISTING LANDSCAPE PLANTER BOX 2)@ AT CONCRETE AND EXISTING BUILDING SLAB. 2) PLANTER BOX SHALL REMAIN A SINGLE, MONOLITHIC BOX ELEMENT. E. STRUCTURAL STEEL 1) STRUCTURAL STEEL DETAILING, FABRICATION AND ERECTION SHALL CONFORM WITH THE LATEST EDITION OF AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" 2) ALL STEEL TO STEEL BOLTED CONNECTIONS SHALL CONFORM TO THE LATEST EDITION OF THE RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" AS ENDORSED BY AISC. 3) ALL STEEL TO STEEL SHOP AND FIELD WELDED CONNECTIONS SHALL CONFORM TO THE LATEST EDITION OF THE AWS D1.1 "STRUCTURAL WELDING CODE — STEEL 4) ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER. ALL WELDERS SHALL POSSESS EVIDENCE OF PASSING THE AWS WELDER QUALIFICATION TEST PROCEDURE (WQTP) FOR THE TYPE OF WORK BEING PERFORMED. CONTRACTOR SHALL SUBMIT PERSONNEL PERFORMANCE QUALIFICATION TEST RECORDS FOR EACH AWS CERTIFIED WELDER AND WELDING PROCEDURE SPECIFICATIONS (WPS) FOR EACH WELDING PROCESS PRIOR TO BEGINNING WORK. 5) MINIMUM SIZE OF FILLET WELD FOR NON-BOLTED CONNECTIONS NOT SPECIFICALLY DETAILED: a) THINNEST MATERIAL THICKNESS: LESS THAN OR EQUAL TO 1/4" SHALL BE 1/8" FILLET b) THINNEST MATERIAL THICKNESS: GREATER THAN 1/4" — 1/2" SHALL BE 3/16" FILLET c) THINNEST MATERIAL THICKNESS: GREATER THAN 1/2" — 3/4" SHALL BE 1/4" FILLET d) THINNEST MATERIAL THICKNESS: GREATER THAN 3/4" SHALL BE 5/16" FILLET 6) MAXIMUM SIZE OF FILLET WELD FOR NON-BOLTED CONNECTIONS NOT SPECIFICALLY DETAILED: a)MATERIAL THICKNESS: LESS THAN 1/4" 1. NOT GREATER THAN THE MATERIAL THICKNESS b)MATERIAL THICKNESS: GREATER THAN OR EQUAL TO 1/4" 1. NOT GREATER THAN THE MATERIAL THICKNESS MINUS 1/16" 7) GENERALLY, BEAM CONNECTIONS HAVE BEEN DESIGNED AS BEARING-TYPE CONNECTIONS AND BOLTS MAY BE INSTALLED TO A SNUG-TIGHT CONDITION UNLESS SPECIFICALLY INDICATED TO BE PRE-TENSIONED. 8) NON-DESTRUCTIVE WELD TESTS MAY BE PERFORMED. DEFICIENT WELDS SHALL BE CORRECTED BY THE CONTRACTOR AND RE-TESTED AT THEIR EXPENSE. 9) ALL EXPOSED STEEL SHALL BE PAINTED UNLESS NOT OTHERWISE SPECIFIED AS GALVANIZED. ALL STRUCTURAL STEEL MEMBERS SHALL RECEIVE A SHOP COAT OF APPROVED PRIMER, 3 MILS DFT. TOUCH UP PRIMER ALL FIELD WELDS AND ABRASIONS TO THE SHOP COAT WITH PAINT COMPATIBLE WITH SHOP COAT. DO NOT SHOP PAINT FAYING SURFACES OF PRE-TENSIONED BOLTED CONNECTIONS. SURFACES SCHEDULED TO RECEIVE SHOP OR FIELD INSTALLED HEADED STUD ANCHORS OR SURFACES EMBEDDED IN CONCRETE. 