

Schellings Residence

5218 16TH AVE NE, SEATTLE WA 98105

APN # 882390-0620

7/7/2020 PROGRESS SET FOR
LANDMARKS BOARD REVIEW

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PROJECT DIRECTORY

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T: (206) 957-3907

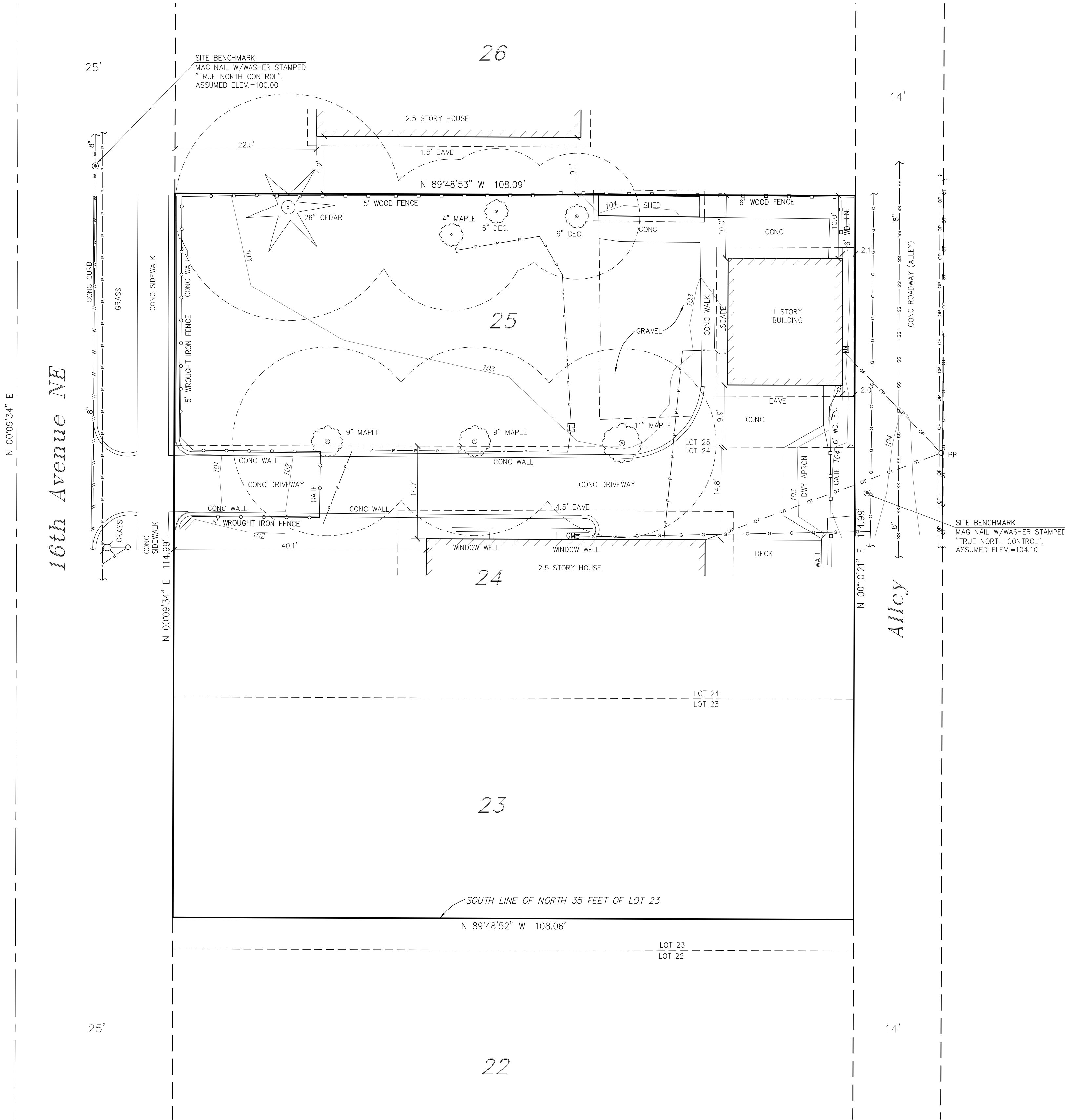
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**SUNDBERG
KENNEDY
LY-AU YOUNG
ARCHITECTS**

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206.322.1130

SKLARCHITECTS.COM



LEGAL DESCRIPTION

THE NORTH 35 FEET OF LOT 23 AND ALL OF LOTS 24 AND 25 IN BLOCK 4 OF UNIVERSITY PARK ADDITION TO THE CITY OF SEATTLE, ACCORDING TO THE PLAT THEREOF RECORDED IN VOLUME 13 OF PLATS, PAGE 85, IN KING COUNTY, WASHINGTON.

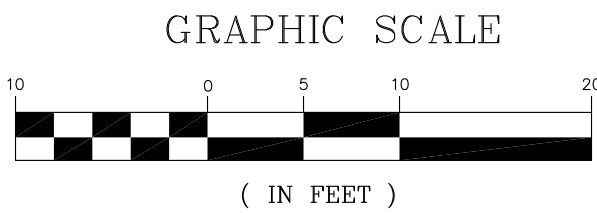
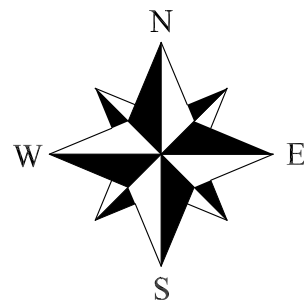
TAX PARCEL NO. 882390-0620

NOTES

1. BASIS OF BEARINGS: THE BEARINGS SHOWN HEREON ARE BASED ON THE CENTERLINE OF 16TH AVENUE NE BETWEEN FOUND MONUMENTS AS SHOWN ON THE RECORD OF SURVEY FILED IN BOOK 312 AT PAGE 54. BEING N 00°09'34\"/>

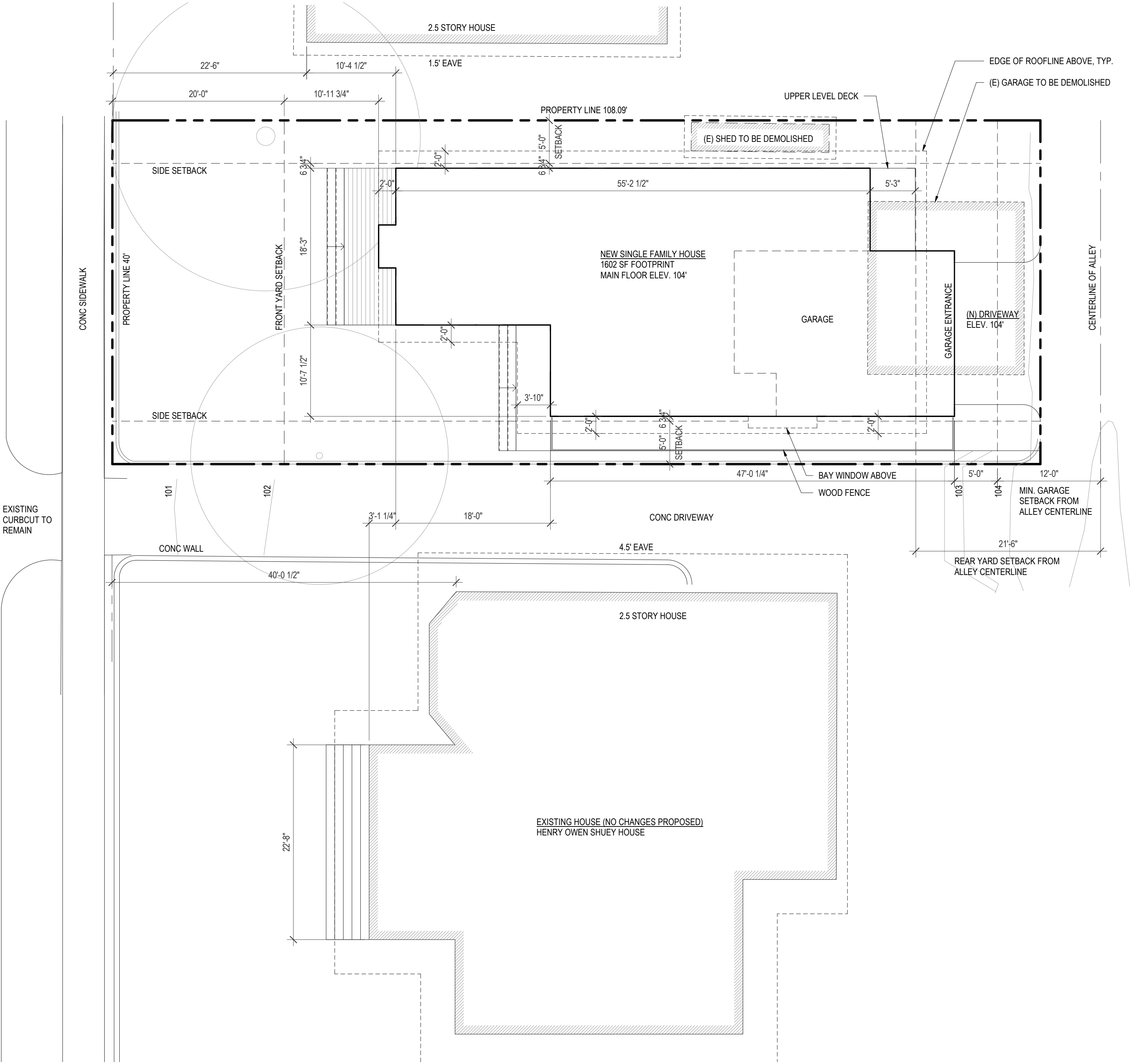
LEGEND

- | | | | |
|-------------|-----------------------------|--|---------------------------|
| — — — — — | BOUNDARY LINE | | ELECTRIC METER |
| - - - - - | RIGHT-OF-WAY LINE | | GAS METER |
| — — — — — | CENTERLINE | | IRRIGATION BOX |
| - - - - - | LOT LINE | | POWER POLE |
| — SS — SS — | SEWER LINE | | POWER POLE W/LUMINAIRE |
| — OP — OP — | OVERHEAD POWER | | DECIDUOUS TREE W/DRIPLINE |
| — P — P — | UNDERGROUND POWER | | CONIFER TREE W/DRIPLINE |
| — OT — OT — | OVERHEAD COMMUNICATION LINE | | |
| — W — W — | WATER LINE | | |
| — G — G — | GAS LINE | | |
| — □ — □ — | WOOD FENCE | | |
| — ○ — ○ — | WROUGHT IRON FENCE | | |



| | | | | | | | | | | |
|---|-----|----------|------|----|-------|---|--|---|--|--------------------------|
| SURVEYED: KH/AC DRAWN: EF CHECKED: JM | | | | | | 815 S. Weller Street Suite 200 Seattle, WA 98104-3023 206.332.0800 | | Date: 9-03-2019 | 5218 16th Avenue NE, Seattle, WA SURVEY SITE PLAN For SCHELLINGS | Job Number: 19-116.00 |
| | | | | | | | | Scale: 1" = 10' | | Sheet: 1 of 1 |
| | | | | | | | | Book: J19119.dwg | | |
| | REV | REVISION | DATE | BY | APP'D | | | PORTION OF THE NW 1/4 OF THE SW 1/4 OF SECTION 9, T 25 N, R 4 E, W.M. | | |

16TH AVENUE NE



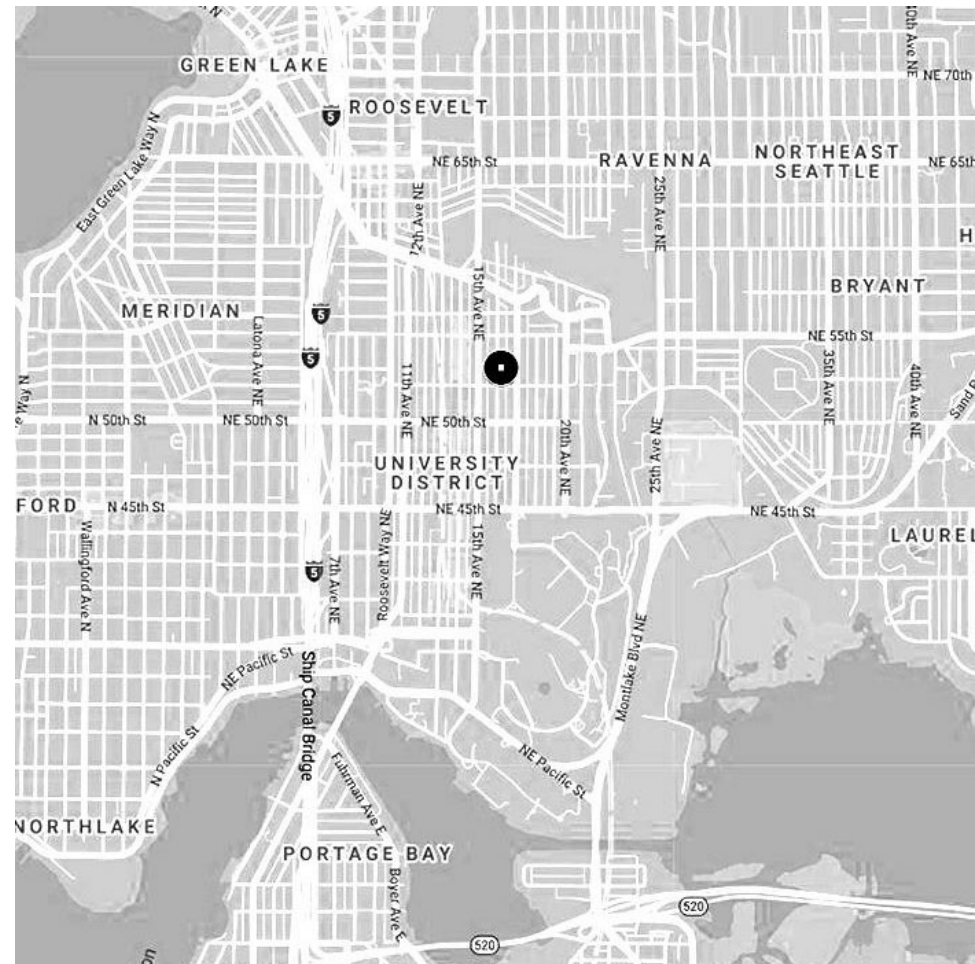
1 SITE PLAN
SCALE: 1/8" = 1'-0"

2 SITE PLAN - DEMOLITION
SCALE: 1/16" = 1'-0"

VICINITY MAP



LOCATION MAP



ZONING./BUILDING CODE SUMMARY

PROJECT ADDRESS:
5218 16TH AVENUE NE
SEATTLE, WA 98105

ASSESSOR'S PARCEL NUMBER:
882390-0620

PROJECT DESCRIPTION:
NEW 2-STORY SINGLE FAMILY HOUSE WITH ATTACHED
GARAGE AND ATTACHED ACCESSORY DWELLING UNIT

LEGAL DESCRIPTION:
THE NORTH 35 FEET OF LOT 23 AND ALL OF LOTS 24 AND 25
IN BLOCK 4 OF UNIVERSITY PARK ADDITION TO THE CITY OF
SEATTLE, ACCORDING TO THE PLAT THEREOF RECORDED IN
VOLUME 13 OF PLATS, PAGE 85, IN KING COUNTY,
WASHINGTON.

LAND USE:
SINGLE FAMILY SF5000

LOT SIZE:
LOT 25 = 108' X 40' = 4320 SF
ALLEY AREA: 40X7 = 280 SF
LOT AREA: 4320 + 280 SF = 4600 SF TOTAL LOT AREA
LOTS COVERAGE CALCULATIONS FOR LOTS LESS THAN 5000
SF IN AREA = 1000 SF + 15% OF LOT = 1000 + (4600)(.15)= **1690**
SF LOT COVERAGE ALLOWED

HEIGHT:
ALLOWED: 30 FT BASE HEIGHT W/ 5 FT ADDITIONAL FOR
PITCHED ROOF
PROPOSED: 27'-4" MAX HEIGHT FROM GROUND LEVEL

YARD SETBACKS:
FRONT: 20' SETBACK MINIMUM. (20 FT OR THE AVERAGE OF
THE FRONT YARDS ON EITHER SIDE OF THE LOT, WHICHEVER
IS LESS.)
30'-11" FRONT SETBACK PROPOSED

SIDE: 5 FT MINIMUM. 5'-6 3/4" SIDE SETBACK PROPOSED.

REAR: 25 FEET OR 20 PERCENT OF LOT DEPTH, WHICHEVER
IS LESS, EXCEPT THAT IT MAY NEVER BE LESS THAN 10 FEET
108.09' X 0.20 = 21.6' **REAR SETBACK OF HOUSE FROM
CENTERLINE OF ALLEY PROPOSED.**

GARAGE: GARAGES MAY BE LOCATED IN A REQUIRED YARD
SUBJECT TO THE STANDARDS OF SECTION 23.44.016.

23.44.016. D.5. PARKING AND GARAGES IN REQUIRED YARDS
ATTACHED GARAGES SHALL NOT BE LOCATED WITHIN 12
FEET OF THE CENTERLINE OF ANY ALLEY.
GARAGE TO BE MIN. 12' SETBACK FROM ALLEY CENTERLINE.

SUNDBERG
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1501 E MADISON, SUITE 205
SEATTLE WA 98122-4465
206.322.1130

Official
Stamps:

SCHELLINGS HOUSE
5218 16th Avenue NE, Seattle, WA 98105

REVISIONS

NO. DESCRIPTION

DATE

19010
7/7/2020

Project Manager
NUGLAY

Drawn by
NL

Checked by
Checker

Scale
As Indicated

LANDMARKS BOARD
REVIEW

PROGRESS SET

7/7/2020

SITE PLAN

A1.00

NOT FOR CONSTRUCTION

Official
Stamps:

SCHELLINGS HOUSE
5218 16th Avenue NE, Seattle, WA 98105

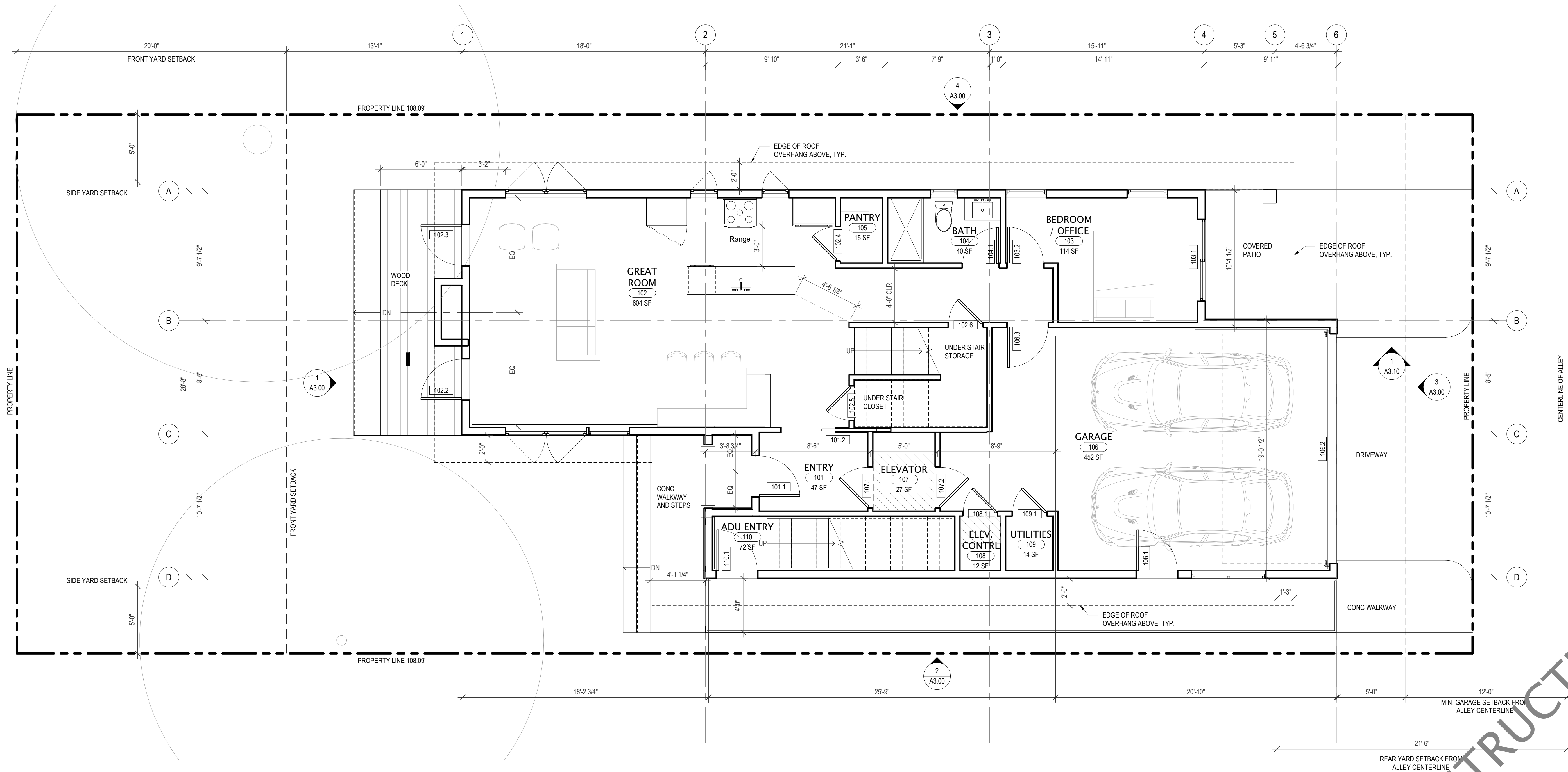
| REVISIONS | NO. | DESCRIPTION | DATE | LANDMARKS BOARD | |
|-----------|-----|-------------|------|-----------------|--------------|
| | | | | REVIEW | PROGRESS SET |
| | | | | | 7/7/2020 |

FLOORPLANS

A2.00

Scale 1/4" = 1'-0"

| | |
|-----------------|----------|
| Project number | 19010 |
| Date | 7/7/2020 |
| Project Manager | NUGLAY |
| Drawn by | NL |
| Checked by | - |