10) THE ERECTOR SHALL NOT EMPLOY FIT-UP MEANS BEYOND THE USE OF DRIFT PINS OR MINOR HOLE REAMING. THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY ERECTION, FABRICATION OR FIT-UP ISSUES DISCOVERED DURING CONSTRUCTION. CORRECTION OF FIT-UP ERRORS OR MODIFICATIONS OF STRUCTURAL STEEL, INCLUDING ANCHOR RODS, OF ANY DEGREE SHALL BE DISCUSSED WITH THE FABRICATOR AND ENGINEER WITH METHODS APPROVED BY THE ENGINEER BEFORE CORRECTIVE ACTIONS ARE TAKEN. 11) ALL EXTERIOR STEEL LOCATED AT OR BELOW GRADE AND EXPOSED TO EARTH OR WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123 MEETING THE MINIMUM AVERAGE COATING THICKNESS REQUIREMENTS AND HAVE ASPHALT EMULSION APPLIED BELOW GRADE TO PROTECT AGAINST EARTH ELEMENTS. SEE ARCH FOR WATERPROOFING MATERIALS AND DETAILS. 12) STRUCTURAL STEEL MEMBER INDUCED CAMBER SHALL BE BASED UPON THE APPROXIMATE PROFILE OF A CIRCULAR CURVE. COLD CAMBERING AND HEAT CAMBERING ARE BOTH ACCEPTABLE METHODS OF ACHIEVING SPECIFIED CAMBER. IT IS NOT ACCEPTABLE TO UTILIZE A SINGLE POINT INDUCED CAMBER AT MID-SPAN. 13) ALL COPES, CUT-OUTS AND OTHER CUTTING OF STRUCTURAL STEEL MEMBERS. SHALL HAVE ALL REINFORCED CORNERS SHAPED AND NOTCH FREE TO A 1/2" RADIUS MINIMUM. F. PRE-INSTALLATION CONFERENCES 1) SCHEDULING AND CONDUCTING PRE-INSTALLATION CONFERENCES ARE THE RESPONSIBILITY OF THE CONTRACTOR. MEETING ATTENDEES AND FORMAT ARE OUTLINED IN THE PROJECT SPECIFICATIONS. COORDINATE LOCATION, TIME AND AGENDA ITEMS WITH THE ENGINEER. CONDUCT PRE-INSTALLATION CONFERENCES FOR THE FOLLOWING ACTIVITIES RELATED TO STRUCTURAL SYSTEMS: a) SIGN COORDINATION b) EXISITNG CONSTRUCTION REVIEW c) SPECIAL INSPECTION REQUIREMENTS G. SPECIAL INSPECTIONS AND TESTS 1) SPECIAL INSPECTIONS DESCRIBED BELOW ARE REQUIRED BY SECTION 1705 OF THE IBC AND SHALL BE PERFORMED PRIOR TO ISSUANCE OF THE CERTIFICATE OF OCCUPANCY. THE CONTRACTOR IS RESPONSIBLE FOR KEEPING THE ENGINEER APPRISED OF WORK PROGRESS AS IT PERTAINS TO SPECIAL INSPECTIONS AND ENSURING THAT NO WORK REQUIRING SPECIAL INSPECTIONS IS CONCEALED BEFORE SPECIAL INSPECTIONS OCCUR. REFER TO THE PROJECT SPECIFICATIONS FOR OTHER INSPECTIONS AND MATERIALS TESTING REQUIREMENTS. 2) THE OWNER SHALL EMPLOY QUALIFIED SPECIAL INSPECTORS DURING CONSTRUCTION TO PERFORM STRUCTURAL OBSERVATIONS FOR THE ELEMENTS NOTED BELOW. a) STEEL CONSTRUCTION: THE SPECIAL INSPECTIONS FOR STEEL ELEMENTS OF BUILDINGS AND STRUCTURES SHALL BE AS REQUIRED IN SECTION 1705.2 OF THE IBC. SPECIAL INSPECTION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360. INSPECTIONS INCLUDE BUT ARE NOT LIMITED TO PERIODIC INSPECTION OF ALL WELDING INCLUDING STRUCTURAL STEEL, PERIODIC INSPECTION DURING AND AFTER INSTALLATION OF ALL HIGH-STRENGTH BOLTING CONNECTIONS REGARDLESS OF TYPE AND STRUCTURAL STEEL DURING OR AFTER