1

MAIN FLOOR PLAN

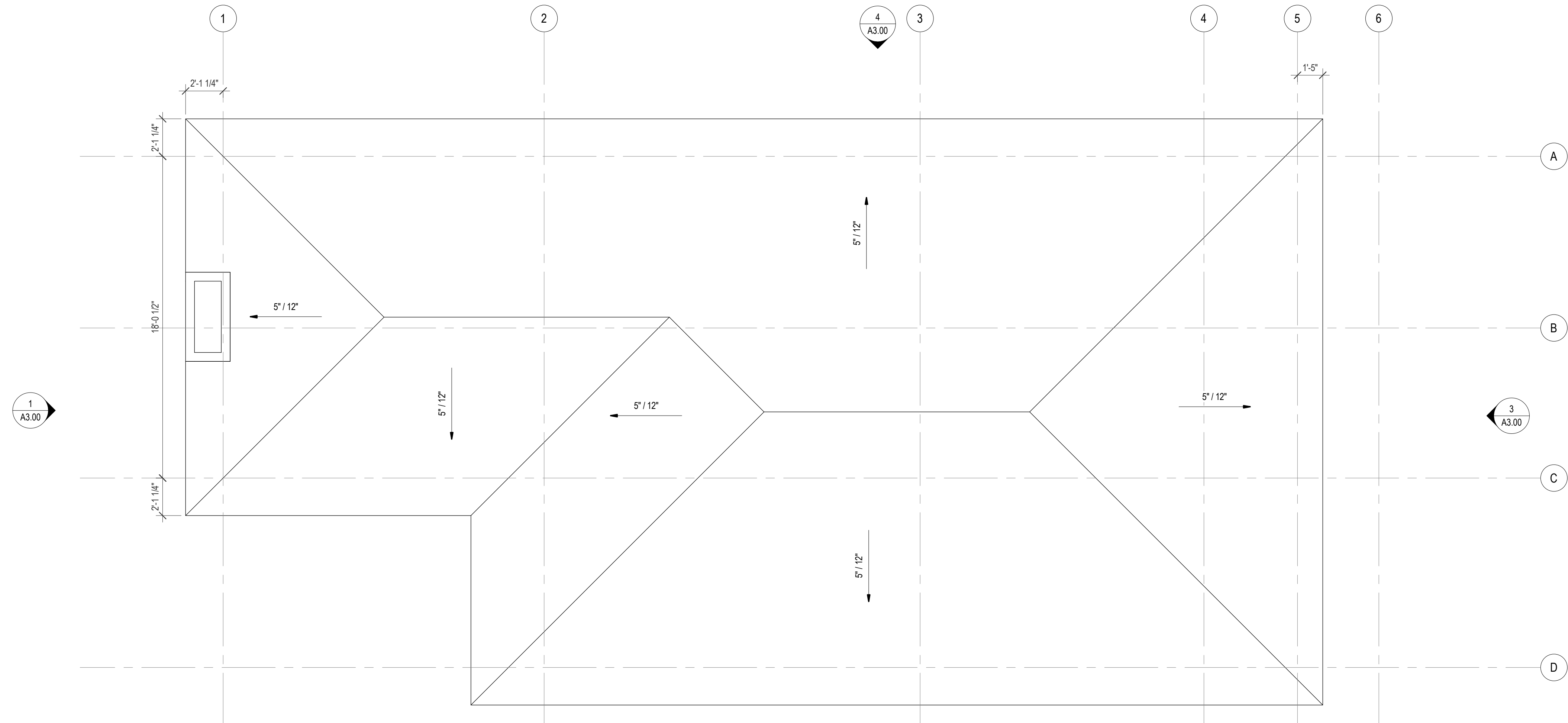
SCALE: 1/4" = 1'-0"

SQ FT BREAKDOWN

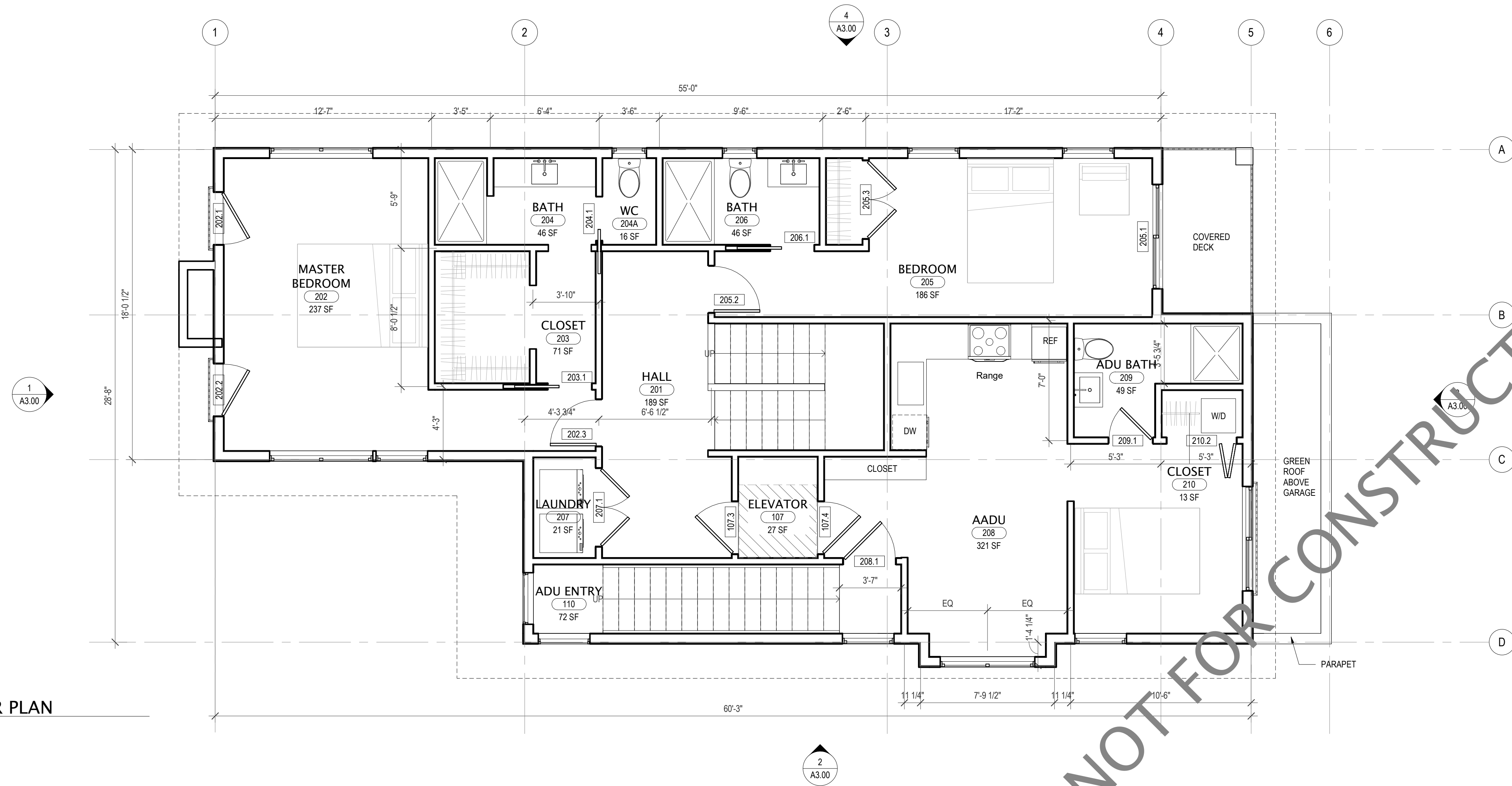
FIRST FLOOR = 1,600 SF
SECOND FLOOR = 1,530 SF
TOTAL HOUSE = 3,030 SF

C:\Users\Wood\Documents\18010 Scheellings House Permit\Drawings\18010 Scheellings House Permit.dwg

2 ROOF PLAN
SCALE: 1/4" = 1'-0"



1 2ND FLOOR PLAN
SCALE: 1/4" = 1'-0"



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Official
Stamps:

SCHELLINGS HOUSE
5218 16th Avenue NE, Seattle, WA 98105

REVISIONS

NO. DESCRIPTION

DATE

LANDMARKS BOARD
REVIEW

PROGRESS SET

7/7/2020

UPPER LEVEL AND ROOF PLANS

A2.10

Scale 1/4" = 1'-0"

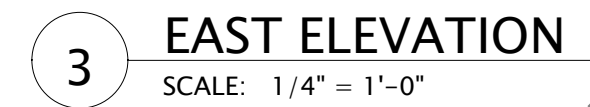
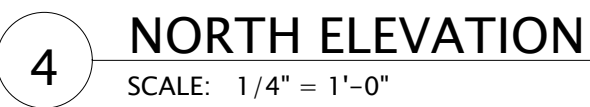
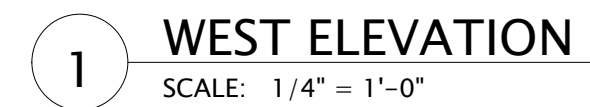
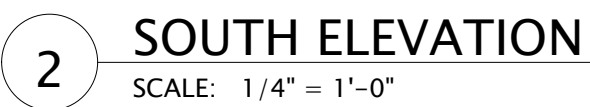
Project number 19010

Date 7/7/2020

Project Manager NUGLAY

Drawn by NL

Checked by



| | | |
|------------------------|-----------------|--------------|
| A3.00 | Scale | 1/4" = 1'-0" |
| | Project number | 1910 |
| | Date | 7/7/2020 |
| | Project Manager | NUGLAY |
| | Drawn by | NL |
| Checked by | | Checker |
| REVISIONS | | DATE |
| NO. DESCRIPTION | | |
| LANDMARKS BOARD REVIEW | | |
| PROGRESS SET | | |
| | | 7/7/2020 |

A3.00

C:\Users\Nicole\Documents\19010 Schellings House Permit Central_nicole@starchitects.com.rvt

GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

GEOTECHNICAL

13. FOUNDATION NOTES: ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE ASSUMED AND THEREFORE MUST BE VERIFIED IN THE FIELD. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED, NOTIFY THE STRUCTURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.

FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL OR BOTH) AT LEAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS SHOWN ON PLANS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS OF FOOTINGS MUST BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD. UNLESS OTHERWISE NOTED, FOOTINGS SHALL BE CENTERED UNDER COLUMNS OR WALLS ABOVE.

BACKFILL BEHIND ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE.

THE STRUCTURAL DESIGN IS BASED ON THE FOLLOWING ASSUMED VALUES:
ALLOWABLE SOIL PRESSURE 2,000 PSF

CONCRETE

14. CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 301. CONSTRUCTION TOLERANCES SHALL NOT EXCEED THOSE LISTED IN ACI 117. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF $f'_c = 3,000$ PSI AND MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENT PER CUBIC YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS (BEFORE THE ADDITION OF ADMIXTURES). THE WATER/CEMENT RATIO SHALL NOT EXCEED 0.55 FOR FOOTINGS AND 0.45 FOR ALL SLABS AND EXPOSED CONCRETE UNLESS OTHERWISE NOTED. EXCEPT FOR FOOTINGS AND SLAB ON GRADE, AGGREGATE SIZE SHALL NOT EXCEED 3/4".

THE MINIMUM AMOUNT OF CEMENT AND THE MAXIMUM SLUMP MAY BE CHANGED IF A CONCRETE PERFORMANCE MIX IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT AND THE SEATTLE DCI FOR APPROVAL TWO WEEKS PRIOR TO PLACING ANY CONCRETE. (THE W/C RATIO LIMITS STILL APPLY). THE PERFORMANCE MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITIOUS MATERIAL, FINE AND COARSE AGGREGATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, CONCRETE YIELD AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 301. CHEMICAL ADMIXTURES AND FLY ASH SHALL CONFORM TO ASTM C494 AND C618 RESPECTIVELY. FLY ASH PERCENTAGE OF TOTAL CEMENTITIOUS MATERIAL SHALL NOT EXCEED 20%. THE USE OF A PERFORMANCE MIX REQUIRES BATCH PLANT INSPECTION, THE COST OF WHICH SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES ONLY THAT INFORMATION PRESENTED CONFORMS GENERALLY TO CONTRACT DOCUMENTS. CONTRACTOR MAINTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

ALL CONCRETE WITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260. TOTAL AIR CONTENT FOR FROST-RESISTANT CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318-14 TABLE 19.3.3.1. ALL CONCRETE EXPOSED TO THE WEATHER AND ALL GARAGE SLABS-ON-GRADE SHALL OBTAIN A 28-DAY STRENGTH f'_c OF 3,000 PSI IN ACCORDANCE WITH ACI 318 TABLE 19.3.2.1 AND IBC SECTION 1904.1. THIS INCREASE IN REQUIRED STRENGTH IS FOR DURABILITY ONLY (SPECIAL INSPECTION IS NOT REQUIRED). ALL CONCRETE TO RECEIVE A STEEL TROWELED FINISH SHALL NOT BE AIR-ENTRAINED.

15. REINFORCING STEEL (FOR RESIDENTIAL) SHALL CONSIST OF #4 BARS CONFORMING TO ASTM A615, GRADE 40, $f_y = 40,000$ PSI AND SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315 AND 318. LAP ALL CONTINUOUS REINFORCEMENT 48 BAR DIAMETERS, 2'-0" MINIMUM. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS, LAP 2'-0" MINIMUM. PROVIDE (2) #4 MIN. U.O. TRIM BARS AROUND ALL OPENINGS IN CONCRETE WALLS OR SLABS EXTENDING 2'-0" PAST CORNERS, TYPICAL.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 8" AT SIDES AND ENDS.

NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS SPECIFICALLY SO DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. NO REINFORCING BARS SHALL BE "WET-SET" INTO THE CONCRETE. PROVIDE A 20' LONG REBAR GROUND (UPER GROUND) PER ELECTRICIAN.

16. CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

FOOTINGS AND OTHER INFORMED SURFACES CAST AGAINST EARTH 3"
FORMED SURFACES EXPOSED TO EARTH (i.e. WALLS BELOW GROUND) OR WEATHER 2"
SLABS AND WALLS (INTERIOR FACE) 1"

17. CAST-IN-PLACE CONCRETE: SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS OF DOOR AND WINDOW OPENINGS IN ALL CONCRETE WALLS. SEE MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF MISCELLANEOUS MECHANICAL OPENINGS THROUGH CONCRETE WALLS. SEE ARCHITECTURAL DRAWINGS FOR ALL GROOVES, NOTCHES, CHAMFERS, FEATURE STRIPS, COLOR, TEXTURE, AND OTHER FINISH DETAILS AT ALL EXPOSED CONCRETE SURFACES, BOTH CAST-IN-PLACE AND PRECAST.

ANCHORAGE

18. EXPANSION BOLTS INTO CONCRETE SHALL BE "STRONG-BOLT 2 WEDGE ANCHOR", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3031 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.
19. SCREW ANCHORS INTO CONCRETE SHALL BE "TITEN HD", AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-2713 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL SCREW ANCHOR INSTALLATION.

20. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO CONCRETE SHALL BE INSTALLED USING "AT-XP" ADHESIVE AS MANUFACTURED BY SIMPSON STRONG-TIE ANCHOR SYSTEMS. INSTALL IN STRICT ACCORDANCE WITH IAPMO UES REPORT NO. ER-263, INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.

WOOD

21. FRAMING LUMBER: SHALL BE KILN DRIED OR MC-19 (MOISTURE CONTENT LESS THAN 19%), AND GRADED AND MARKED IN CONFORMANCE WITH N.C.L.I.B. STANDARD NO. 17 GRADING RULES FOR WEST COAST LUMBER. FURNISH TO THE FOLLOWING MINIMUM STANDARDS:

JOISTS (2X, 3X, AND 4X MEMBERS) DOUGLAS FIR NO. 2

BEAMS AND STRINGERS (INCLUDING 6 X AND LARGER MEMBERS) DOUGLAS FIR NO. 1

POSTS AND TIMBERS DOUGLAS FIR NO. 1

STUDS, PLATES & MISCELLANEOUS LIGHT FRAMING DOUGLAS FIR OR HEM-FIR NO. 2
(AS NOTED ON PLANS / DETAILS)

22. GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ASTM D3737 AND ANSI A190.1 STANDARDS IN A CITY OF SEATTLE CERTIFIED PLANT. EACH MEMBER SHALL BEAR AN A.I.T.C. IDENTIFICATION MARK AND SHALL BE ACCOMPANIED BY AN A.I.T.C. CERTIFICATE OF CONFORMANCE. CERTIFICATES OF CONFORMANCE MUST BE MADE AVAILABLE TO BUILDING INSPECTORS. CITY INSPECTION IS REQUIRED PRIOR TO COVERING GLUED LAMINATED MEMBERS. ALL SIMPLE SPAN BEAMS SHALL BE DOUGLAS FIR COMBINATION 24F-V4, $F_b = 2,400$ PSI, $F_v = 240$ PSI, $E = 1,800$ KSI. ALL CANTILEVERED OR CONTINUOUS BEAMS SHALL BE DOUGLAS FIR COMBINATION 24F-V8, $F_b = 2,400$ PSI, $F_v = 240$ PSI, $E = 1,800$ KSI. CAMBER ALL SIMPLE SPAN GLULAM BEAMS TO 5,000' RADIUS UNLESS SHOWN OTHERWISE ON THE PLANS. ALL GLUE LAMINATED COLUMNS SHALL BE DOUGLAS FIR COMBINATION 2, $F_c = 1,900$ PSI, $F_{by} = 1,800$ PSI, $F_{bx} = 1,700$ PSI, $E = 1,700$ KSI (4 LAMS MINIMUM DEPTH).

23. LAMINATED VENEER LUMBER (LVL) SHALL BE DESIGNED AND MANUFACTURED PER ASTM D5456. EACH PIECE SHALL BEAR A STAMP OR STAMPS NOTING THE NAME AND PLANT NUMBER OF THE MANUFACTURER, THE GRADE, AND THE INDEPENDENT INSPECTION AGENCY'S LOGO. ALL LAMINATED VENEER LUMBER SHALL BE MANUFACTURED USING DOUGLAS FIR VENEER GLUED WITH A WATERPROOF ADHESIVE MEETING THE REQUIREMENTS OF ASTM D2554 WITH ALL GRAIN PARALLEL WITH THE LENGTH OF THE MEMBER. MINIMUM STRUCTURAL PROPERTIES ARE AS FOLLOWS:

$F_b = 2600$ PSI, $E = 2.0 \times 10^6$ PSI, $F_v = 285$ PSI

DESIGN SHOWN ON PLANS IS BASED ON MATERIALS MANUFACTURED BY THE MEYERHAEUSER CORPORATION. ALTERNATE MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.

24. LAMINATED STRAND LUMBER (LSL) SHALL BE DESIGNED AND MANUFACTURED PER ASTM D5456. EACH PIECE SHALL BEAR A STAMP OR STAMPS NOTING THE NAME AND PLANT NUMBER OF THE MANUFACTURER, THE GRADE, AND THE INDEPENDENT INSPECTION AGENCY'S LOGO. ALL LAMINATED STRAND LUMBER SHALL BE MANUFACTURED USING A WATERPROOF ADHESIVE MEETING THE REQUIREMENTS OF ASTM D2554. MINIMUM STRUCTURAL PROPERTIES ARE AS FOLLOWS:

RIM JOISTS AND BLOCKING (1-1/4" MINIMUM THICKNESS AT NON-SHEAR WALLS; SEE SCHEDULE FOR MINIMUM THICKNESS AT SHEAR WALLS):

$F_b = 1700$ PSI, $E = 1.3 \times 10^6$ PSI, $F_v = 400$ PSI

BEAMS AND HEADERS:
 $F_b = 2325$ PSI, $E = 1.55 \times 10^6$ PSI, $F_v = 310$ PSI

DESIGN SHOWN ON PLANS IS BASED ON MATERIALS MANUFACTURED BY THE MEYERHAEUSER CORPORATION. ALTERNATE MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.

25. PARALLEL STRAND LUMBER (PSL) SHALL BE DESIGNED AND MANUFACTURED PER ASTM D5456. EACH PIECE SHALL BEAR A STAMP OR STAMPS NOTING THE NAME AND PLANT NUMBER OF THE MANUFACTURER, THE GRADE, AND THE INDEPENDENT INSPECTION AGENCY'S LOGO. ALL PARALLEL STRAND LUMBER SHALL BE MANUFACTURED USING DOUGLAS FIR STRANDS GLUED WITH A WATERPROOF ADHESIVE MEETING THE REQUIREMENTS OF ASTM D2554 WITH ALL GRAIN PARALLEL WITH THE LENGTH OF THE MEMBER. MINIMUM STRUCTURAL PROPERTIES ARE AS FOLLOWS:

$F_b = 2400$ PSI, $E = 2.2 \times 10^6$ PSI, $F_v = 240$ PSI

DESIGN SHOWN ON PLANS IS BASED ON MATERIALS MANUFACTURED BY THE MEYERHAEUSER CORPORATION. ALTERNATE MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.

26. WOOD I-JOIST DESIGN SHOWN ON PLANS IS BASED ON JOISTS MANUFACTURED BY THE MEYERHAEUSER CORPORATION. ALTERNATE I-JOIST MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER. ALTERNATE JOIST HANGERS AND OTHER HARDWARE MAY BE SUBSTITUTED FOR ITEMS SHOWN PROVIDED THEY HAVE I.C.C. OR IAPMO UES APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH WOOD JOIST PROVIDED. GLUE FLOOR JOISTS TO SHEATHING AS REQUIRED BY THE JOIST MANUFACTURER.

CRITERIA

1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, THE 2015 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC), AND THE SEATTLE DEPARTMENT OF CONSTRUCTION AND INSPECTION (DCI) BUILDING CODE MODIFICATIONS TO THE IBC.

DESIGN LOADING CRITERIA

ROOF SNOW LOAD 25 PSF
FLOOR LIVE LOAD (RESIDENTIAL) 40 PSF
FLOOR LIVE LOAD (RESIDENTIAL EXTERIOR DECKS AND BALCONIES) 60 PSF

WIND : ANALYSIS PROCEDURE: ASCE 7-10 CHAPTER 27 "PART II - ENCLOSED SIMPLE DIAPHRAGM"
RISK CATEGORY II
110 MPH
EXPOSURE "B"
TOPOGRAPHIC FACTOR $K_{zt} = 1.36$
WIND BASE SHEAR, NORTH/SOUTH $V_w = 32.0$ K
WIND BASE SHEAR, EAST/WEST $V_w = 12.5$ K

EARTHQUAKE : ANALYSIS PROCEDURE: IBC "EQUIVALENT LATERAL FORCE PROCEDURE"
SEISMIC DESIGN CATEGORY (SDC) = D
RISK CATEGORY = II
SEISMIC SITE CLASS = D
IMPORTANCE FACTOR $I_e = 1.0$
MAFFED MCE $S_s = 1.28$; $S_d = 0.50$
DESIGN ACCELERATION $S_{ds} = 0.85$; $S_{d1} = 0.50$
SEISMIC RESISTING SYSTEM: WOOD PANEL BEARING SHEAR WALL, $R = 6.5$
SEISMIC BASE SHEAR $V_s = 13.1$ K

3. LATERAL LOADS ARE TRANSFERRED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE SHEAR WALLS. FORCES ARE BASED ON THE TRIBUTARY AREA FOR EACH SHEAR WALL AND ARE CARRIED BY THE SHEAR WALLS TO THE FOUNDATION.

4. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.

5. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.

6. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THEIR WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.

7. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED; SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. WHERE INFORMATION ON THE DRAWINGS IS IN CONFLICT WITH THE SPECIFICATIONS, THE MORE STRINGENT SHALL APPLY, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. DO NOT SCALE THE DRAWINGS.

8. ALL STRUCTURAL SYSTEMS WHICH ARE COMPOSED OF FIELD ERECTED COMPONENTS SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.

9. SHOP DRAWINGS FOR GLUED LAMINATED MEMBERS, CONNECTOR PLATE WOOD ROOF TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.

10. SHOP DRAWING REVIEW: DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO. A MINIMUM OF TWO WEEKS SHALL BE ALLOWED FOR REVIEW.

11. SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.

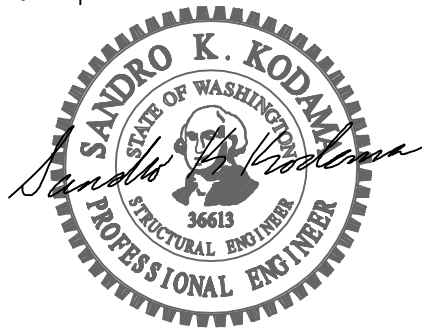
12. DEFERRED SUBMITTALS OF DESIGN BUILD COMPONENTS SHALL BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE AND SHALL INCLUDE DESIGN CALCULATIONS WITH THE ENGINEER'S STAMP.

THE FOLLOWING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT: TRUSSES.

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6/29/2020

GENERAL STRUCTURAL NOTES

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| Project number | 20196.01 |
| Date | 6/29/2020 |
| Project Manager | SKK |
| Drawn by | SC |
| Checked by | TON |

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GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

27. PREFABRICATED CONNECTOR PLATE WOOD ROOF TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH IBC SECTION 2309.4 AND ANSI/TPI 1-2014 "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION" FOR THE SPANS AND CONDITIONS SHOWN ON THE PLANS. TRUSSES SHALL BE HANDLED, INSTALLED, AND BRACED PER "HIB 91" PER THE TRUSS PLATE INSTITUTE. LOADING SHALL BE AS FOLLOWS:

| | |
|-----------------------------|--------------------------------|
| TOP CHORD SNOW LOAD | 25 PSF |
| TOP CHORD DEAD LOAD | 10 PSF |
| BOTTOM CHORD LIVE LOAD | 10 PSF (NOT INCLUDED IN TOTAL) |
| BOTTOM CHORD DEAD LOAD | 5 PSF |
| TOTAL LOAD | 40 PSF |
| NET WIND UPLIFT (TOP CHORD) | 10 PSF |

THE LOADS ABOVE SHALL BE INCREASED TO THE FOLLOWING IF THE TRUSSES MEET THE DESCRIPTION OF AN "UNINHABITABLE ATTIC WITH LIMITED STORAGE" AS DEFINED IN FOOTNOTE J OF IBC TABLE 1607.1:

| | |
|------------------------|---------------------------|
| BOTTOM CHORD LIVE LOAD | 20 PSF - INCLUDE IN TOTAL |
| BOTTOM CHORD DEAD LOAD | 10 PSF |

SNOW LOAD DUE TO DRIFTING AND UNBALANCED LOADS SHALL BE INCLUDED PER THE IBC. TOP CHORDS SHALL BE DF LUMBER. UTILIZE A MINIMUM CREEP FACTOR OF 2.0 FOR DEAD AND SUSTAINED LIVE LOADS IN DETERMINING THE TRUSS DEFLECTIONS. MAXIMUM TOTAL DEFLECTION SHALL BE LESS THAN OR EQUAL TO L/240 OF THE TOTAL SPAN AND MAXIMUM LIVE LOAD DEFLECTION SHALL BE LESS THAN OR EQUAL TO L/360 OF THE TOTAL SPAN. PROVIDE ADEQUATE PLIES AND/OR METAL BRACKETS TO ADEQUATELY DISTRIBUTE THE BEARING PRESSURE AT THE ENDS OF THE GIRDER TRUSSES TO THE TOP PLATES OF THE BEARING WALLS SUCH THAT THE BEARING PRESSURE DOES NOT EXCEED 405 PSI. PROVIDE ADDITIONAL TRUSSES (AS REQUIRED) TO CARRY ALL CONCENTRATED LOADS AND MECHANICAL UNITS.

WOOD TRUSSES SHALL UTILIZE I.C.C. OR IAPMO UES APPROVED CONNECTOR PLATES. SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. SUBMITTED DOCUMENTS SHALL BEAR THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER. PROVIDE FOR SHAPES, BEARING POINTS, INTERSECTIONS, HIPS, VALLEYS, ETC., SHOWN ON THE DRAWINGS. EXACT COMPOSITION OF SPECIAL HIP, VALLEY, AND INTERSECTION AREAS (USE OF GIRDER TRUSSES, JACK TRUSSES, STEP-DOWN TRUSSES, ETC.) SHALL BE DETERMINED BY THE MANUFACTURER UNLESS SPECIFICALLY INDICATED ON THE PLANS. PROVIDE ALL TRUSS TO TRUSS AND TRUSS TO GIRDER TRUSS CONNECTION DETAILS AND REQUIRED CONNECTION MATERIALS. PROVIDE FOR ALL TEMPORARY AND PERMANENT TRUSS BRACING AND BRIDGING.

28. TRUSS SUPPLIERS NOTE: THE TRUSS CONFIGURATIONS, INCLUDING DEPTHS AND MEMBER SIZES SHOWN ON THE DRAWINGS INDICATE THE DESIRED TRUSS CONFIGURATION AND ARE TO BE COMPLIED WITH WHEREVER POSSIBLE. IF A TRUSS MANUFACTURER IS UNABLE TO MEET THE LOAD REQUIREMENTS SPECIFIED WITH THE TRUSS CONFIGURATION INDICATED, THE MANUFACTURER IS TO SUBMIT WRITTEN NOTICE TO THAT EFFECT TO THE ARCHITECT PRIOR TO SUBMITTING A COST PROPOSAL OR BID.

IF A DIFFERENT SYSTEM IS PROPOSED THAT REQUIRES REVISIONS TO PRESENT STRUCTURAL FRAMING OR DETAILS, SUCH SYSTEM SHALL BE CONSIDERED SUBJECT TO THE APPROVAL OF THE OWNER, ARCHITECT, AND STRUCTURAL ENGINEER.

IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND TRUSS MANUFACTURER TO VERIFY THE WEIGHT AND LOCATIONS OF ALL MECHANICAL EQUIPMENT PRIOR TO SUBMITTING SHOP DRAWINGS. IT SHALL BE NOTED IN THE TRUSS MANUFACTURER'S BID WHETHER OR NOT AN ALLOWANCE HAS BEEN MADE FOR MECHANICAL UNITS.

TRUSS SHOP DRAWINGS WILL NOT BE REVIEWED WITHOUT CALCULATIONS BEARING THE STAMP AND SIGNATURE OF A STATE OF WASHINGTON REGISTERED PROFESSIONAL ENGINEER.

29. WOOD SHEATHING SHALL BE APA RATED, EXTERIOR BLUE; EXPOSURE I, IN CONFORMANCE WITH THE REQUIREMENTS FOR THEIR TYPE IN DOC PS-1 OR PS-2. SEE PLANS FOR THICKNESS, PANEL IDENTIFICATION INDEX AND NAILING REQUIREMENTS. UNLESS OTHERWISE NOTED ON THE PLANS, ROOF AND FLOOR SHEATHING SHALL BE LAID UP WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. PROVIDE APPROVED PLYWOOD EDGE CLIPS CENTERED BETWEEN JOISTS/TRUSSES AT UNBLOCKED ROOF SHEATHING EDGES. ALL FLOOR SHEATHING EDGES SHALL HAVE APPROVED TONGUE-AND-GROOVE JOINTS OR SHALL BE SUPPORTED WITH SOLID BLOCKING. ALLOW 1/8" SPACING AT ALL PANEL EDGES AND ENDS OF FLOOR AND ROOF SHEATHING. TOENAIL BLOCKING TO SUPPORTS WITH (2) 10d-F NAILS AT EACH END, UNLESS OTHERWISE NOTED. AT BLOCKED FLOOR AND ROOF DIAPHRAGMS PROVIDE FLAT 2X BLOCKING AT ALL UNFRAMED PANEL EDGES AND NAIL WITH EDGE NAILING SPACED PER PLANS. WHERE NOT NOTED OTHERWISE, NAIL PANEL EDGES WITH 8d NAILS @ 6" O.C. EDGES, 12" O.C. IN THE FIELD.

30. ALL WOOD EXPOSED TO WEATHER, OR BEARING ON UNPROTECTED CONCRETE BELOW GRADE, OR BEARING ON UNPROTECTED CONCRETE LESS THAN 8" FROM EXPOSED EARTH SHALL BE PRESSURE-TREATED, U.O.N. PRESSURE TREATMENT SHALL BE WITH AN APPROVED PRESERVATIVE AND BRANDED WITH A QUALITY CONTROL AGENCY MARK BY THE AMERICAN WOOD PRESERVERS BUREAU OR EQUAL. ALL METAL HARDWARE IN CONTACT WITH TREATED WOOD SHALL BE PROTECTED WITH A G105 GALVANIZED COATING (ZMAX) OR BETTER. ALL NAILS IN TREATED WOOD SHALL BE HOT-DIP GALVANIZED OR BETTER. PROVIDE 2 LAYERS OF 30# ASPHALT IMPREGNATED BUILDING PAPER BETWEEN NON-PRESSURE-TREATED LEDGERS, BLOCKING, ETC., AND CONCRETE.

31. TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY, AS SPECIFIED IN THEIR CATALOG NO. C-G-2019. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE SUBSTITUTED, PROVIDED THEY HAVE I.C.C. OR IAPMO UES APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. CONNECTORS SHALL BE SIZED TO MATCH THE SIZE OF THE FRAMING MEMBERS BEING CONNECTED. PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. WHERE CONNECTOR STRAPS CONNECT TWO MEMBERS, PLACE ONE-HALF OF THE NAILS OR BOLTS IN EACH MEMBER. ALL BOLTS IN WOOD MEMBERS SHALL CONFORM TO ASTM A307. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD, UNLESS NOTED OTHERWISE. ALL NAILS SHALL BE COMMON. ALL SHIMS SHALL BE SEASONED AND DRIED AND THE SAME GRADE (MINIMUM) AS MEMBERS CONNECTED. ALL BOLTS TIGHTENED TO SNUG TIGHT.

32. WOOD FASTENERS:

A. NAIL SIZES SPECIFIED ON DRAWINGS ARE BASED ON THE FOLLOWING SPECIFICATIONS:

| DRAWING ID | NAIL NAME | NAIL DIAMETER | NAIL LENGTH |
|------------|------------|---------------|-------------|
| "6d" | 6d Common | 0.113" | 2" |
| "8d Box" | 8d Box | 0.113" | 2-1/2" |
| "8d" | 8d Common | 0.131" | 2-1/2" |
| "10d-F" | 10d Framer | 0.131" | 3" |
| "10d" | 10d Shear | 0.148" | 2-1/4" |
| "16d" | 16d Sinker | 0.148" | 3-1/4" |

IF CONTRACTOR PROPOSES THE USE OF ALTERNATE NAILS, THEY SHALL SUBMIT NAIL SPECIFICATIONS TO THE STRUCTURAL ENGINEER (PRIOR TO CONSTRUCTION) FOR REVIEW AND APPROVAL.

B. NAILS - SHEATHING FASTENERS TO FRAMING SHALL BE DRIVEN FLUSH TO FACE OF SHEATHING WITH NO COUNTERSINKING PERMITTED.

C. SCREWS SHALL BE WOOD SCREWS OF THE DIAMETER AND LENGTH NOTED ON THE DRAWINGS. SDS FASTENERS ARE SIMPSON STRONG DRIVE SCREWS.

D. HOT DIPPED GALVANIZED NAILS, BOLTS AND METAL PLATES - ALL NAILS, BOLTS AND METAL PLATES IN CONTACT WITH PRESSURE TREATED (INCLUDING FIRE-RETARDANT TREATED) LUMBER SHALL BE HOT DIPPED GALVANIZED.

33. WOOD FRAMING NOTES: THE FOLLOWING APPLY UNLESS OTHERWISE SHOWN ON THE PLANS:

A. ALL WOOD FRAMING DETAILS NOT SHOWN OTHERWISE SHALL BE CONSTRUCTED TO THE MINIMUM STANDARDS OF THE IBC. MINIMUM NAILING, UNLESS OTHERWISE NOTED, SHALL CONFORM TO IBC TABLE 2304.10.1. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH MECHANICAL AND ARCHITECTURAL DRAWINGS. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. TIGHTEN BOLTS AND LAG SCREWS SNUGLY AGAINST WOOD FRAMING AFTER WOOD HAS REACHED SPECIFIED MOISTURE CONTENT.

B. WALL FRAMING: ALL BEARING AND SHEAR WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE 2 x 4 STUDS @ 16" O.C. AT INTERIOR WALLS AND 2 x 6 @ 16" O.C. AT EXTERIOR WALLS. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL BEARING AND SHEAR WALLS AND AT EACH SIDE OF ALL OPENINGS. SOLID BLOCKING FOR WOOD COLUMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORTS BELOW.

ALL BEARING STUD WALLS SHALL HAVE THEIR LOWER WOOD PLATES ATTACHED TO WOOD FRAMING BELOW WITH 16d NAILS AT 8" O.C. STAGGERED OR BOLTED TO CONCRETE WITH 5/8" DIAMETER ANCHOR BOLTS WITH 3"x3"x1/4" PLATE WASHERS @ 4'-0" O.C., UNLESS INDICATED OTHERWISE. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE NAILED TO EACH OTHER WITH 10d-F NAILS @ 8" O.C. STAGGERED. REFER TO THE PLANS AND SHEAR WALL SCHEDULE FOR REQUIRED SHEATHING AND NAILING. WHEN NOT OTHERWISE NOTED, PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES AND GYPSUM SHEATHING ON EXTERIOR SURFACES ATTACHED TO ALL STUDS, TOP AND BOTTOM PLATES AND BLOCKING WITH SCREWS AT 8" O.C. USE 1-1/4 " W #6 SCREWS FOR 1/2" GWB AND 5/8" GWB WHERE OCCURS. USE 1-1/4" W #6 GALVANIZED SCREWS FOR 1/2" GWB AND 5/8" EXTERIOR GYPSUM SHEATHING, WHERE OCCURS. VERIFY THE FIRE ASSEMBLY REQUIREMENTS WHERE APPLICABLE WITH THE ARCHITECT.

C. FLOOR AND ROOF FRAMING: PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS THAT EXTEND OVER MORE THAN HALF THE JOIST LENGTH AND AROUND ALL OPENINGS IN FLOORS OR ROOFS UNLESS OTHERWISE NOTED. PROVIDE SOLID BLOCKING AT ALL BEARING POINTS. NAIL ALL MULTI-JOIST BEAMS TOGETHER WITH 10d-F NAILS @ 8" O.C. STAGGERED UNLESS OTHERWISE NOTED.

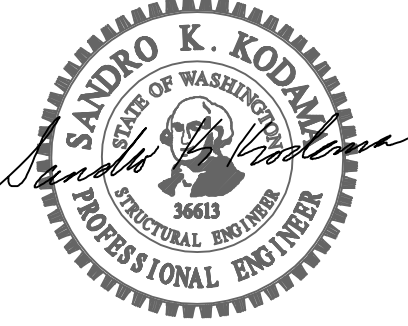
D. POSITIVE CONNECTIONS: PROVIDE THE FOLLOWING SIMPSON CONNECTORS AT TYPICAL FRAMING UNLESS OTHERWISE NOTED ON PLAN OR DETAIL. PROVIDE CGQ/ECCQ CAPS AND PBS BASES AT POSTS. PROVIDE BC BASE WHERE POST BEARS ON WOOD FRAMING BELOW. PROVIDE LUG SERIES HANGERS FOR 2X FLOOR AND ROOF JOISTS. CONNECTORS SHALL BE SIZED TO MATCH THE SIZE OF THE FRAMING MEMBERS BEING CONNECTED.

| ABBREVIATIONS | | | |
|---------------|-------------------------------------|----------------|----------------------------|
| @ | At | L | Angle |
| d | Penny (Nails) | LB. | Pound |
| Ø | Diameter | LL | Live Load |
| ° | Degrees | LLH | Long Leg Horizontal |
| ...# | Pounds | LLV | Long Leg Vertical |
| #... | Number | LONGIT. | Longitudinal |
| | | LT. WT. | Lightweight |
| (A) | Above | | |
| A.B. | Anchor Bolt | MAX. | Maximum |
| ADD'L | Additional | MECH. | Mechanical |
| ALT. | Alternate | MEZZ. | Mezzanine |
| APPROX. | Approximate | MF | Moment Frame |
| ARCH. | Architect | MFR. | Manufacturer |
| | | MIN. | Minimum |
| (B) | Below | MISC. | Miscellaneous |
| B/ | Bottom of | MK. | Mark |
| BF | Braced Frame | | |
| BLKG. | Blocking | (N) | New |
| BLDG. | Building | N. | North |
| BM. | Beam | N.S. | Near Side |
| BOT. | Bottom | NOM. | Nominal |
| BRG. | Bearing | NTS | Not to Scale |
| BTWN. | Between | | |
| | | O.C. | On Center |
| CL | Centerline | O.D. | Outside Diameter |
| C | Camber | O.F. | Outside Face |
| CIP | Cast In Place | O.H. | Overhang |
| C.J. | Construction Joint or Control Joint | OPNG. | Opening |
| CJP | Complete Joint Penetration | OPP. | Opposite |
| CLG. | Ceiling | | |
| CLR. | Clear | PAF | Powder Actuated Fastener |
| CMU | Concrete Masonry Unit | PC | Precast |
| COL. | Column | PERM. | Permanent |
| CONG. | Concrete | PERP. | Perpendicular |
| CONN. | Connections | PJP | Partial Joint Penetration |
| CONST. | Construction | PL or P | Plate |
| CONT. | Continuous | PLF | Pounds per linear Foot |
| CSK. | Countersink | PLYWD | Plywood |
| | | PREFAB. | Prefabricated |
| DBA | Deformed Bar Anchor | PSF | Pounds per Square Foot |
| DBL. | Double | PSI | Pounds per Square Inch |
| DEG. | Degree | P.T. or PT | Post-Tensioning |
| DF | Doug Fir-Larch | P/T | Pressure-Treated |
| DIA. | Diameter | | |
| DIAG. | Diagonal | RAD. | Radius |
| DIAPH. | Diaphragm | REF. | Reference |
| DIM. | Dimension | REINF. | Reinforce or Reinforcement |
| DN. | Down | REQD. | Required |
| DO | Ditto | REV. | Revise |
| DTL. | Detail | R.O. | Rough Opening |
| DWG. | Drawing | | |
| | | S. | South |
| (E) | Existing | SCH. or SCHED. | Schedule |
| E. | East | SECT. | Section |
| E.A. | Each | SHT. | Sheet |
| E.F. | Each Face | SIM. | Similar |
| EL. | Elevation | SOG | Slab On Grade |
| ELEV. | Elevator | SPEC. | Specification |
| EMBED. | Embedment | SQ. | Square |
| ENGR. | Engineer | SQ. FT. | Square Feet |
| EQ. | Equal | SQ. IN. | Square Inch(es) |
| E.W. | Each Way | SFF | Spruce-Fine-Fir |
| EXP. | Expansion | S.S. | Stainless Steel |
| EXT. | Exterior | STD. | Standard |
| | | STIFF. | Stiffener |
| FDN. | Foundation | STL. | Steel |
| FIN. | Finish | STR. | Structural |
| FLR. | Floor | SUB. | Substitute |
| FRP | Fiber Reinforced Polymer | SYM. | Symmetrical |
| F.S. | Far Side | | |
| FT. | Foot or Feet | T/ | Top of |
| FTG. | Footing | T&B | Top and Bottom |
| | | T&G | Tongue & Groove |
| GA. | Gauge | TEMP. | Temporary |
| GALV. | Galvanized | THRU | Through |
| GL | Glove Laminated | T.O.C. | Top of Concrete |
| GWB | Gypsum Wall Board | T.O.S. | Top of Steel |
| | | T.O.M. | Top of Wall |
| Hdg | Hot Dipped Galvanized | TRANS. | Transverse |
| HF | Hem Fir | TS | Tube Steel |
| HGR. | Hanger | TYP. | Typical |
| HORIZ. | Horizontal | | |
| HSS | Hollow Structural Section | U.O.N. | Unless Otherwise Noted |
| HT. | Height | | |
| | | VERT. | Vertical |
| I.D. | Inside Diameter | VIF | Verify in Field |
| I.F. | Inside Face | | |
| IN. | Inch | W. | West |
| INFO. | Information | W/ or w/ | With |
| INT. | Interior | W.H.S. | Welded Headed Stud |
| | | W/O | Without |
| JP | Joint | WP | Work Point |
| | | W.T.S. | Welded Threaded Stud |
| K | Kips | WVF | Welded Wire Fabric |
| KSF | Kips per Square Foot | | |
| KSI | Kips per Square Inch | X SECT. | Cross Section |
| | | X-STR | Extra Strong |
| | | XX-STR | Double Extra Strong |