b) CONCRETE CONSTRUCTION: THE SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION SHALL BE AS REQUIRED BY SECTION 1705.3 AND TABLE 1705.3 OF THE IBC. INSPECTIONS INCLUDE BUT ARE NOT LIMITED TO PERIODIC INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. H. DEFERRED SUBMITTALS 1) DOCUMENTATION SUCH AS SHOP DRAWINGS, ERECTION DRAWINGS AND CALCULATIONS FOR DEFERRED SUBMITTAL ITEMS WILL BE REVIEWED BY THE ENGINEER WHEN AVAILABLE AND FORWARDED TO THE BUILDING OFFICIAL. CONTRACTOR SHALL ALLOW FOR A MINIMUM OF FIVE WORKING DAYS FOR ENGINEER REVIEW OF ALL DEFERRED SUBMITTALS. 2) SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS REQUIRED BY THE PROJECT SPECIFICATIONS FOR REVIEW BY THE ENGINEER PRIOR TO

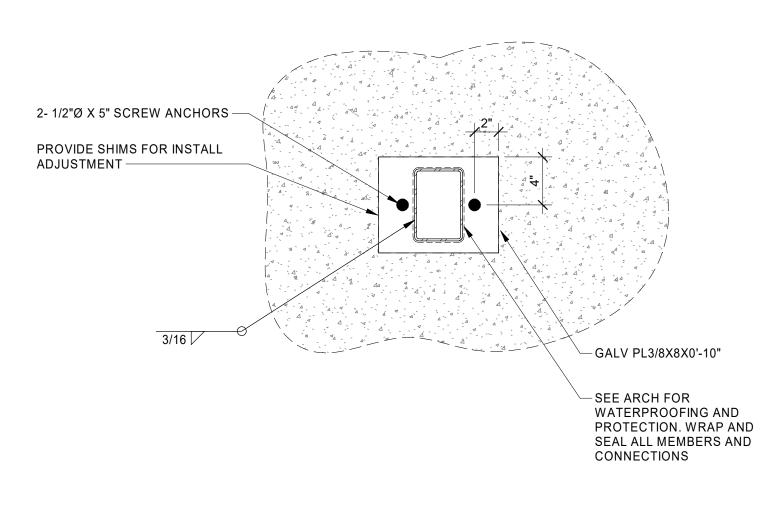
- ENSURE CONFORMANCE WITH CONSTRUCTION DOCUMENTS PRIOR TO SUBMITTING FOR ARCHITECTURAL AND ENGINEERING REVIEW. CONTRACTOR IS RESPONSIBLE FOR VERIFICATION AND COORDINATION OF ALL DIMENSIONS AND DETAILS WITH SUBCONTRACTORS. SHOP DRAWINGS OR PRODUCT DATA NOT STAMPED BY THE CONTRACTOR WILL NOT BE REVIEWED. 4) SHOP DRAWINGS SHALL NOT REPLACE THE CONTRACT DRAWINGS. ITEMS OMITTED OR SHOWN INCORRECTLY ARE NOT CONSIDERED AS CHANGES TO THE CONTRACT DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR THE CORRECTNESS AND COMPLETENESS OF ALL DEFERRED SUBMITTALS 5) DEFERRED SUBMITTALS SHALL CLOUD AND NOTE ANY DEVIATIONS OR SUBSTITUTIONS FROM THE CONTRACT DRAWINGS IN ALL INSTANCES. DEVIATIONS NOT CLOUDED ARE CONSIDERED NOT APPROVED, UNLESS NOTED SPECIFICALLY OTHERWISE BY THE ENGINEER. 6) PRODUCT SUBMITTALS SHALL INCLUDE THE FOLLOWING, UNLESS NOTED OTHERWISE: a) SIGN REHABILITATION AND RE-INSTALLATION DRAWINGS. I. MISCELLANEOUS 1) CONTRACTOR SHALL REVIEW AND VERIFY ALL DIMENSIONS SHOWN ON THE CONSTRUCTION DOCUMENTS AND SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO PROCEEDING WITH CONSTRUCTION. 2) USE ONLY WRITTEN DIMENSIONS FOR CONSTRUCTION. WHERE NO DIMENSION IS PROVIDED, CONSULT THE ENGINEER FOR CLARIFICATION PRIOR TO CONSTRUCTION. 3) DETAIL MARKS ANNOTATED ON PLANS ARE INTENDED TO INDICATE SPECIFIC CONFIGURATION(S) AND INFORMATION. FOR PLAN CLARITY, NOT EVERY LOCATION WHERE A SPECIFIC DETAIL MAY APPLY IS ANNOTATED. CONTACT THE ENGINEER IF CLARIFICATION IS NEEDED. 4) NOTIFY ENGINEER OF ANY DISCREPANCIES DISCOVERED WITH OTHER TRADES. 5) CONSTRUCTION LOADS SHALL NOT BE GREATER THAN THE DESIGN LOADS INDICATED IN DESIGN LOADS AND CRITERIA SECTION B.1, UNLESS REVIEWED AND APPROVED BY THE ENGINEER. 6) TEMPORARILY BRACE THE STRUCTURE TO RESIST ALL LOADS OR COMBINATIONS OF LOADS UNTIL ALL PERMANENT ELEMENTS ARE IN PLACE AND ALL CONNECTIONS ARE COMPLETE AS SHOWN. THE DESIGN AND SAFETY OF ALL ERECTION BRACING, SHORING AND TEMPORARY SUPPORTS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. 7) COSTS ASSOCIATED WITH STRUCTURAL DRAWING CHANGES RESULTING FROM USE OF ALTERNATES OR SUBSTITUTIONS, INCLUDING MECHANICAL EQUIPMENT, IS