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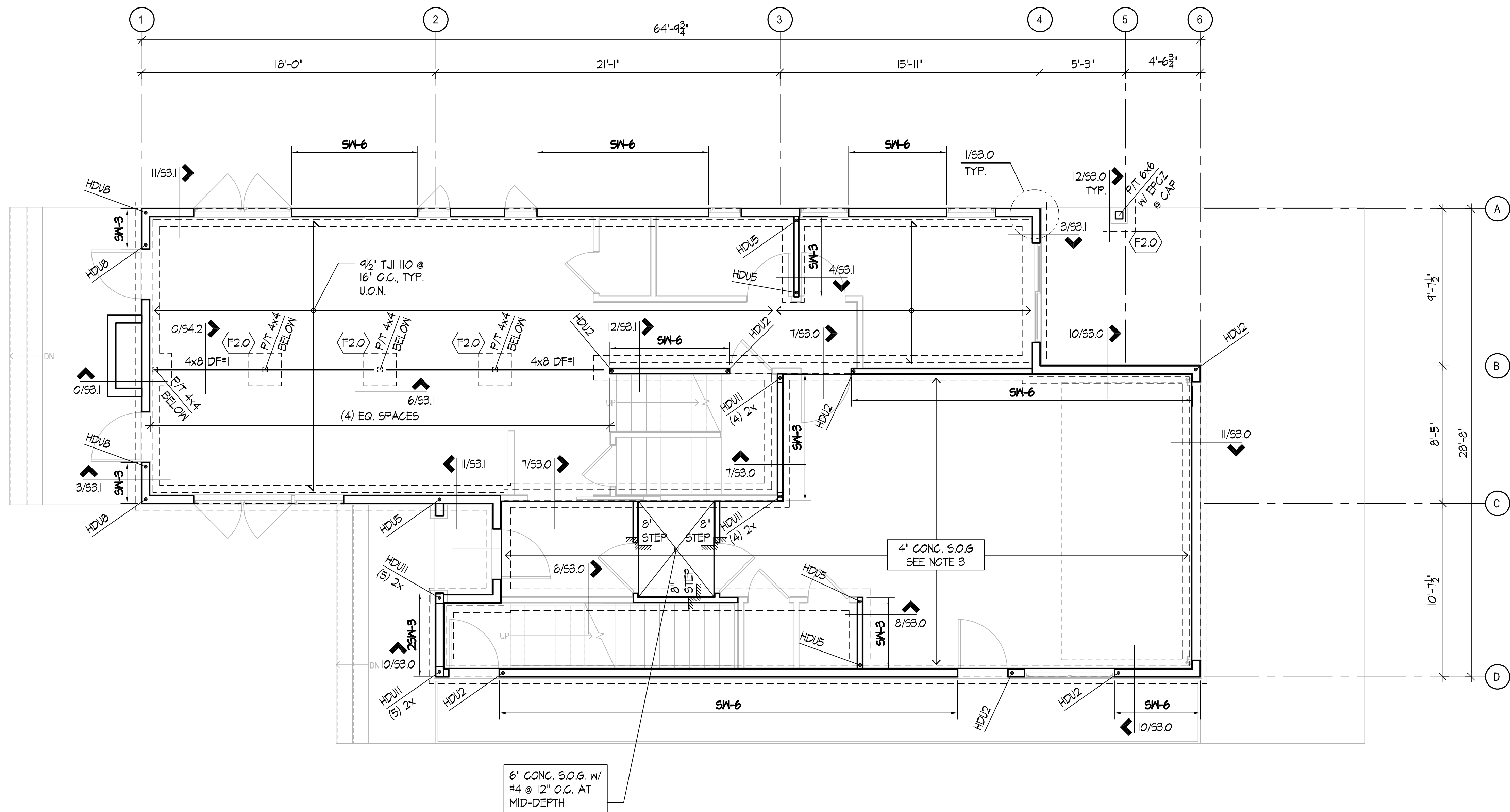
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GENERAL STRUCTURAL NOTES

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Scale AS NOTED

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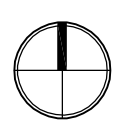
FOUNDATION / MAIN FLOOR FRAMING PLAN NOTES:

- ALL DIMENSIONS AND ELEVATIONS ON THE STRUCTURAL PLANS ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED BY THE CONTRACTOR WITH THE ARCHITECTURAL DRAWINGS BEFORE CONSTRUCTION BEGINS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- SEE SHEETS S1.0 AND S1.1 FOR GENERAL STRUCTURAL NOTES AND ABBREVIATIONS. SEE SHEET S3.0 FOR TYPICAL CONCRETE AND FOUNDATION DETAILS. SEE SHEET S4.0 FOR TYPICAL WOOD DETAILS.
- SLAB-ON-GRADE SHALL BE 4" THICK CONCRETE REINFORCED WITH #4 @ 16" O.C. EACH WAY AT MID-DEPTH, U.O.N. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION REGARDING SUB-GRADE MOISTURE BARRIER AND ELEVATIONS, ETC. THE SLAB-ON-GRADE IS A STRUCTURAL DIAPHRAGM AND PART OF THE LATERAL FORCE RESISTING SYSTEM.
- FOR SLAB-ON-GRADE JOINTS, SEE DETAIL 2/S3.0.
- ALL WOOD BEARING ON UNPROTECTED CONCRETE EXPOSED TO WEATHER, OR WITHIN 8" OF FINISHED GRADE SHALL BE PRESSURE-TREATED, U.O.N.
- FOR SILL PLATE ANCHOR BOLT LAYOUT TO CONCRETE FOUNDATION WALLS AND SLABS, SEE DETAIL 1/S4.0.
- ALL BEARING AND SHEAR WALLS SHALL BE 2x4 @ 16" O.C. INTERIOR AND 2x6 @ 16" O.C. EXTERIOR U.O.N.
- POSTS INDICATED ARE AT THIS LEVEL. ALL POSTS NOT SPECIFIED SHALL BE (2) 2x U.O.N. SOLID SAWN MEMBERS OF EQUIVALENT SIZE MAY BE SUBSTITUTED FOR BUILT-UP MEMBERS (SUCH AS A 4x6 FOR (3) 2x4).

- TYPICAL FLOOR FRAMING CONSISTS OF 23/32" APA RATED T&G SHEATHING (INDEX 48/24), LAID FACE GRAIN PERPENDICULAR OVER 9'-1/2" TJI 110 JOISTS AT 16" O.C. HANG TJI JOISTS WITH ITS TOP FLANGE HANGERS TYPICAL AT FLUSH BEAMS, U.O.N.
- NAIL FLOOR SHEATHING TO FRAMING WITH 8d NAILS (0.131" x 2.5" LONG) AT 6" O.C. AT ALL PANELS EDGES AND 8d NAILS AT 12" O.C. AT INTERMEDIATE FRAMING MEMBERS (UNBLOCKED). SEE DETAIL 6/S4.0.
- Fx-x INDICATES SPREAD FOOTING TYPE, SEE 12/S3.0 FOR SCHEDULE.
- SW-x INDICATES SHEAR WALL AT THIS LEVEL. SEE SHEAR WALL SCHEDULE 8/S4.0 FOR SHEATHING, BLOCKING, NAILING, AND ANCHOR BOLT REQUIREMENTS. ALL EXTERIOR WALLS SHALL BE SHEATHED PER SW-6 CRITERIA U.O.N.
- HDUx INDICATES HOLDOWN TO CONCRETE FOUNDATION WALLS OR FOOTINGS. SEE 12/S4.0 FOR HOLDOWN DETAIL. USE MIN. (2) 2x POST U.O.N.

LEGEND:

- Fxx** INDICATES SPREAD FOOTING. SEE 12/S3.0 FOR SCHEDULE
- INDICATES FOOTING
- INDICATES FOUNDATION WALL, WOOD BEARING WALL OR SHEAR WALL
- INDICATES WOOD BEARING OR SHEAR WALL AT THIS LEVEL. SEE PLAN NOTES 1 & 12
- INDICATES NON-BEARING/ NON-SHEAR WALL AT THIS LEVEL. SEE 1 & 2/S4.1 FOR CONNECTION DETAILS



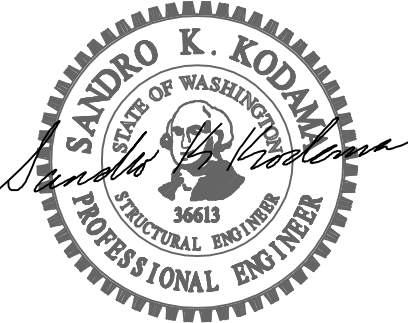
FOUNDATION / MAIN FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

**SUNDBERG
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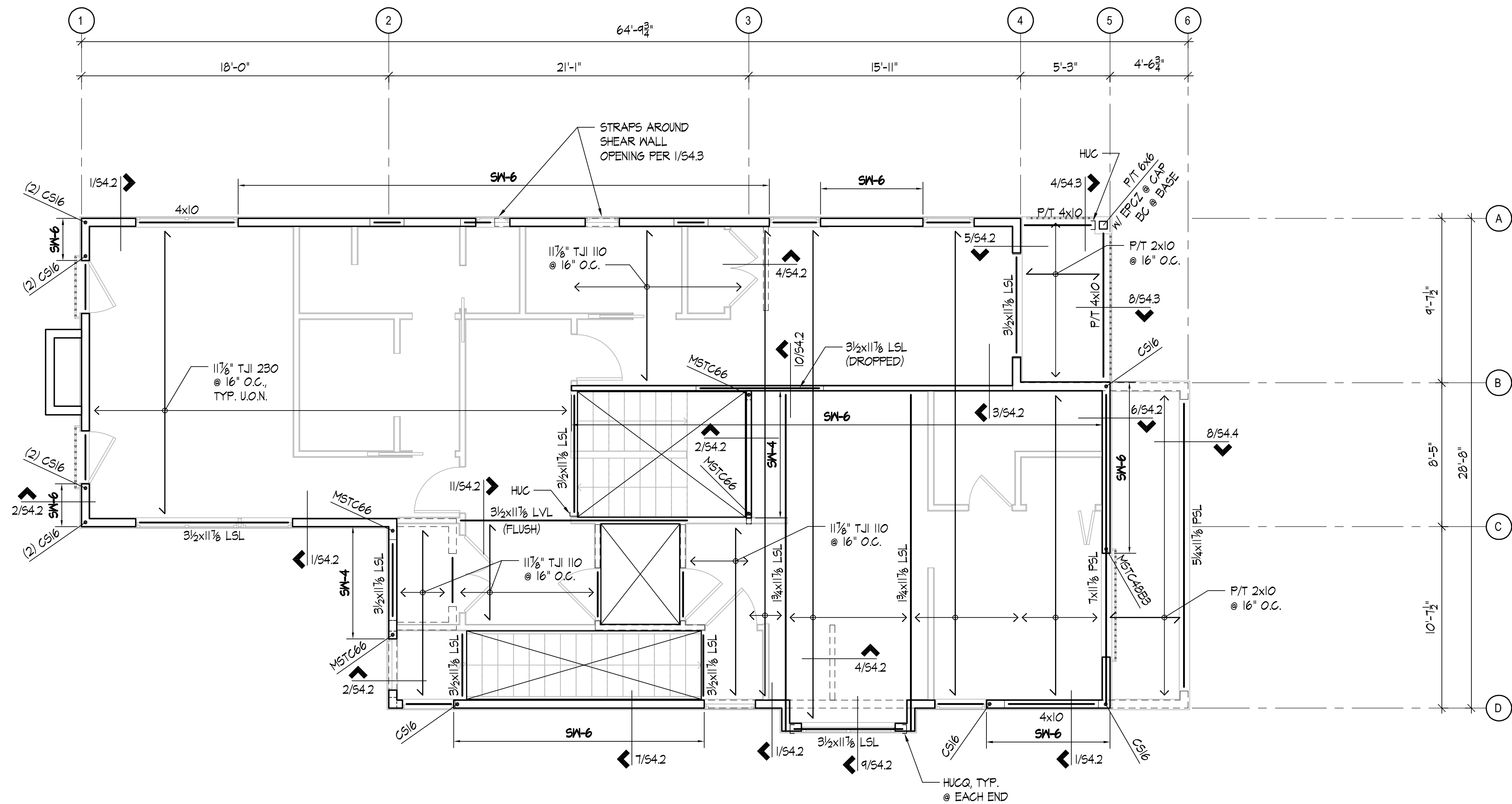
**FOUNDATION / MAIN FLOOR
FRAMING PLAN**

S2.0

Scale AS NOTED

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| Project number | 20196.01 | TON |
| Date | 6/29/2020 | SC |
| Project Manager | SKK | SC |
| Drawn by | | |
| Checked by | | |

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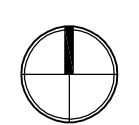
UPPER FLOOR FRAMING PLAN NOTES:

- ALL DIMENSIONS AND ELEVATIONS ON THE STRUCTURAL PLANS ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED BY THE CONTRACTOR WITH THE ARCHITECTURAL DRAWINGS BEFORE CONSTRUCTION BEGINS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- SEE SHEETS S1.0 AND S1.1 FOR GENERAL STRUCTURAL NOTES AND ABBREVIATIONS. SEE SHEETS S4.0, S4.1 AND S4.2 FOR TYPICAL WOOD DETAILS.
- TYPICAL FLOOR FRAMING CONSISTS OF 23/32" APA RATED T&G SHEATHING (INDEX 48/24), LAID FACE GRAIN PERPENDICULAR OVER 11-7/8" TJI 230 JOISTS AT 16" O.C. HANG TJI JOISTS WITH ITS TOP FLANGE HANGERS TYPICAL AT FLUSH BEAMS, U.O.N.
- NAIL FLOOR SHEATHING TO FRAMING WITH 8d NAILS (0.131" x 2.5" LONG) AT 6" O.C. AT ALL PANELS EDGES AND 8d NAILS AT 12" O.C. AT INTERMEDIATE FRAMING MEMBERS (UNBLOCKED). SEE DETAIL 6/54.0.
- ALL BEARINGS AND SHEAR WALLS SHALL BE 2x4 @ 16" O.C. INTERIOR AND 2x6 @ 16" O.C. EXTERIOR U.O.N.
- POSTS INDICATED ARE AT THIS LEVEL. ALL POSTS NOT SPECIFIED SHALL BE (2) 2x U.O.N. SOLID SAWN MEMBERS OF EQUIVALENT SIZE MAY BE SUBSTITUTED FOR BUILT-UP MEMBERS (SUCH AS A 4x6 FOR (3) 2x4).
- PROVIDE SOLID OR BUILT-UP WOOD POSTS BENEATH THE ENDS OF ALL FLOOR BEAMS AND ALL POSTS ABOVE FOR FULL BEARING. PROVIDE BLK6. AT JOISTS PER DETAIL 1/54.1.
- ALL HEADERS NOT SHOWN ON PLAN SHALL BE (2) 2x8 FOR EXTERIOR BEARING WALLS AND (2) 2x8 FOR INTERIOR BEARING WALLS. SEE 10/54.1 FOR HEADER DETAIL.

- FOR TOP PLATE SPLICE SEE DETAIL 6/54.1.
- ALIGN A JOIST OR JOIST BLOCKING OVER THE FULL LENGTH OF ALL BEARINGS/SHEAR WALLS. SEE 8/54.0 FOR SPECIAL SHEAR WALL BLOCKING REQUIREMENTS.
- SW-x INDICATES SHEAR WALL AT THIS LEVEL. SEE SHEAR WALL SCHEDULE 8/54.0 FOR SHEATHING, BLOCKING, NAILING, AND ANCHOR BOLT REQUIREMENTS. ALL EXTERIOR WALLS SHALL BE SHEATHED PER SW-6 CRITERIA U.O.N.
- CSI6 INDICATES HOLDOWN STRAP TO FRAMING BELOW WALL. SEE 10/54.0 FOR STRAP HOLDOWN DETAIL AT FLOOR-TO-FLOOR AND BEAM SUPPORTING SHEAR WALL END. USE MIN. (2) 2x POST U.O.N.

LEGEND:

- INDICATES FRAMING DIRECTION
- INDICATES EXTENT OF FRAMING
- SW-x INDICATES SHEAR WALL TYPE AT THIS LEVEL. SEE PLAN NOTE 11
- INDICATES WOOD BEARING OR SHEAR WALL AT THIS LEVEL. SEE PLAN NOTES 5 & 11
- INDICATES WOOD BEARING WALL OR SHEAR WALL BELOW.
- INDICATES NON-BEARING/ NON-SHEAR WALL AT THIS LEVEL. SEE 1 & 2/54.1 FOR CONNECTION DETAILS
- INDICATES HEADER MEMBER. SEE PLAN NOTE 8
- INDICATES MULTIPLE STUD POST AT THIS LEVEL. SEE PLAN NOTE 6
- INDICATES HOLDOWN TYPE AT THIS LEVEL. SEE PLAN NOTE 12



UPPER FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

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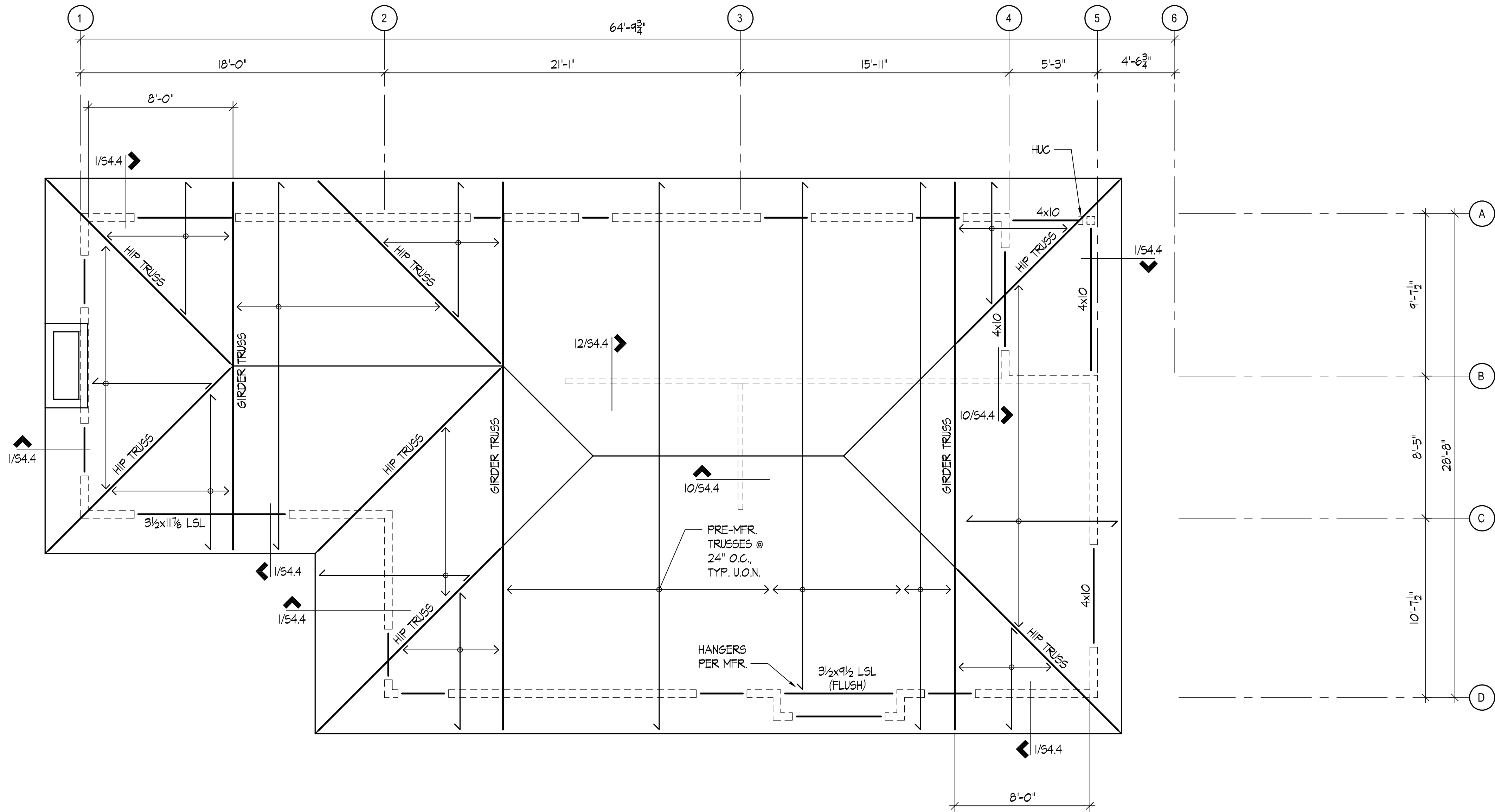
UPPER FLOOR FRAMING PLAN

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| Project number | 20196.01 | AS NOTED |
| Date | 6/29/2020 | |
| Project Manager | SKK | |
| Drawn by | SC | |
| Checked by | TON | |

S2.1

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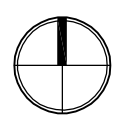
ROOF FRAMING PLAN NOTES:

- ALL DIMENSIONS AND ELEVATIONS ON THE STRUCTURAL PLANS ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED BY THE CONTRACTOR WITH THE ARCHITECTURAL DRAWINGS BEFORE CONSTRUCTION BEGINS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- SEE SHEETS S1.0 AND S1.1 FOR GENERAL STRUCTURAL NOTES AND ABBREVIATIONS. SEE SHEETS S4.0, S4.1 AND S4.3 FOR TYPICAL WOOD DETAILS.
- TYPICAL ROOF FRAMING CONSISTS OF 5/8" APA RATED SHEATHING (INDEX 32/16), LAID FACE GRAIN PERPENDICULAR OVER PRE-FABRICATED ROOF TRUSSES @ 24" O.C., U.O.N. (SEE THE STRUCTURAL GENERAL NOTES FOR TRUSS DESIGN CRITERIA).
- NAIL ROOF SHEATHING TO FRAMING WITH 8d NAILS (0.131" ϕ x 2.5" LONG) AT 6" O.C. AT ALL PANELS EDGES AND 8d NAILS AT 12" O.C. AT INTERMEDIATE FRAMING MEMBERS (UNBLOCKED). SEE DETAIL 6/S4.0.
- PROVIDE SOLID BLOCKING BETWEEN EACH ROOF TRUSS AT SUPPORTS. PROVIDE AN HI CLIP AT EVERY MEMBER TO TOP PLATE.
- ALL HEADERS NOT SHOWN ON PLAN SHALL BE (2) 2x8 FOR EXTERIOR BEARING WALLS AND (2) 2x8 FOR INTERIOR BEARING WALLS. SEE 10/S4.1 FOR HEADER DETAIL.
- PROVIDE SOLID OR BUILT-UP WOOD POSTS BENEATH THE ENDS OF ALL ROOF BEAMS FOR FULL BEARING.
- FOR TOP PLATE SPLICE SEE DETAIL 6/S4.1.