- THE CONTRACTOR'S RESPONSIBILITY. 8) CONTRACTOR IS RESPONSIBLE FOR LOCATING, PROTECTING AND STABILIZING ALL ADJACENT STRUCTURES AND UTILITIES THROUGH ALL PHASES OF CONSTRUCTION.
- 9) STRUCTURAL GENERAL NOTES SHALL NOT BE A SUBSTITUTE FOR THE PROJECT SPECIFICATIONS. CONFLICTS BETWEEN THE STRUCTURAL GENERAL NOTES AND PROJECT SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OR THE STRICTER OF THE TWO CRITERIA SHALL BE USED.
- J. ABBREVIATIONS LIST (SOME OF THE LISTED ABBREVIATIONS MAY NOT APPEAR ON THE DRAWINGS) 1)& AND
- 3) ACI AMERICAN CONCRETE INSTITUTE
- 4) AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION 5) AISI AMERICAN IRON AND STEEL INSTITUTE 6) ANC ANCHOR
- 7) APPR APPROXIMATE
- 8) ARCH ARCHITECTURE OR ARCHITECTURAL 9) ASCE AMERICAN SOCIETY OF CIVIL ENGINEERS
- 10) ASTM AMERICAN SOCIETY FOR TESTING AND MATERIALS 11) AWS AMERICAN WELDING SOCIETY
- 12) BLDG BUILDING
- 13) BOT BOTTOM
- 14) BRG BEARING 15) BTWN BETWEEN
- 16) CIP CAST-IN-PLACE
- 17) CL CENTER LINE
- 18) CLR CLEAR
- 19) COL COLUMN
- 20) CONC CONCRETE 21) CONN CONNECTION OR CONNECTOR
- 22) CONT CONTINUE OR CONTINUOUS
- 23) DET DETAIL
- 24) DIA DIAMETER 25) DIM DIMENSION
- 26) DIMS DIMENSIONS
- 27) DIR DIRECTION
- 28) EA EACH
- 29) EQ EQUAL
- 30) EXIST OR (E) EXISTING
- 31) FIN FINISH
- 32) FL FLOOR
- 33) FDN FOUNDATION
- 34) FT FOOT OR FEET
- 35) FTG FOOTING 36) GA GAUGE OR GAGE
- 37) GALV GALVANIZED OR GALVANIZE
- 38) HOR HORIZONTAL
- 39) HP HIGH POINT
- 40) HS HIGH STRENGTH
- 41) HSS HOLLOW STRUCTURAL SECTION (TUBE STEEL) 42) HT HEIGHT
- 43) IBC INTERNATIONAL BUILDING CODE 44) ICBO INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS
- 45) IN INCH OR INCHES
- 46) K KIP (1000 POUNDS)
- 47) KSI KIPS PER SQUARE INCH
- 48) LB POUND
- 49) LBS POUNDS 50) LF LINEAR FEET OR LINEAL FEET
- 51) LLH LONG LEG HORIZONTAL
- 52) LLV LONG LEG VERTICAL
- 53) MAX MAXIMUM 54) MFR MANUFACTURER
- 55) MIN MINIMUM
- 56) (N) NEW 57) N/A NOT APPLICABLE
- 58) NTS NOT TO SCALE
- 59) OC ON CENTER 60) PERP PERPENDICULAR
- 61) PL PLATE
- 62) PROJ PROJECTION
- 63) PSF POUNDS PER SQUARE FOOT 64) PSI POUNDS PER SQUARE INCH
- 65) PVC POLYVINYL CHLORIDE
- 66) QTY QUANTITY 67) (R) RELOCATE OR RELOCATED
- 68) REINF REINFORCE, REINFORCED, REINFORCEMENT OR REINFORCING
- 69) REQD REQUIRED 70) SF SQUARE FOOT OR SQUARE FEET
- 71) SIM SIMILAR
- 72) SLV SHORT LEG VERTICAL
- 73) SOG SLAB ON GRADE 74) SPA SPACE OR SPACES
- 75) SPEC SPECIFIED OR SPECIFICATION 76) STD STANDARD
- 77) STIFF STIFFENER
- 78) STL STEEL
- 79) STRUCT STRUCTURAL OR STRUCTURE
- 80) SUP SUPPORT
- 81) SYM SYMMETRICAL 82) TOCW TOP OF CONCRETE WALL
- 83) TYP TYPICAL
- 84) UNO UNLESS NOTED OTHERWISE
- 85) VIF VERIFY IN FIELD 86) W/WITH
- 87) W/O WITHOUT

INSTALLATION, WELDING INSPECTION AND INSPECTOR QUALIFICATION SHALL BE IN COMPLIANCE WITH AWS D1.1 "STRUCTURAL WELDING CODE - STEEL".

FABRICATION. SHOP DRAWINGS FOR PROPRIETARY PRODUCTS DESIGNED BY THE MANUFACTURER SHALL INCLUDE DESIGN CALCULATIONS STAMPED BY AN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. 3) THE CONTRACTOR SHALL REVIEW AND STAMP ALL DEFERRED SUBMITTALS TO

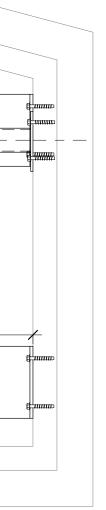
ORIENTATION OF SIGN ABOVE - EXIST CONCRETE PLANTER CLIP ANGLE, 4 LOCS. LOCATION CAN VARY TO MISS SLAB REINF — \S101 TYP \ \$101 <u>ω_</u>λ__ 8"Ø STD PIPE -HSS6X4X1/4 3 LOCS -3'-2" S101 3'-0" NOTE: ALL DIMENSIONS, ELEVATIONS AND ANGLES TO BE VERIFIED PRIOR TO FABRICATION. ALL MATERIAL TO BE GALVANIZED.