- ATTACH NON-BEARING INTERIOR WALLS TO BOTTOM OF TRUSSES WITH STC CLIPS AT 48" O.C. INSTALL IN ACCORDANCE WITH MFR. RECOMMENDATIONS. SEE DETAIL 9/S4.4.

LEGEND:

- INDICATES FRAMING DIRECTION
- INDICATES EXTENT OF FRAMING
- INDICATES WOOD BEARING WALL OR SHEAR WALL BELOW
- INDICATES HEADER MEMBER. SEE PLAN NOTE 6



ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

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ROOF FRAMING PLAN

| Project number | Date | Project Manager | Drawn by | Checked by | AS NOTED |
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S2.2

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| | | | | | | | | | | | |
| DETAIL | SCALE: 1"=1'-0" | TYPICAL OFFSET OF ADJACENT FOOTINGS | SCALE: NONE | 2 | TYPICAL FOUNDATION - I-JOIST PARALLEL w/ HANGER | SCALE: NONE | 3 | TYPICAL INTERIOR FOUNDATION - I-JOIST PARALLEL | SCALE: NONE | 4 | |
| | | | | | | | | | | | |
| DETAIL | SCALE: 1"=1'-0" | 5 | TYPICAL CRAWL SPACE SPREAD FOOTING (I-JOIST ON BEAM) | SCALE: NONE | 6 | | | | | | |
| | | | | | | | | | | | |
| DETAIL | SCALE: 1"=1'-0" | 9 | TYPICAL CRAWL SPACE POST AT FOUNDATION WALL (I-JOIST ON BEAM) EXTERIOR GRADE HIGH | SCALE: NONE | 10 | TYPICAL FOUNDATION - I-JOIST PERPENDICULAR w/ HANGER | SCALE: NONE | 11 | TYPICAL INTERIOR FOUNDATION - I-JOIST PERPENDICULAR | SCALE: NONE | 12 |

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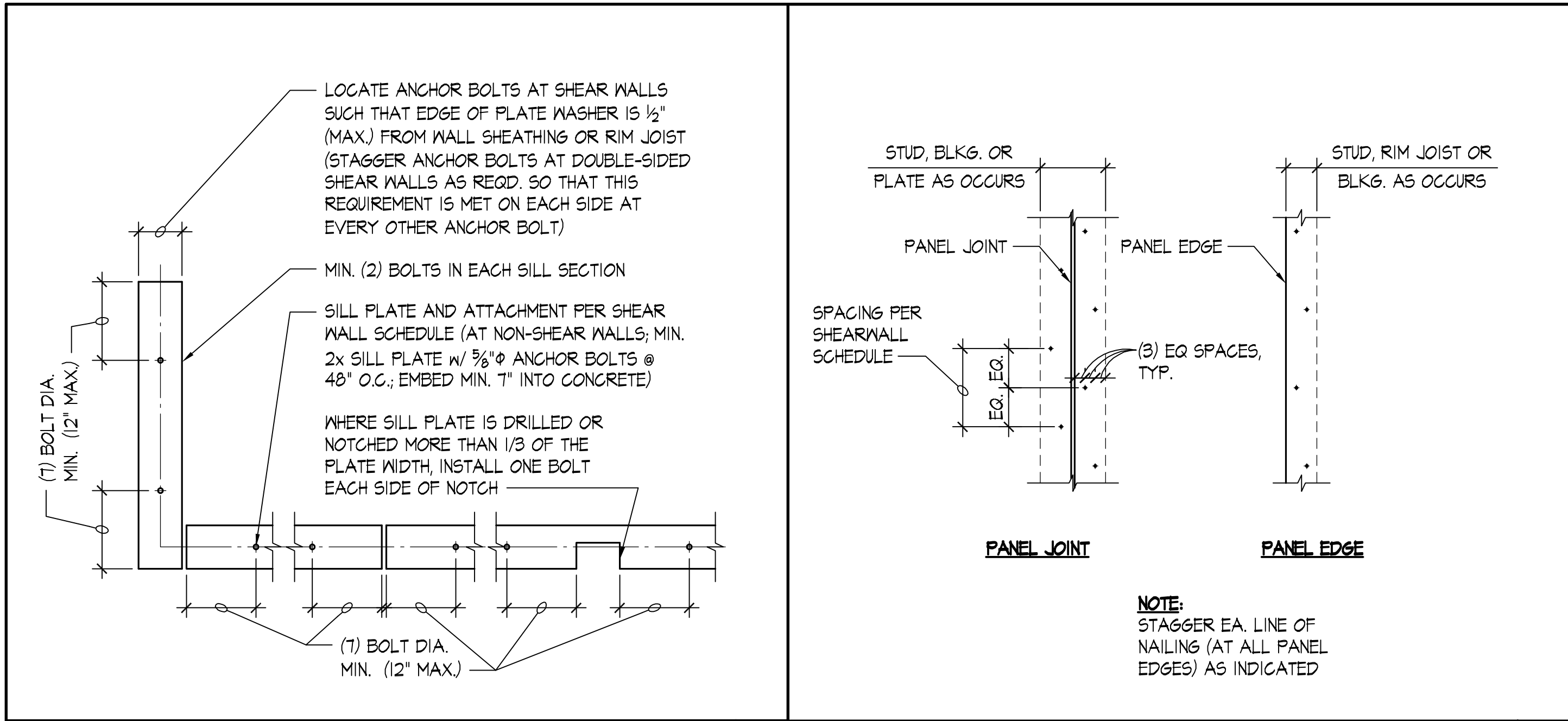
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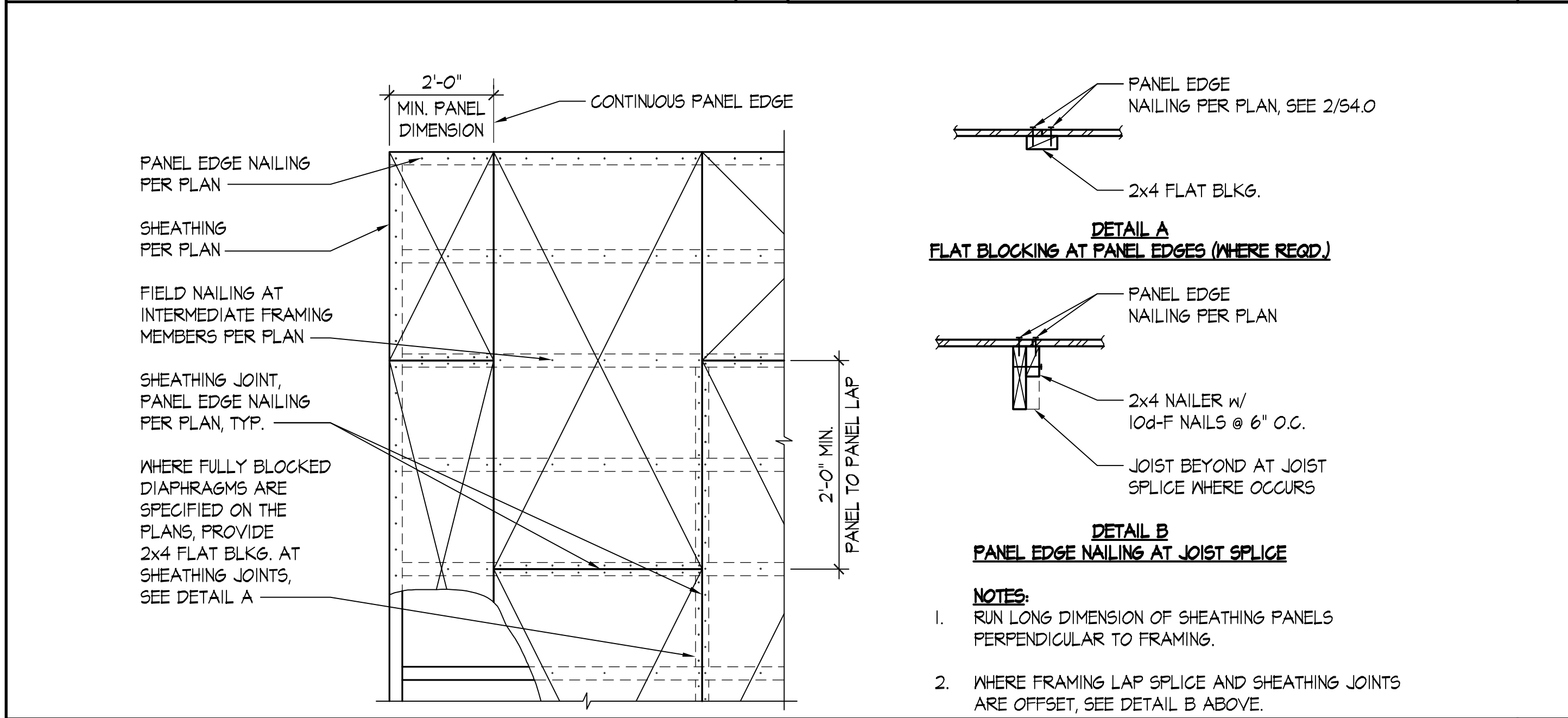
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Date
Project Manager
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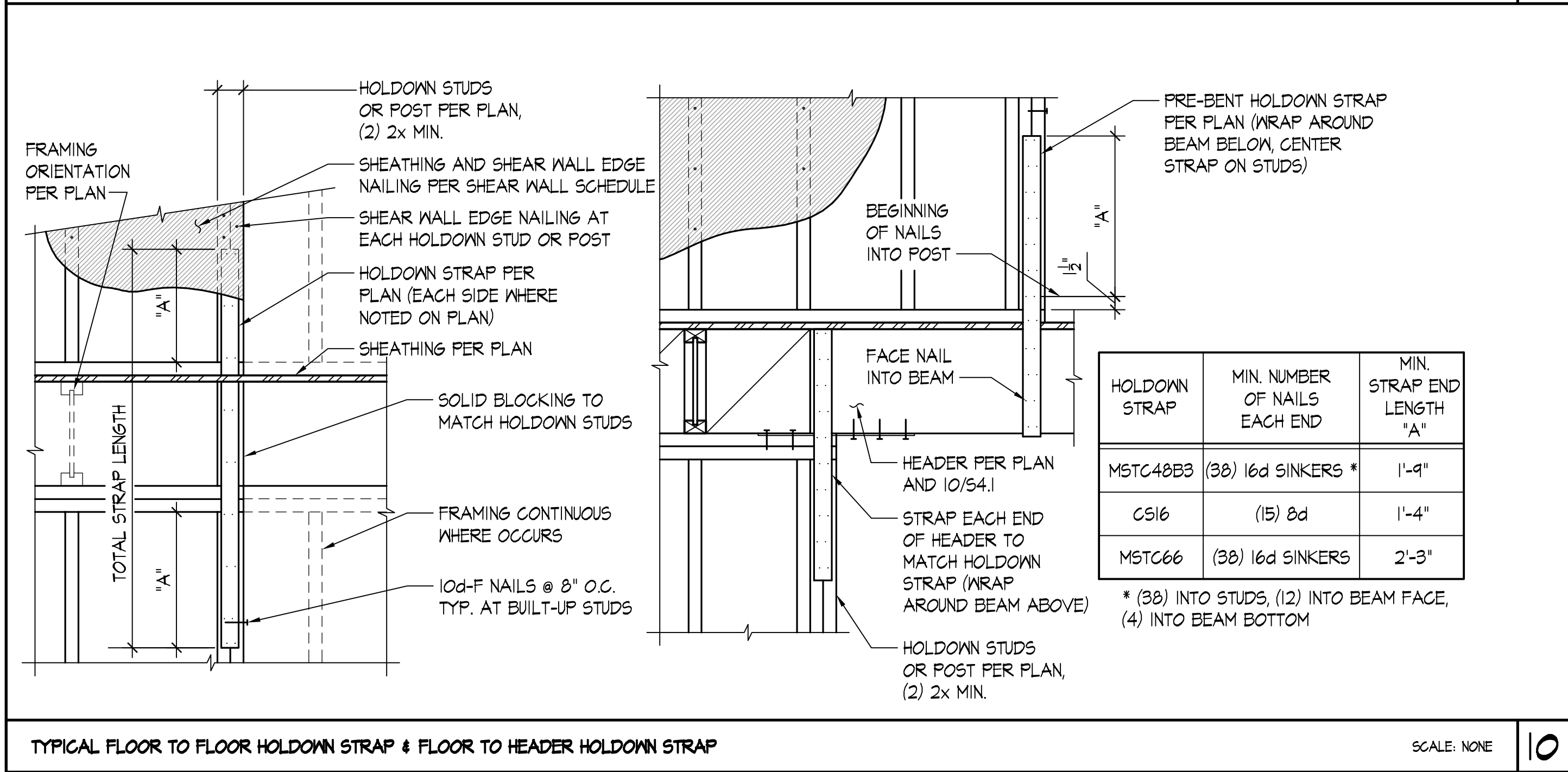
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TYPICAL SILL PLATE BOLTING - PLAN VIEW SCALE: NONE 2 TYPICAL STAGGERED NAILING SCALE: NONE 2



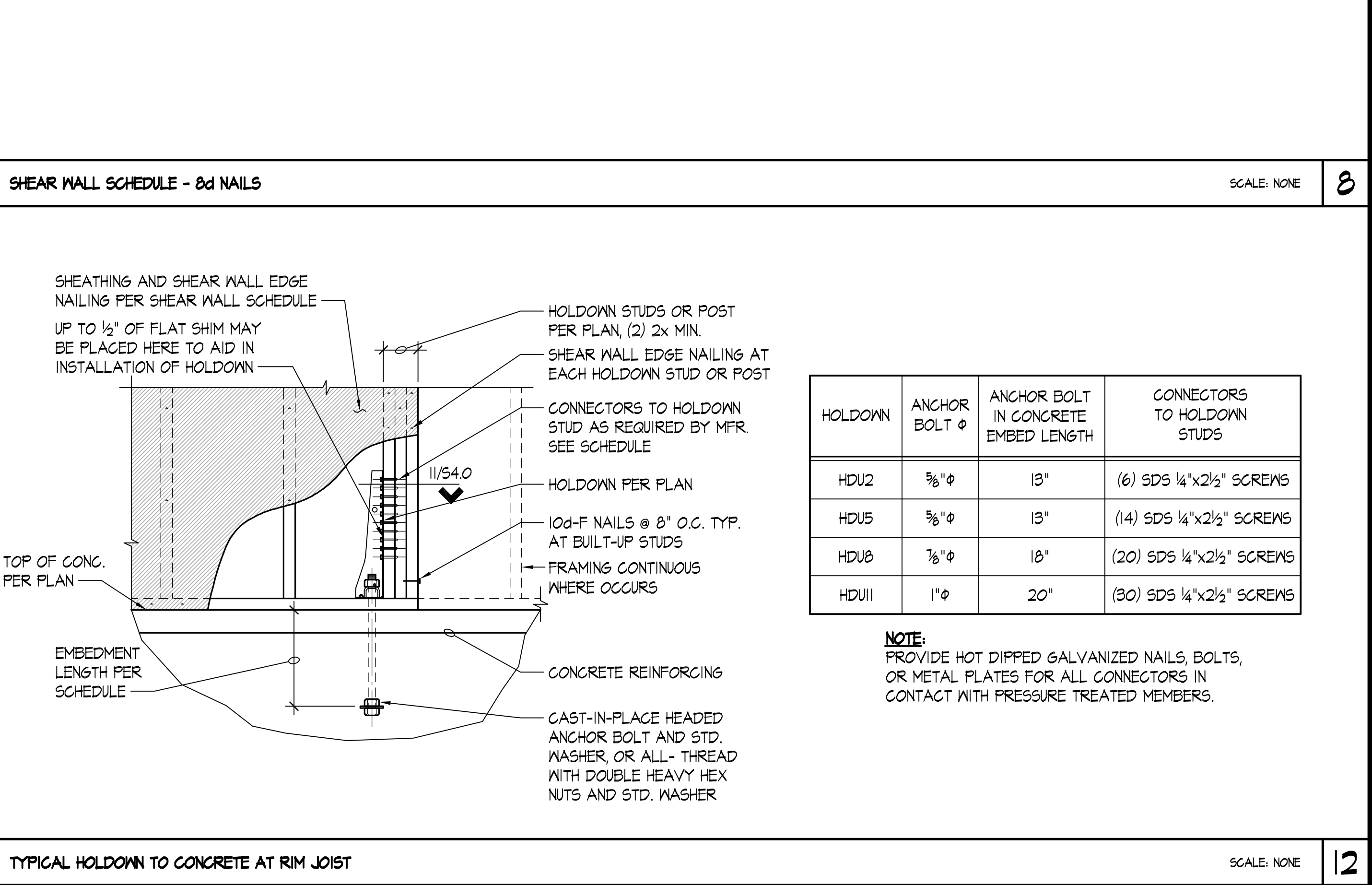
TYPICAL ROOF AND FLOOR DIAPHRAGM SHEATHING SCALE: NONE 6 SHEAR WALL SCHEDULE - Ed NAILS SCALE: NONE 8



TYPICAL FLOOR TO FLOOR HOLDDOWN STRAP & FLOOR TO HEADER HOLDDOWN STRAP SCALE: NONE 10

| SHEAR WALL SCHEDULE | | | | | | | | |
|---------------------|-----------------------------|---------------------------|-------------------------|---|--|----------------|---|-----------------|
| SHEAR WALL TYPE | SHEAR WALL SHEATHING ① | PANEL EDGE FRAMING ② ⑦ | PANEL EDGE NAILING ③ | BOTTOM PLATE ATTACHMENT | | | TOP PLATE ATTACHMENT | |
| | | | | 2x BOTTOM PLATE CONNECTION TO RIM JOIST OR BLOCKING BELOW | ANCHOR BOLTING OF SILL PLATE TO CONCRETE BELOW ④ ⑤ | | RIM JOIST OR BLOCKING CONNECTION TO TOP PLATE ⑥ | |
| | | | | | 3x PLATE | 2x PLATE | INTERIOR WALL | EXTERIOR WALL |
| SW-6 | 7/16" APA ONE-SIDE SHTG. | 2x | 0.131"φx2½" @ 6" O.C. | 0.148"φx3¼" @ 6" O.C. ⑩ | ⅝"φ @ 48" O.C. | ⅝"φ @ 48" O.C. | A35 @ 16" O.C. | LTP4 @ 16" O.C. |
| SW-4 | 7/16" APA ONE-SIDE SHTG. | 3x OR (2) 2x | 0.131"φx2½" @ 4" O.C. ⑧ | 0.148"φx3¼" @ 4" O.C. ⑩ | ⅝"φ @ 48" O.C. | ⅝"φ @ 32" O.C. | A35 @ 16" O.C. | LTP4 @ 16" O.C. |
| SW-3 | 7/16" APA ONE-SIDE SHTG. | 3x OR (2) 2x | 0.131"φx2½" @ 3" O.C. ⑧ | 0.148"φx3¼" @ 3" O.C. ⑩ | ⅝"φ @ 32" O.C. | ⅝"φ @ 24" O.C. | A35 @ 12" O.C. | LTP4 @ 12" O.C. |
| 25W-3 | 7/16" APA TWO-SIDES SHTG. ⑨ | 3x | 0.131"φx2½" @ 3" O.C. ⑧ | (2) ROWS 0.148"φx3¼" @ 3" O.C. ⑪ | ⅝"φ @ 16" O.C. ⑫ | N/A | A35 @ 6" O.C. | LTP4 @ 6" O.C. |

- NOTES:**
- INSTALL PANEL SHEATHING EITHER HORIZONTALLY OR VERTICALLY FOR THE ENTIRE LENGTH OF THE WALL PER PLAN. WALL STUD SPACING SHALL BE 16" O.C. MAXIMUM.
 - ALL INTERMEDIATE WALL STUDS SHALL BE PER PLAN. PROVIDE BACKING FRAMING AT ALL PANEL EDGES INCLUDING HORIZONTAL BLOCKING PER THE SCHEDULE.
 - PROVIDE NAILING TO ALL PANEL EDGES, TOP & BOTTOM PLATES AND HORIZONTAL BLOCKING. PROVIDE THE SAME NAILING PATTERN TO EACH MULTIPLE STUD OF THE BUILT-UP HOLD DOWN POST. NAIL PANEL TO INTERMEDIATE FRAMING MEMBERS w/ 0.131"φ x 2½" @ 12" O.C.
 - EMBED CAST-IN-PLACE 5/8"φ ANCHOR BOLTS 1" MIN. (OR EMBED ADHESIVE ANCHOR BOLTS 5 1/2" IN (E) CONCRETE; SEE STRUCTURAL NOTES). PROVIDE PLATE WASHER 3" x 3" x 1/4" AT EACH ANCHOR BOLT. SILL PLATES SHALL BE TREATED PER GENERAL NOTES, AND SHALL BE 2x OR 3x PER THE SCHEDULE. SEE DETAIL 11/54.0 FOR OTHER REQUIREMENTS.
 - PROVIDE HOT DIPPED GALVANIZED NAILS, BOLTS, OR METAL PLATES FOR ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED MEMBERS.
 - PROVIDE 0.131"φ x 1-1/2" LONG NAILS FOR CLIPS DIRECTLY ATTACHED TO FRAMING MEMBERS; PROVIDE 0.131"φ x 2-1/2" LONG NAILS FOR CLIPS INSTALLED OVER FLOOR OR WALL SHEATHING ON FRAMING MEMBERS. SEE 6/54.1 FOR TOP PLATE SPLICE.
 - ALTERNATIVE TO 3x STUDS AND 3x HORIZ. BLOCKING IS (2) 2x STUDS/BLKG. NAILED TOGETHER WITH 0.148"φ x 3" LONG NAILS WITH THE SAME SPACING AS THE PANEL EDGE NAILING PER THE SCHEDULE (STAGGER).
 - STAGGER NAILS PER 2/54.0.
 - STAGGER PANEL EDGE JOINTS AT DOUBLE-SIDED SHEAR WALLS SO THAT JOINTS ON OPPOSITE SIDES ARE NOT AT THE SAME STUD.
 - RIM JOIST/BLOCKING MINIMUM WIDTH OF 1¾". STAGGER NAILS PER 2/54.0 WHERE SPACING IS LESS THAN 6" O.C.
 - RIM JOIST/BLOCKING MINIMUM WIDTH OF 1¾" AT EXTERIOR WALLS, 3½" AT INTERIOR WALLS. STAGGER NAILS PER 2/54.0.
 - STAGGER ANCHOR BOLTS ON EITHER SIDE OF SILL PLATE AS NOTED ON 1/54.0.

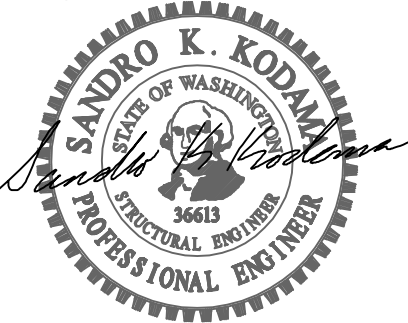


TYPICAL HOLDDOWN TO CONCRETE AT RIM JOIST SCALE: NONE 12

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| 201906.01 | 1 | Project number | 6/29/2020 | SKK | SC |
| 201906.01 | 2 | Date | 6/29/2020 | SKK | SC |
| 201906.01 | 3 | Project Manager | 6/29/2020 | SKK | SC |
| 201906.01 | 4 | Drawn by | 6/29/2020 | SKK | SC |
| 201906.01 | 5 | Checked by | 6/29/2020 | SKK | SC |
| 201906.01 | 6 | AS NOTED | 6/29/2020 | SKK | SC |

DETAILS

S4.0

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JOIST PER PLAN

NON-BEARING WALL PER ARCH. DRAWINGS

(2) 16d NAILS @ 48" O.C. TO BLOCKING

FULL-DEPTH BLOCKING @ 48" O.C.

WALL PARALLEL TO FLOOR FRAMING

JOIST PER PLAN

NON-BEARING WALL PER ARCH. DRAWINGS

16d NAILS @ 16" O.C. TO BLOCKING

FULL-DEPTH BLOCKING

WALL PERPENDICULAR TO FLOOR FRAMING

TYPICAL NON-STRUCTURAL WALL SUPPORT (BOTTOM) - I-JOIST

SCALE: NONE

1

JOIST PER PLAN

NON-BEARING WALL PER ARCH. DRAWINGS

SIMPSON STC CLIP @ 48" O.C. SLOT VERTICAL (NAIL AT MID-HEIGHT OF SLOT)

WALL PARALLEL TO ROOF/FLOOR FRAMING

JOIST PER PLAN

NON-BEARING WALL PER ARCH. DRAWINGS

SIMPSON STC CLIP @ 48" O.C. SLOT VERTICAL (NAIL AT MID-HEIGHT OF SLOT)

WALL PERPENDICULAR TO ROOF/FLOOR FRAMING

TYPICAL NON-STRUCTURAL WALL SUPPORT (TOP) - I-JOIST

SCALE: NONE

2

| CEILING JOIST SCHEDULE | |
|------------------------------------|------------------|
| SIZE | MAX. SPAN |
| 2x4 @ 24" O.C. 2x4 @ 16" O.C. | 8'-0" 9'-2" |
| 2x6 @ 24" O.C. 2x6 @ 16" O.C. | 12'-6" 14'-4" |
| 2x8 @ 24" O.C. 2x8 @ 16" O.C. | 16'-6" 19'-0" |
| 2x10 @ 24" O.C. 2x10 @ 16" O.C. | 21'-2" 24'-3" |

NOTES:
CEILING JOIST TABLE BASED ON
HF #2, F_b=850 PSI (REPETITIVE
MEMBER USE), F_v = 150 PSI
E=1.3X10⁶ PSI, DEFL. < L/240

ATTIC LIVE LOAD = 10.0 PSF
CEILING DEAD LOAD = 5.0 PSF

CEILING JOIST SCHEDULE

SCALE: NONE

3

10d FACE NAILS w/ THE SAME SPACING AS THE PANEL EDGE NAILING PER THE SHEAR WALL SCHEDULE (SEE NOTE 1 AT NON-SHEAR WALLS)

WALL CORNER

10d FACE NAILS w/ THE SAME SPACING AS THE PANEL EDGE NAILING PER THE SHEAR WALL SCHEDULE (SEE NOTE 1 AT NON-SHEAR WALLS)

WALL INTERSECTION

TYPICAL WALL INTERSECTIONS - RESIDENTIAL

SCALE: NONE

8

CUT

CIRCULAR NOTCH

V NOTCH

LET-IN NOTCH

STUD DEPTH

MAX. DEPTH

MIN. DEPTH

MIN. 5/8" EDGE DIST (TYP)

MAX. DIAMETER OF HOLES

HOLES ARE TO BE SPACED AT LEAST A STUD DEPTH FROM A CUT OR NOTCH

HOLES ARE TO BE SPACED AT LEAST TWICE THE DIAMETER OF THE LARGEST HOLE

A. CUTTING AND NOTCHING WOOD STUDS
(DO NOT NOTCH MORE THAN 3 ADJACENT STUDS w/o REVIEW BY ENGINEER)

BEARING WALL STUDS:

| STUD SIZE | MAX. DEPTH OF SAW CUT OR NOTCH | MIN. DEPTH REMAINING AFTER CUT OR NOTCH |
|-----------|--------------------------------|---|
| 2x4 | 1/8" | 2-3/8" |
| 2x6 | 1-3/8" | 4-1/8" |
| 2x8 | 1-7/8" | 5-3/8" |

NON-BEARING WALL STUDS:

| STUD SIZE | MAX. DEPTH OF SAW CUT OR NOTCH | MIN. DEPTH REMAINING AFTER CUT OR NOTCH |
|-----------|--------------------------------|---|
| 2x4 | 1-1/2" | 2" |
| 2x6 | 2-3/8" | 3-1/8" |
| 2x8 | 3" | 4-1/4" |

B. HOLES IN WOOD STUDS

BEARING WALL:

| STUD SIZE | MAX. DIAMETER OF HOLE |
|-----------|-----------------------|
| 2x4 | 1-1/2" |
| 2x6 | 2-3/8" |
| 2x8 | 3" |

NON-BEARING WALL:

| STUD SIZE | MAX. DIAMETER OF HOLE |
|-----------|-----------------------|
| 2x4 | 2-1/4" |
| 2x6 | 3-3/8" |
| 2x8 | 4-1/2" |

TYPICAL ALLOWABLE HOLES AND NOTCHES IN STUDS

SCALE: NONE

9

(12) 10d-F NAILS @ 3" O.C. STAGGERED IN 2 ROWS AT EACH SIDE OF EACH SPLICE

10d-F NAILS @ 12" O.C. STAGGERED IN 2 ROWS ELSEWHERE

TOP CHORD SPLICE

BOTTOM CHORD SPLICE

4'-0" MIN. BETWEEN SPLICES

SPLICE TO OCCUR AT 1/2 OF STUD (TYP)

TYPICAL TOP PLATE SPLICE

SCALE: NONE

6

WHERE HEADER INTERRUPTS DOUBLE TOP PLATE, PROVIDE MST24, U.O.N.

DOUBLE TOP PLATE

A35 AT EXTERIOR HEADERS OVER 6'-0" IN LENGTH

(8) 10d-F NAILS, MIN.

KING STUD

SINGLE PLATE (PROVIDE DOUBLE PLATE AT EXTERIOR OPENINGS OVER 8'-0" IN LENGTH)

HEADER PER PLAN

10d-F NAILS @ 8" O.C. AT BUILT-UP STUDS, TYP.

TRIM STUD POST PER PLAN (PROVIDE MIN. (2) 2x STUDS AT HEADERS OVER 6'-0" IN LENGTH)

TYPICAL HEADER

SCALE: NONE

10

PROVIDE BLOCKING OR RIM JOIST AT BEARING WALLS

BEAM BEARING LENGTH TO MATCH POST BELOW

JOISTS PER PLAN

BEAM PER PLAN

STRAP PER PLAN (WHERE OCCURS), CENTER STRAP LENGTH ON WALL END

PROVIDE LTP4 IF NO STRAP, BEAM TO PLATE

10d-F NAILS @ 8" O.C. AT BUILT-UP STUDS, TYP.

DOUBLE STUD POST U.O.N.

3" MIN.

TYPICAL FLUSH BEAM

SCALE: NONE

11

10d FACE NAILS w/ THE SAME SPACING AS THE PANEL EDGE NAILING PER THE SHEAR WALL SCHEDULE (SEE NOTE 1 AT NON-SHEAR WALLS)

10d FACE NAILS w/ THE SAME SPACING AS THE PANEL EDGE NAILING PER THE SHEAR WALL SCHEDULE (SEE NOTE 1 AT NON-SHEAR WALLS)

L590 CLIP EA. END OF EA. STRINGER

TREADS & RISERS PER ARCH.

BEAM PER PLAN

2x6 STRONGBACK (FAR SIDE) w/ (2) ROWS OF 10d-F NAILS @ 8" O.C. STAGGERED AS SHOWN

11'-0" MAX.