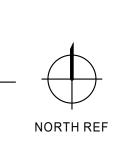


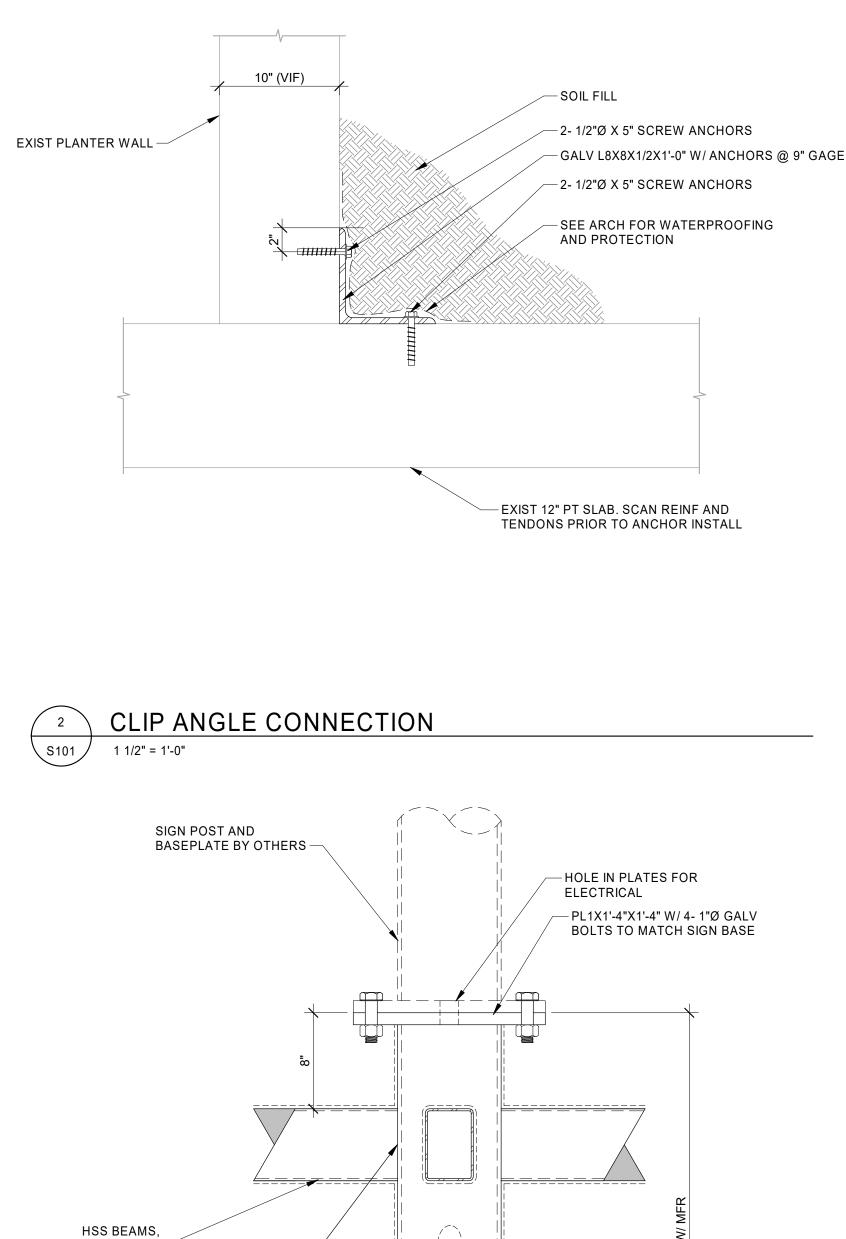


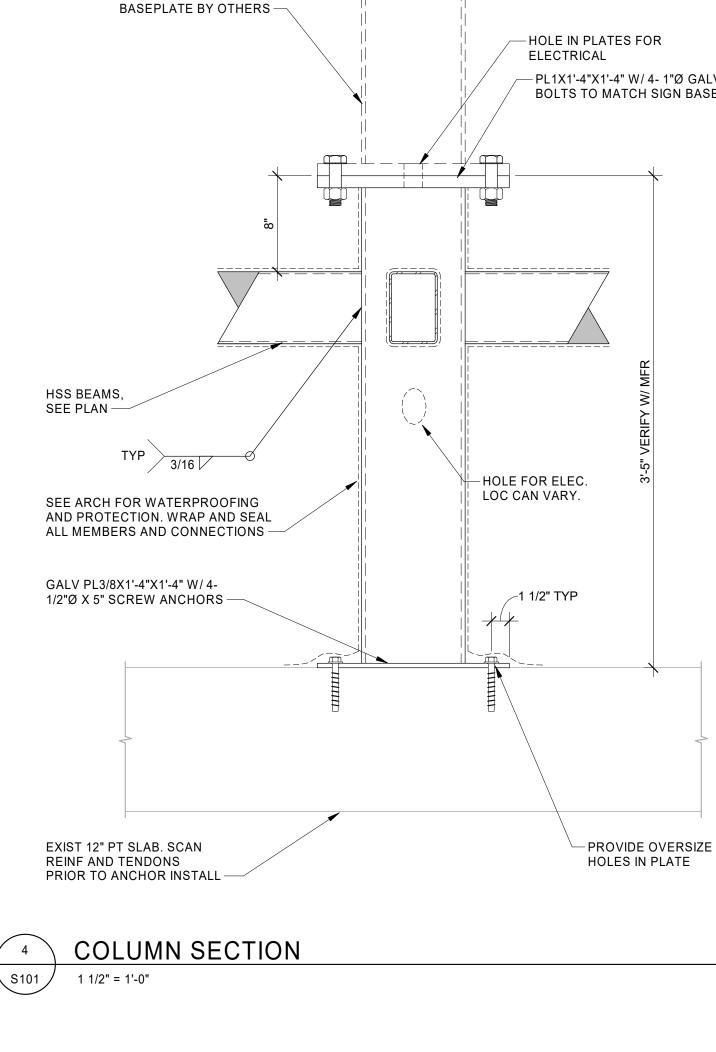


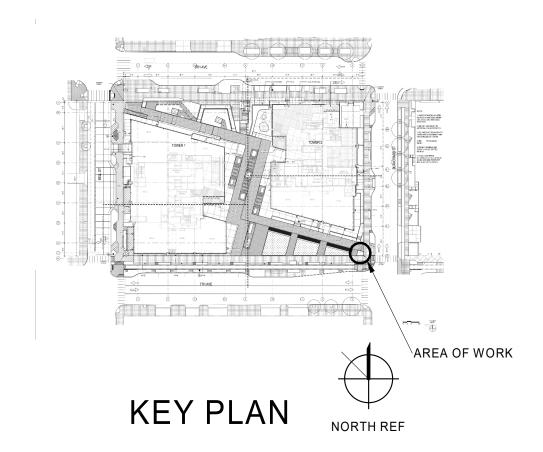










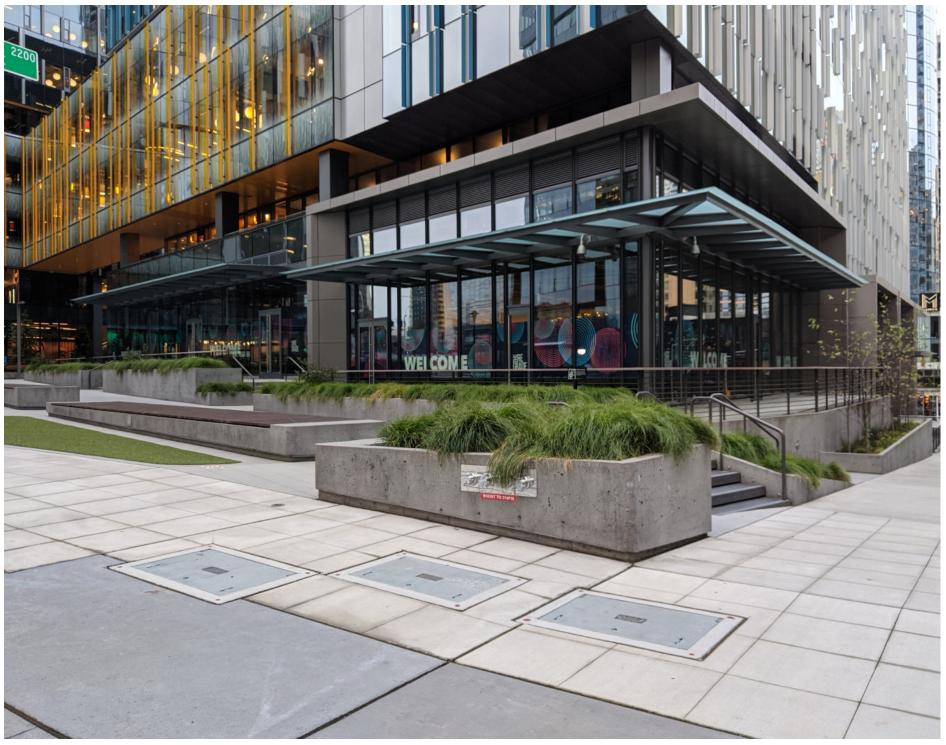


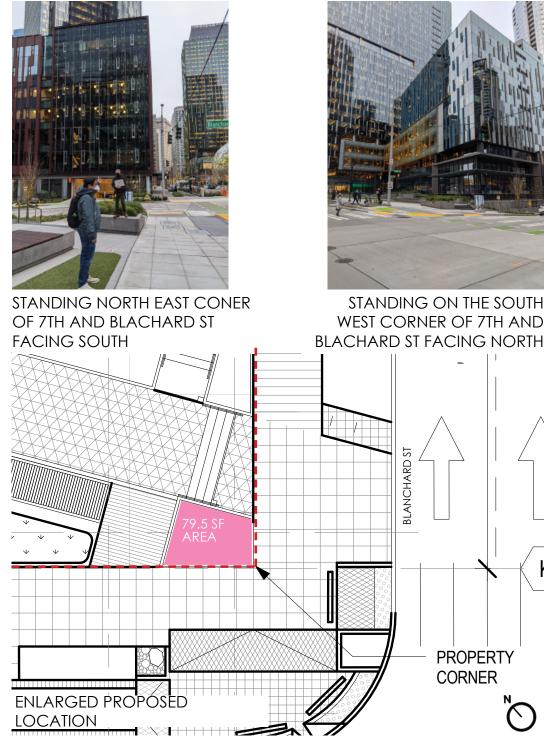


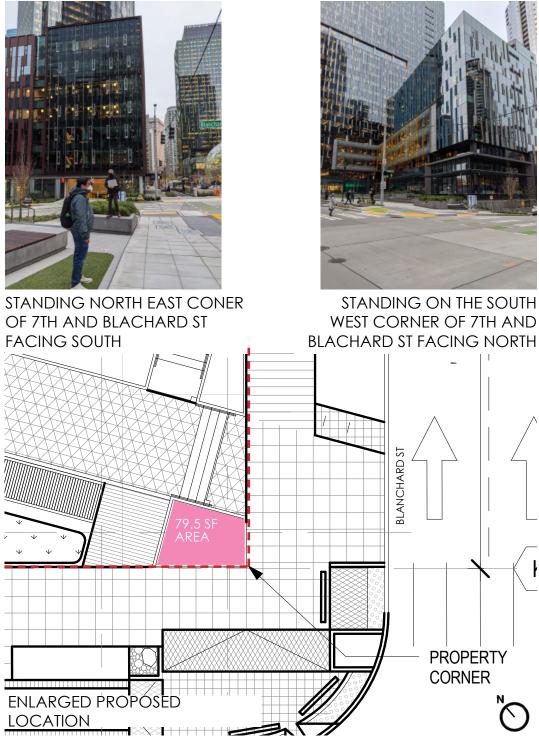
cushingterrell.com 800.757.9522

Description:	No:	By:	Date:
	-		
	-		
ART INSTALLATION 2200 7th Avenue Seattle WA 98121			
2200 7th Avenue, Seattle WA 98121			
Project Title & Address:			
•			
Seal:			
City Approval:			
FOUNDATION PLAN			
Sheet Title:	_	_	
Project No: 117_7817 Drawn By: GLEASON			
Project Manager:			
Designer: JORGENSEN	C	10	1
Reviewed By:	С	ТU	1
Approved By:			
ALL DIMENSIONS AND CONDITIONS MU ON SITE BY THE CONTRACTOR AND SUB	B-CONTRA	CTORS. T	HE PROJECT
MANAGER SHALL BE NOTIFIED IN WRITING OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH THE WORK.			
-			

PROPOSED LOCATION - EXISITING CONDITION OF PLANTER IN PUBLIC ACCESSIBLE PLAZA







PLANTER IN PUBLIC ACCESSIBLE SPACE FACING NORTH EAST