TYPICAL STAIR STRINGER

SCALE: NONE

12

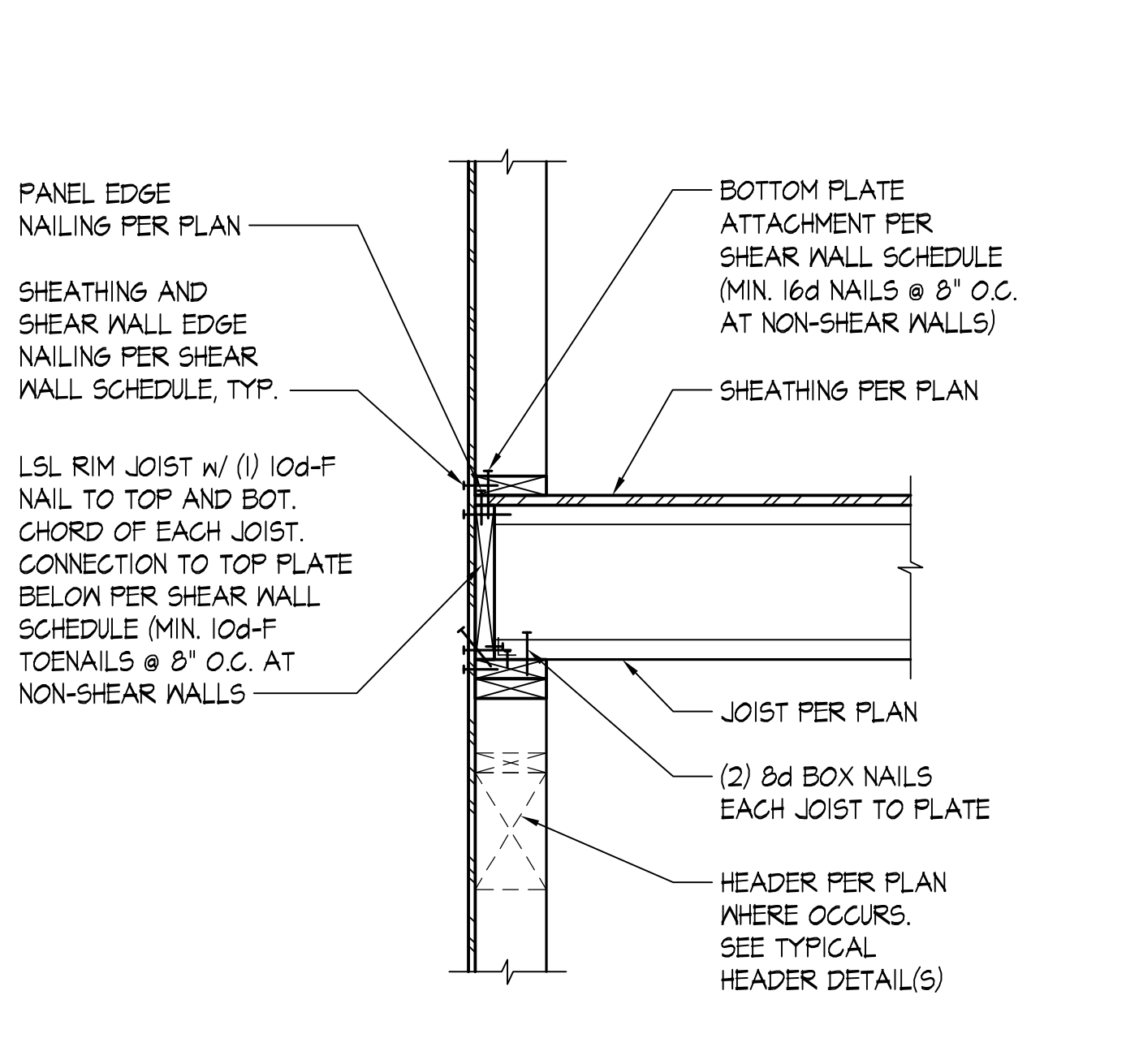
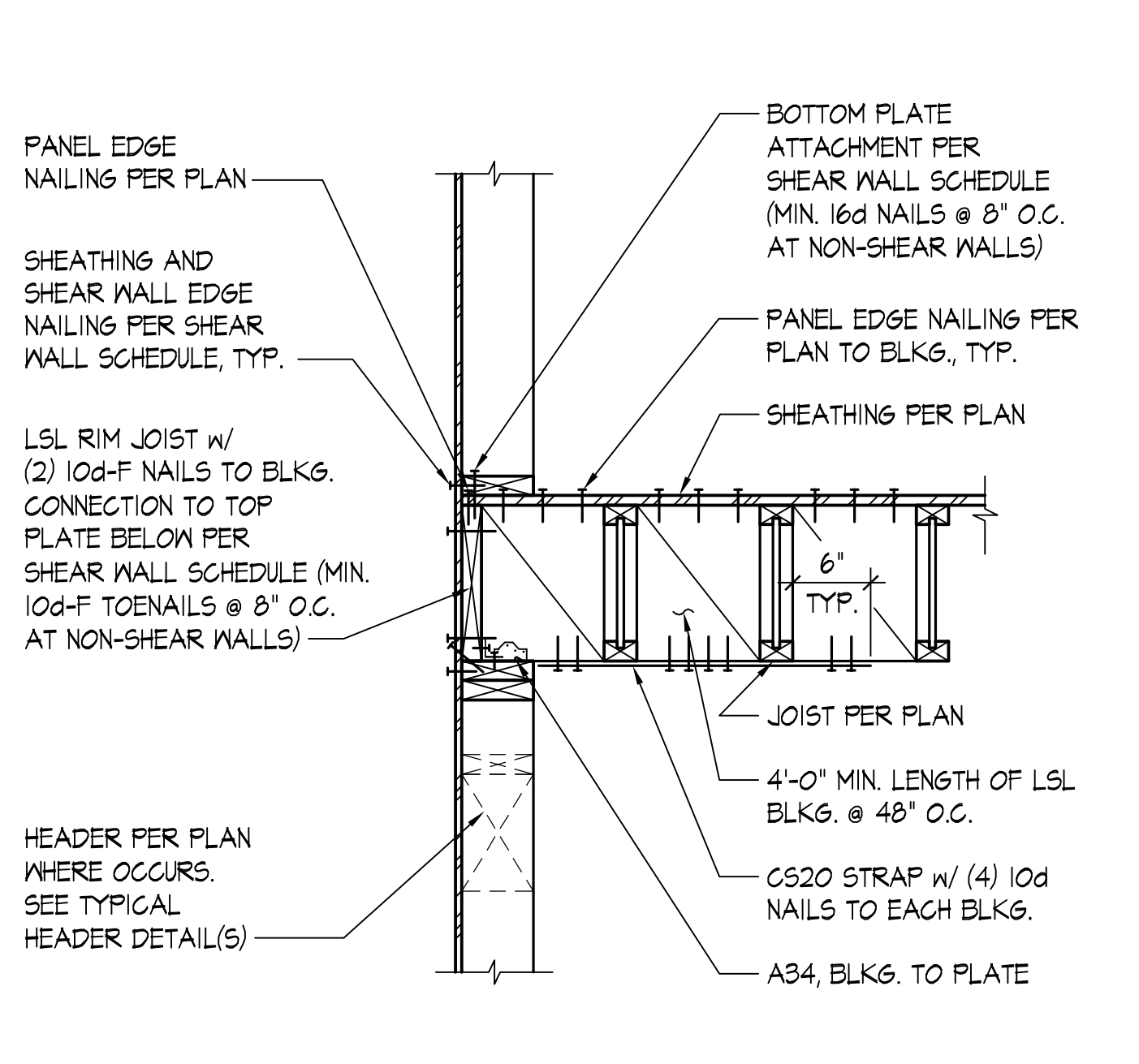
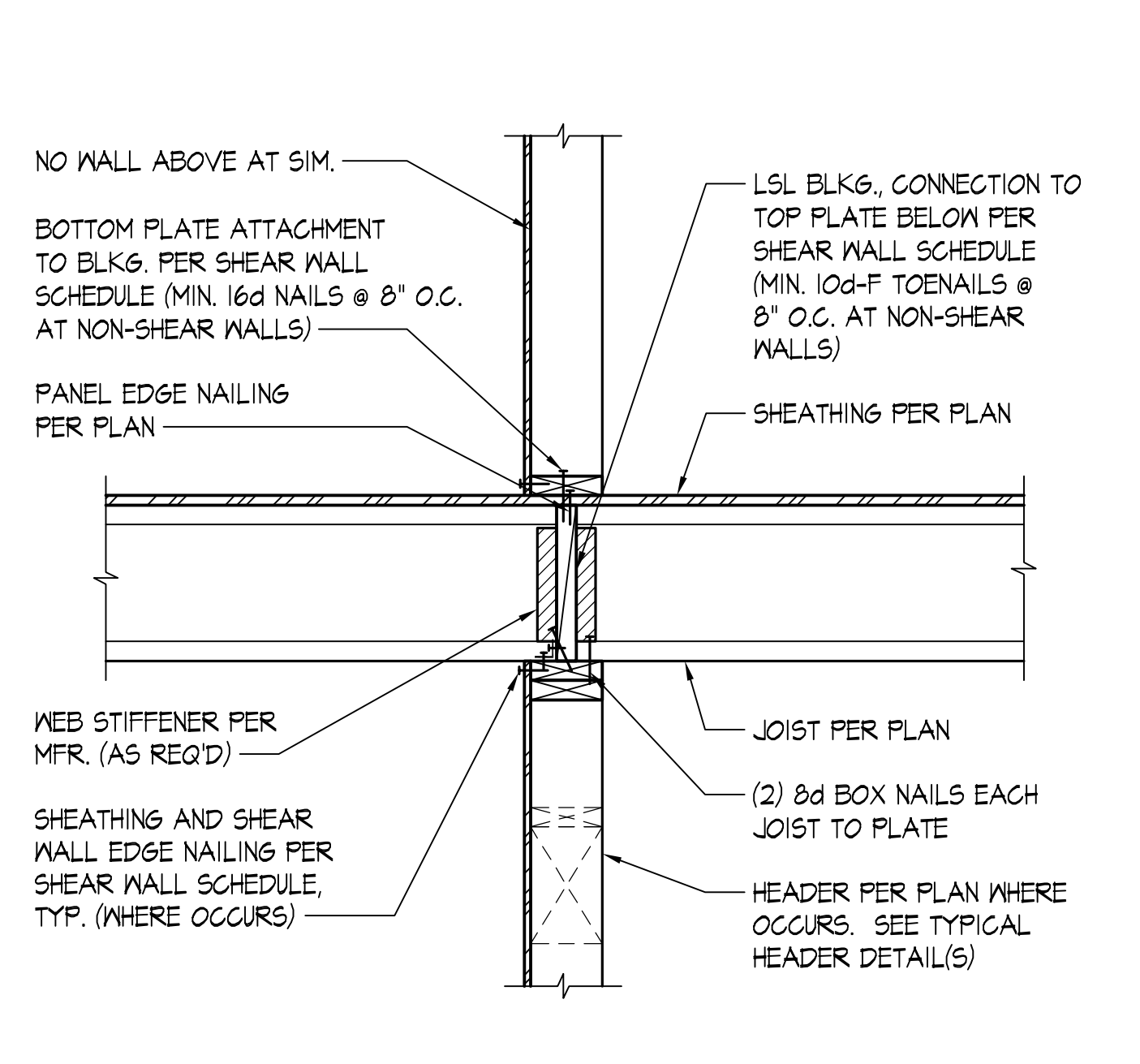
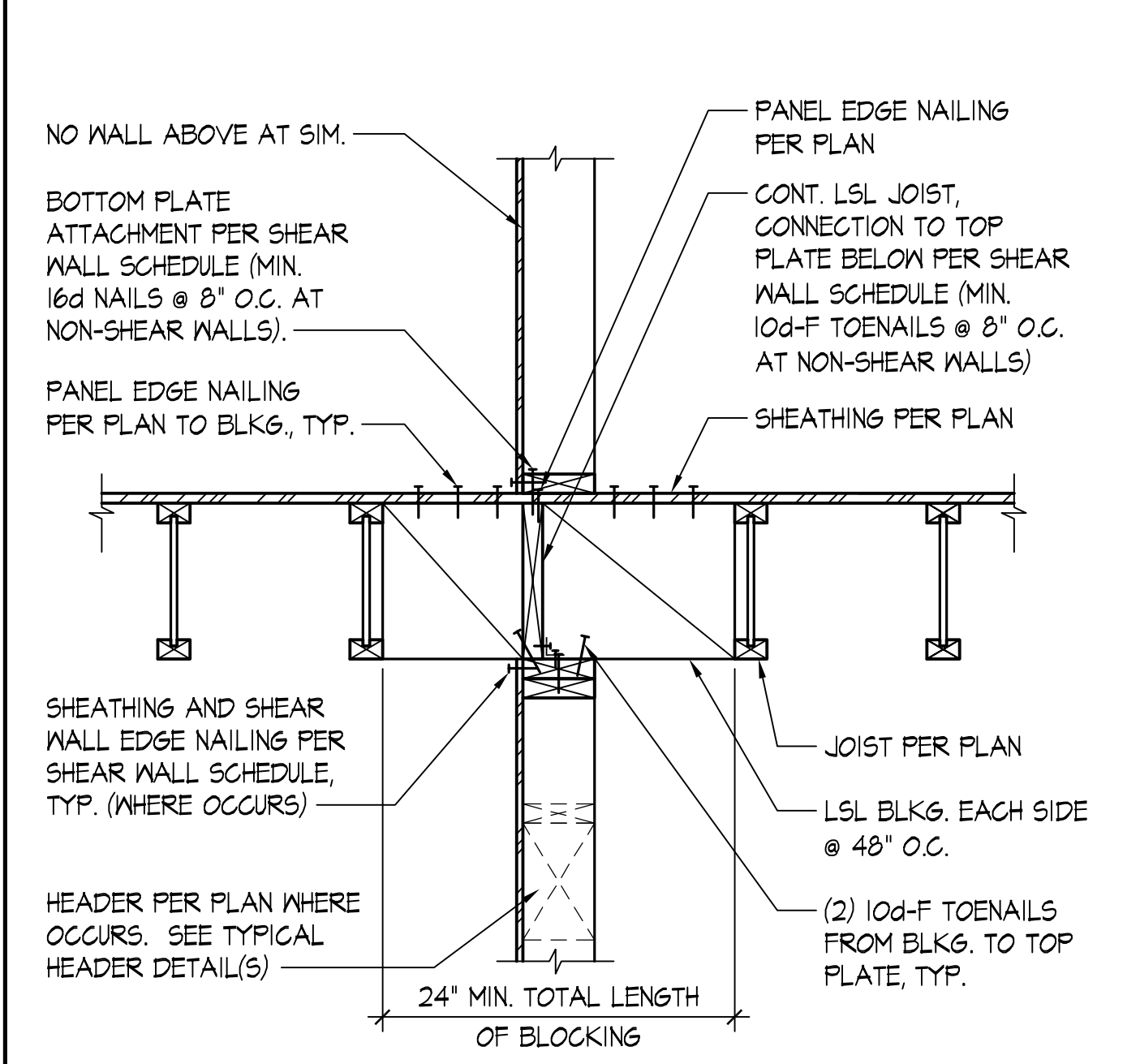
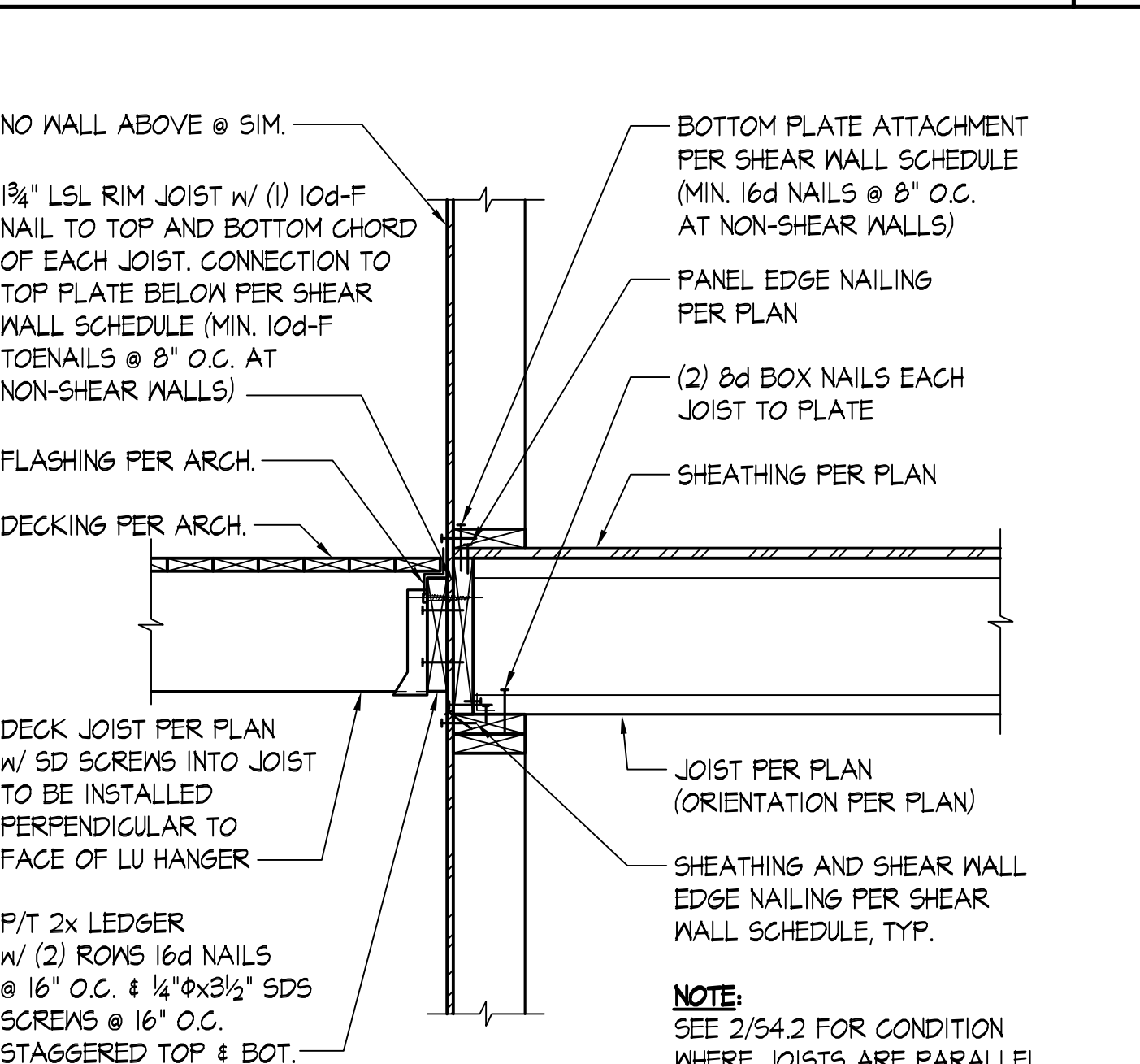
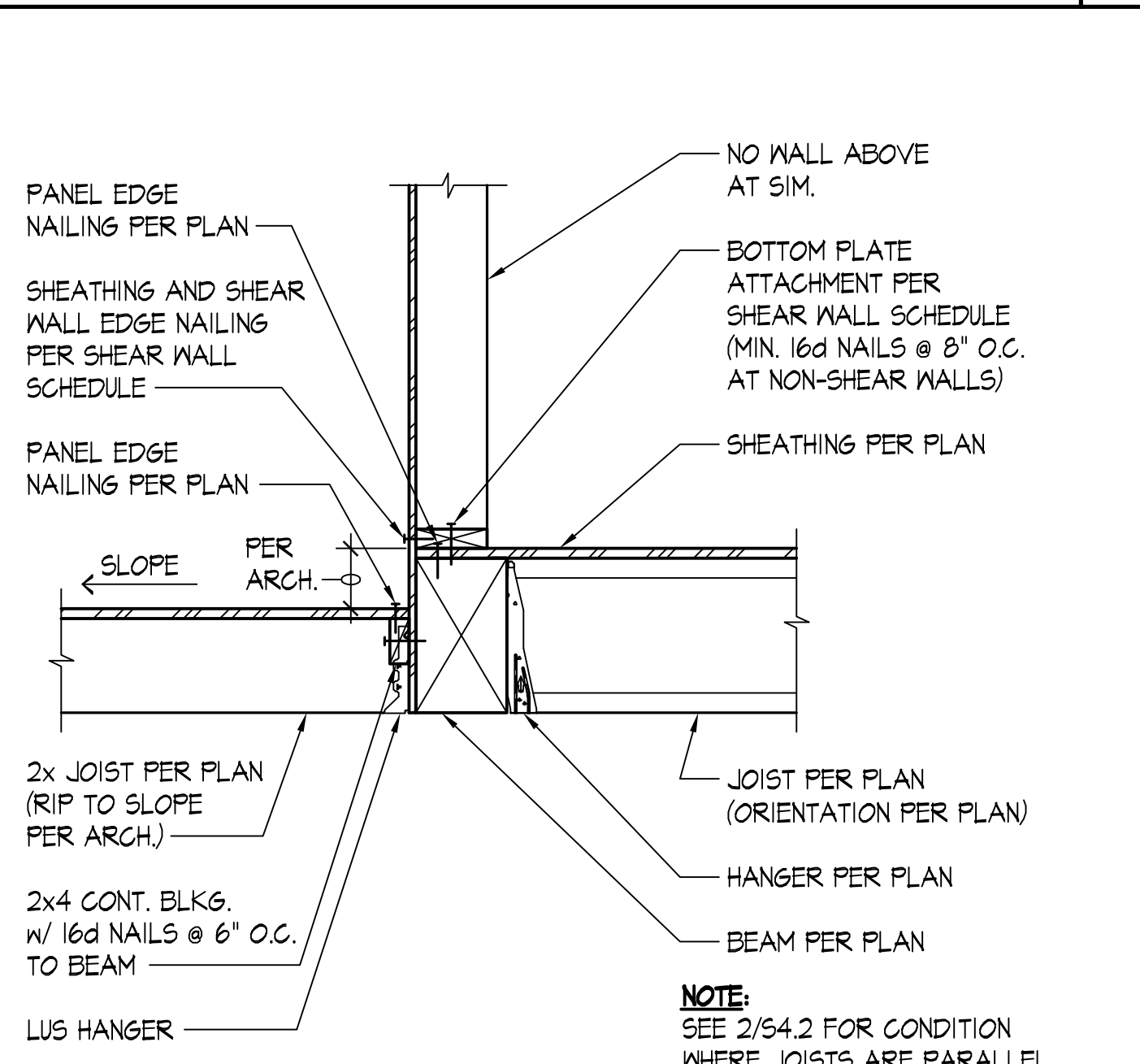
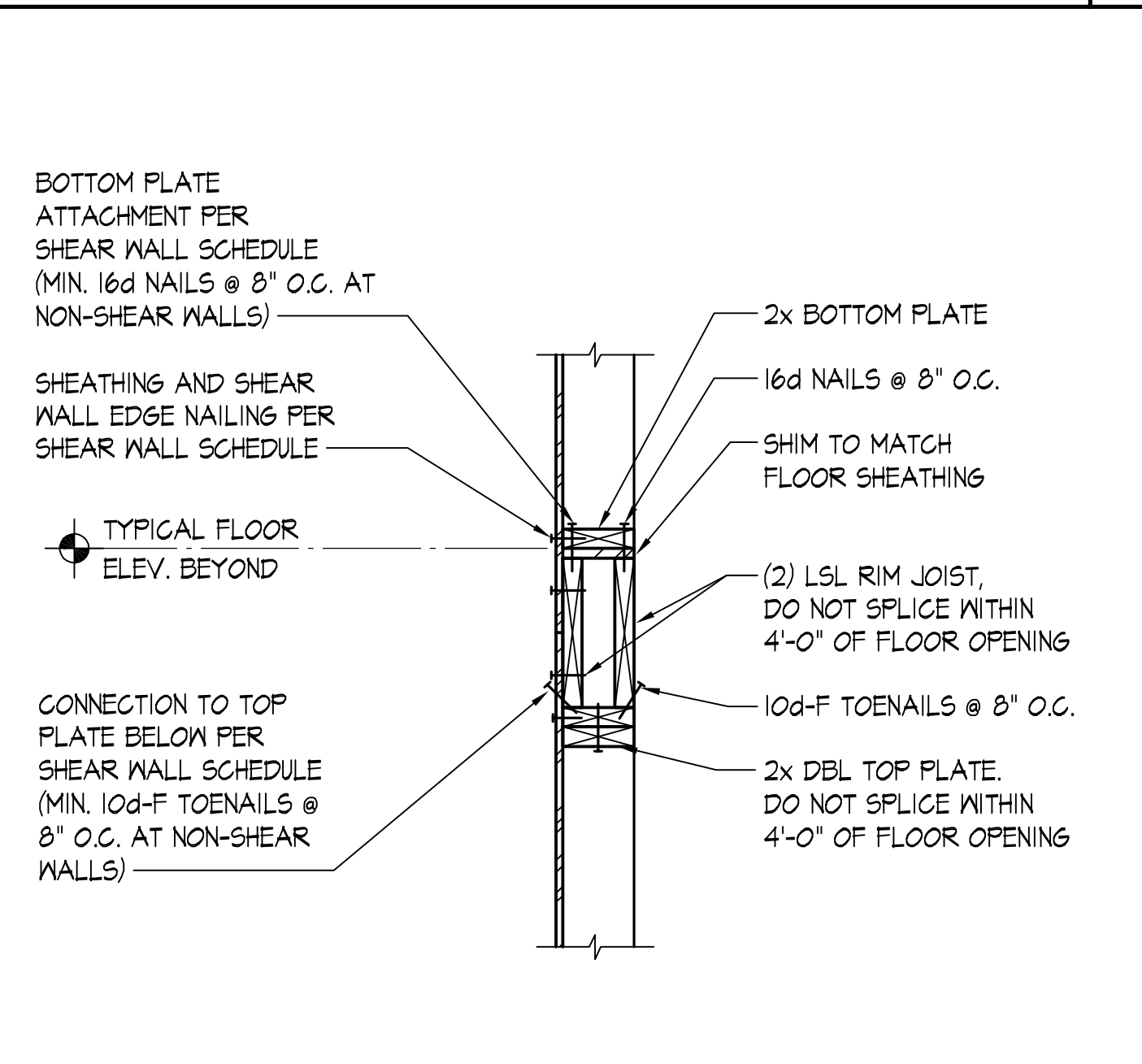
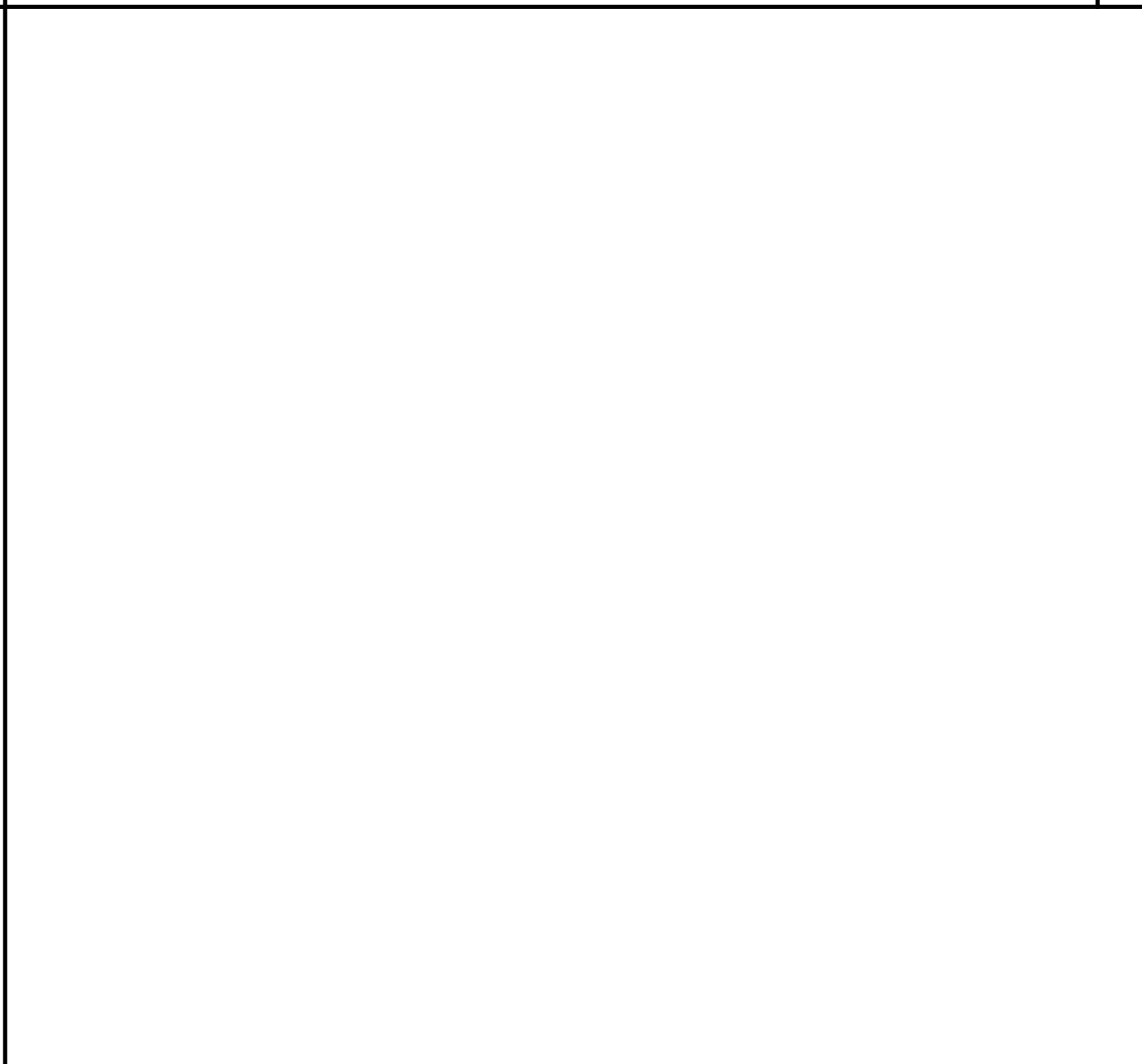
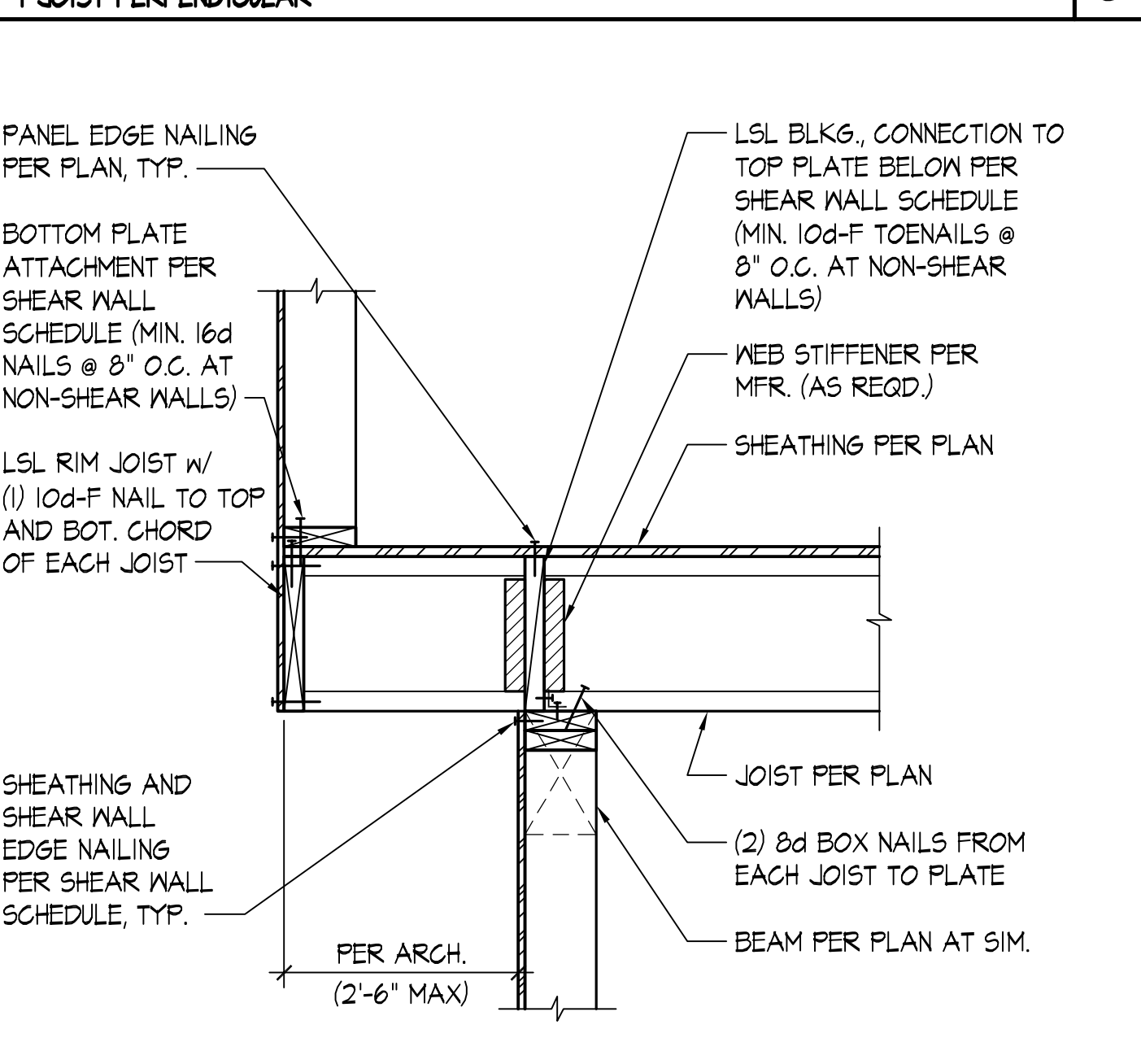
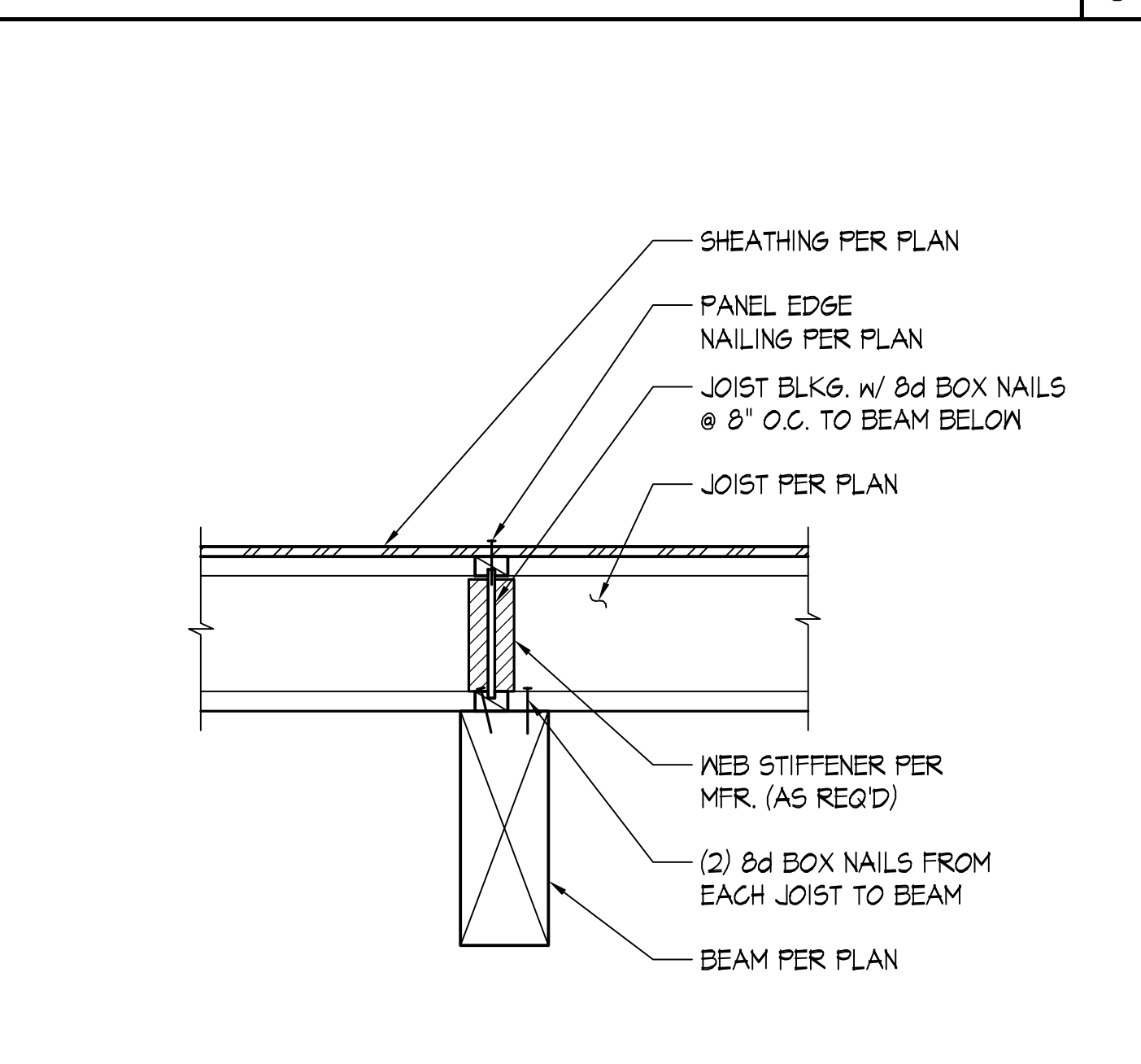
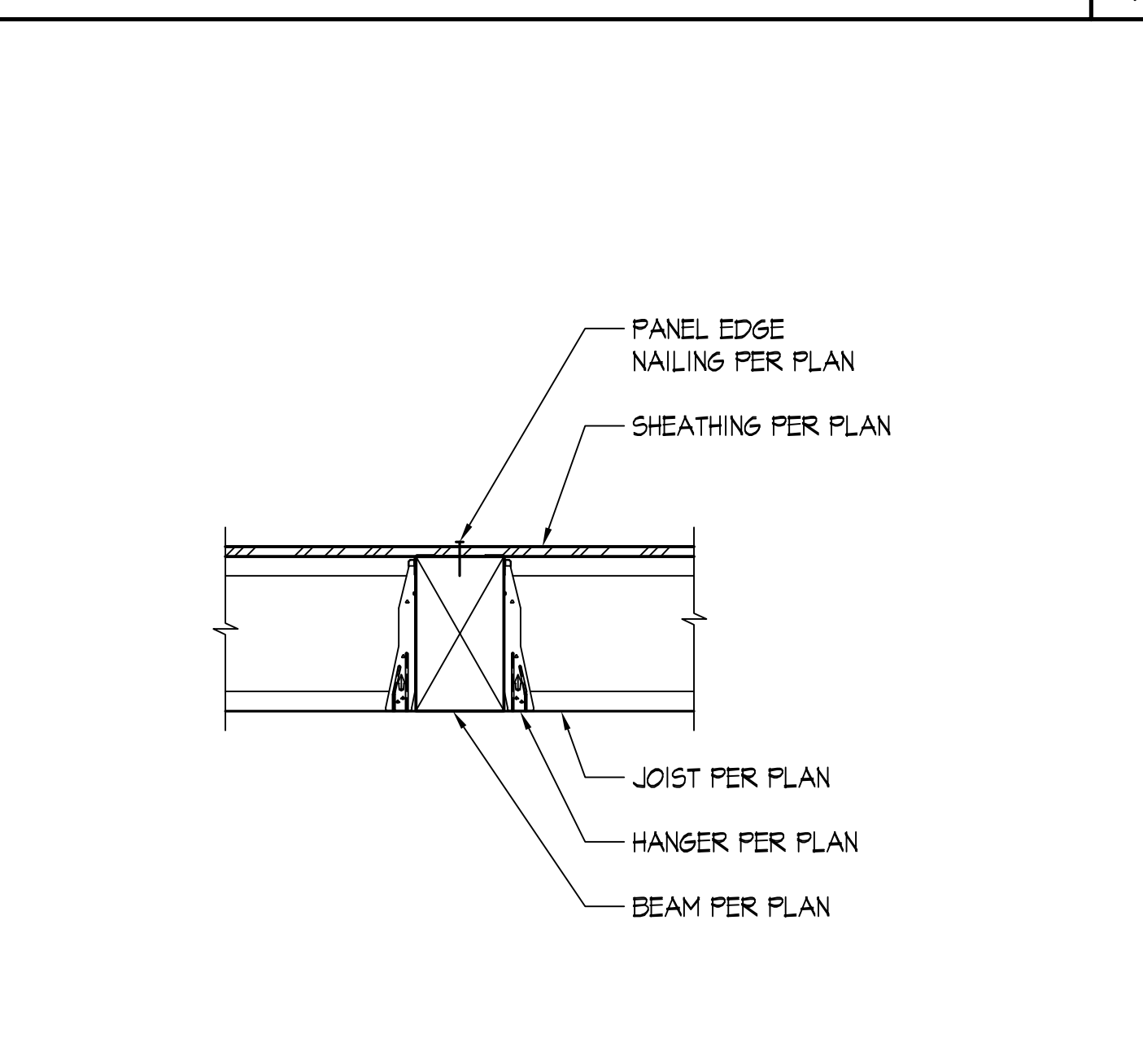
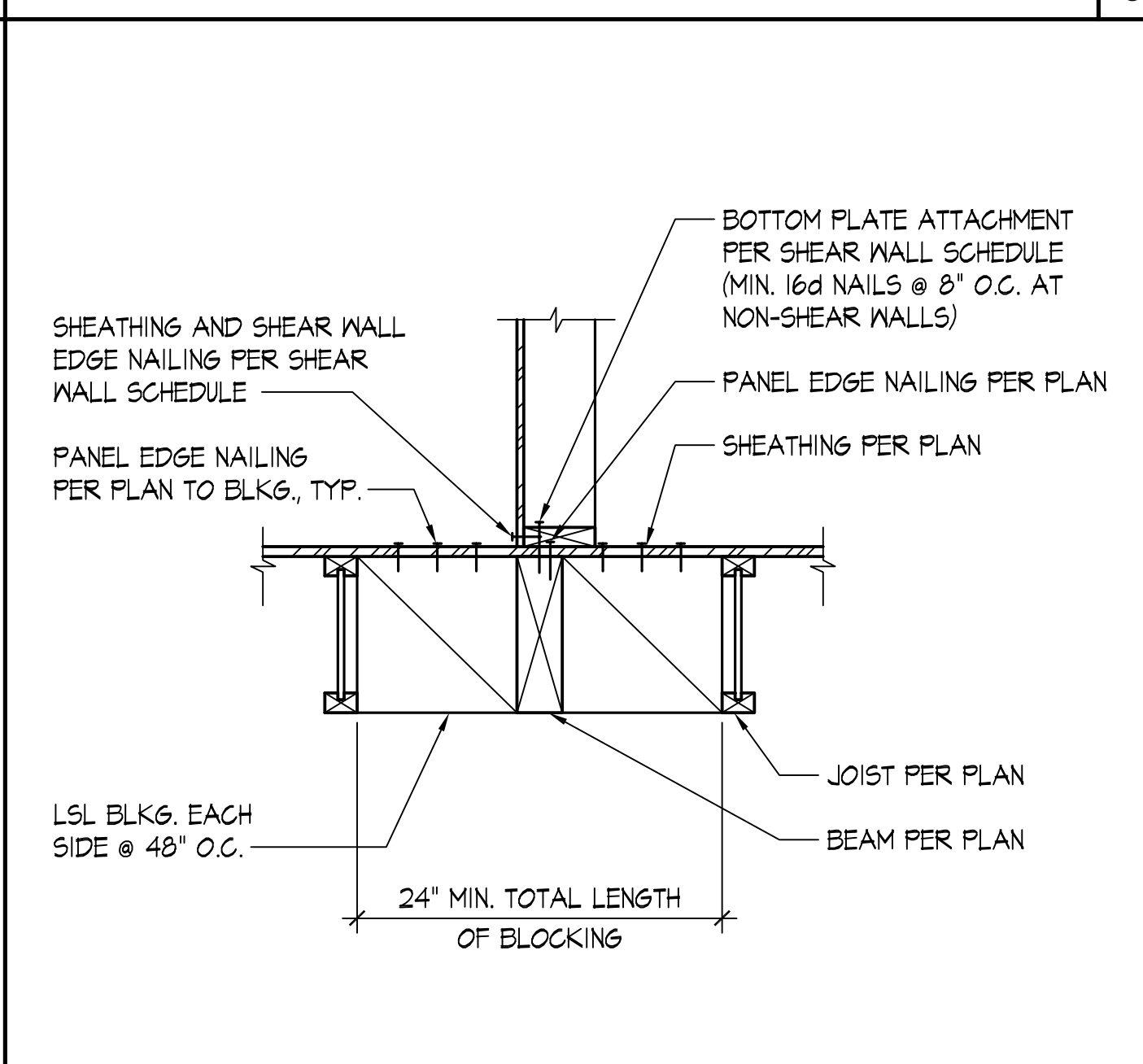
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Official
Stamps:

| | | | | | |
|--|----------------|-----------------|-----------------|-----|-----------|
| SCHELLINGS HOUSE 5218 16th Avenue NE, Seattle, WA 98105 | DATE | | PERMIT SET | | 6/29/2020 |
| | REVISIONS | NO. DESCRIPTION | | | |
| | 201906.01 | 201906.01 | SKK | SC | TON |
| DETAILS | Project number | 6282020 | Project Manager | SKK | AS NOTED |
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| | Checked by | | | | |
| S4.1 | | | | | Scale |

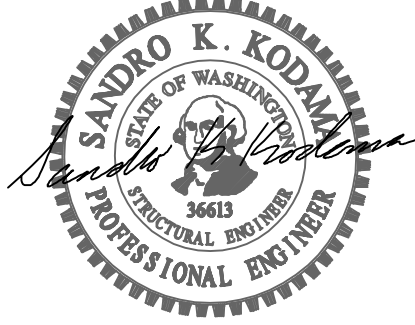
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|--|---|-------------|--|----|-------------|---|----|-------------|--|----|-------------|
|  <p>PANEL EDGE NAILING PER PLAN</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE, TYP.</p> <p>LSL RIM JOIST w/ (1) 10d-F NAIL TO TOP AND BOT. CHORD OF EACH JOIST. CONNECTION TO TOP PLATE BELOW PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>JOIST PER PLAN</p> <p>(2) 8d BOX NAILS EACH JOIST TO PLATE</p> <p>HEADER PER PLAN WHERE OCCURS. SEE TYPICAL HEADER DETAIL(S)</p> <p>BOTTOM PLATE ATTACHMENT PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>SHEATHING PER PLAN</p> | 1 | SCALE: NONE |  <p>PANEL EDGE NAILING PER PLAN</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE, TYP.</p> <p>LSL RIM JOIST w/ (2) 10d-F NAILS TO BLKG. CONNECTION TO TOP PLATE BELOW PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>JOIST PER PLAN</p> <p>4'-0" MIN. LENGTH OF LSL BLKG. @ 48" O.C.</p> <p>CS20 STRAP w/ (4) 10d NAILS TO EACH BLKG.</p> <p>A34, BLKG. TO PLATE</p> <p>HEADER PER PLAN WHERE OCCURS. SEE TYPICAL HEADER DETAIL(S)</p> <p>6" TYP.</p> <p>4'-0" MIN. LENGTH OF LSL BLKG. @ 48" O.C.</p> <p>CS20 STRAP w/ (4) 10d NAILS TO EACH BLKG.</p> <p>A34, BLKG. TO PLATE</p> | 2 | SCALE: NONE |  <p>NO WALL ABOVE AT SIM.</p> <p>BOTTOM PLATE ATTACHMENT TO BLKG. PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>SHEATHING PER PLAN</p> <p>WEB STIFFENER PER MFR. (AS REQ'D)</p> <p>JOIST PER PLAN</p> <p>(2) 8d BOX NAILS EACH JOIST TO PLATE</p> <p>HEADER PER PLAN WHERE OCCURS. SEE TYPICAL HEADER DETAIL(S)</p> <p>LSL BLKG. CONNECTION TO TOP PLATE BELOW PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE, TYP. (WHERE OCCURS)</p> | 3 | SCALE: NONE |  <p>NO WALL ABOVE AT SIM.</p> <p>BOTTOM PLATE ATTACHMENT PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS).</p> <p>PANEL EDGE NAILING PER PLAN TO BLKG., TYP.</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE, TYP. (WHERE OCCURS)</p> <p>HEADER PER PLAN WHERE OCCURS. SEE TYPICAL HEADER DETAIL(S)</p> <p>24" MIN. TOTAL LENGTH OF BLOCKING</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>CONT. LSL JOIST, CONNECTION TO TOP PLATE BELOW PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>JOIST PER PLAN</p> <p>LSL BLKG. EACH SIDE @ 48" O.C.</p> <p>(2) 10d-F TOENAILS FROM BLKG. TO TOP PLATE, TYP.</p> | 4 | SCALE: NONE |
|  <p>NO WALL ABOVE @ SIM.</p> <p>1 3/4" LSL RIM JOIST w/ (1) 10d-F NAIL TO TOP AND BOTTOM CHORD OF EACH JOIST. CONNECTION TO TOP PLATE BELOW PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>FLASHING PER ARCH.</p> <p>DECKING PER ARCH.</p> <p>DECK JOIST PER PLAN w/ SD SCREWS INTO JOIST TO BE INSTALLED PERPENDICULAR TO FACE OF LU HANGER</p> <p>P/T 2x LEDGER w/ (2) ROWS 16d NAILS @ 16" O.C. & 1/4"x3 1/2" SDs SCREWS @ 16" O.C. STAGGERED TOP & BOT.</p> <p>JOIST PER PLAN (ORIENTATION PER PLAN)</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE, TYP.</p> <p>NOTE: SEE 2/54.2 FOR CONDITION WHERE JOISTS ARE PARALLEL</p> <p>BOTTOM PLATE ATTACHMENT PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>(2) 8d BOX NAILS EACH JOIST TO PLATE</p> <p>SHEATHING PER PLAN</p> | 5 | SCALE: NONE |  <p>PANEL EDGE NAILING PER PLAN</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>2x JOIST PER PLAN (RIP TO SLOPE PER ARCH.)</p> <p>2x4 CONT. BLKG. w/ 16d NAILS @ 6" O.C. TO BEAM</p> <p>LU5 HANGER</p> <p>NO WALL ABOVE AT SIM.</p> <p>BOTTOM PLATE ATTACHMENT PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>SHEATHING PER PLAN</p> <p>JOIST PER PLAN (ORIENTATION PER PLAN)</p> <p>HANGER PER PLAN</p> <p>BEAM PER PLAN</p> <p>NOTE: SEE 2/54.2 FOR CONDITION WHERE JOISTS ARE PARALLEL</p> | 6 | SCALE: NONE |  <p>BOTTOM PLATE ATTACHMENT PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE</p> <p>TYPICAL FLOOR ELEV. BEYOND</p> <p>CONNECTION TO TOP PLATE BELOW PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>2x BOTTOM PLATE</p> <p>16d NAILS @ 8" O.C.</p> <p>SHIM TO MATCH FLOOR SHEATHING</p> <p>(2) LSL RIM JOIST, DO NOT SPLICE WITHIN 4'-0" OF FLOOR OPENING</p> <p>10d-F TOENAILS @ 8" O.C.</p> <p>2x DBL TOP PLATE, DO NOT SPLICE WITHIN 4'-0" OF FLOOR OPENING</p> | 7 | SCALE: NONE |  <p>DETAIL</p> | 8 | SCALE: NONE |
|  <p>PANEL EDGE NAILING PER PLAN, TYP.</p> <p>BOTTOM PLATE ATTACHMENT PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>LSL RIM JOIST w/ (1) 10d-F NAIL TO TOP AND BOT. CHORD OF EACH JOIST</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE, TYP.</p> <p>PER ARCH. (2'-6" MAX)</p> <p>JOIST PER PLAN</p> <p>(2) 8d BOX NAILS FROM EACH JOIST TO PLATE</p> <p>BEAM PER PLAN AT SIM.</p> <p>LSL BLKG. CONNECTION TO TOP PLATE BELOW PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>WEB STIFFENER PER MFR. (AS REQ'D)</p> <p>SHEATHING PER PLAN</p> | 9 | SCALE: NONE |  <p>SHEATHING PER PLAN</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>JOIST BLKG. w/ 8d BOX NAILS @ 8" O.C. TO BEAM BELOW</p> <p>JOIST PER PLAN</p> <p>WEB STIFFENER PER MFR. (AS REQ'D)</p> <p>(2) 8d BOX NAILS FROM EACH JOIST TO BEAM</p> <p>BEAM PER PLAN</p> | 10 | SCALE: NONE |  <p>PANEL EDGE NAILING PER PLAN</p> <p>SHEATHING PER PLAN</p> <p>JOIST PER PLAN</p> <p>HANGER PER PLAN</p> <p>BEAM PER PLAN</p> | 11 | SCALE: NONE |  <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE</p> <p>PANEL EDGE NAILING PER PLAN TO BLKG., TYP.</p> <p>LSL BLKG. EACH SIDE @ 48" O.C.</p> <p>24" MIN. TOTAL LENGTH OF BLOCKING</p> <p>BOTTOM PLATE ATTACHMENT PER SHEAR WALL SCHEDULE (MIN. 16d NAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>SHEATHING PER PLAN</p> <p>JOIST PER PLAN</p> <p>BEAM PER PLAN</p> | 12 | SCALE: NONE |
| TYPICAL EXTERIOR WALL - I-JOIST PERPENDICULAR | 1 | SCALE: NONE | TYPICAL EXTERIOR WALL - I-JOIST PARALLEL | 2 | SCALE: NONE | TYPICAL INTERIOR WALL - I-JOIST PERPENDICULAR | 3 | SCALE: NONE | TYPICAL INTERIOR WALL - I-JOIST PARALLEL | 4 | SCALE: NONE |
| TYPICAL EXTERIOR WALL WITH DECK ATTACHED - I-JOIST PERPENDICULAR | 5 | SCALE: NONE | FLUSH BEAM AT DECK - I-JOIST PERPENDICULAR | 6 | SCALE: NONE | EXTERIOR WALL AT FLOOR OPENING - I-JOIST | 7 | SCALE: NONE | DETAIL | 8 | SCALE: NONE |
| TYPICAL CANTILEVER JOIST AT EXTERIOR WALL - I-JOIST | 9 | SCALE: NONE | TYPICAL I-JOIST TO DROP BEAM CONNECTION | 10 | SCALE: NONE | TYPICAL I-JOIST TO FLUSH BEAM CONNECTION | 11 | SCALE: NONE | TYPICAL STRUCTURAL WALL TO PARALLEL BEAM BELOW - I-JOIST PARALLEL | 12 | SCALE: NONE |

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5218 16th Avenue NE, Seattle, WA 98105

REVISIONS

| NO. | DESCRIPTION | DATE |
|-----------|-----------------|-----------|
| 201906.01 | Project number | 6/29/2020 |
| 6/29/2020 | Date | SKK |
| | Project Manager | SC |
| | Drawn by | TON |
| | Checked by | |

PERMIT SET

6/29/2020

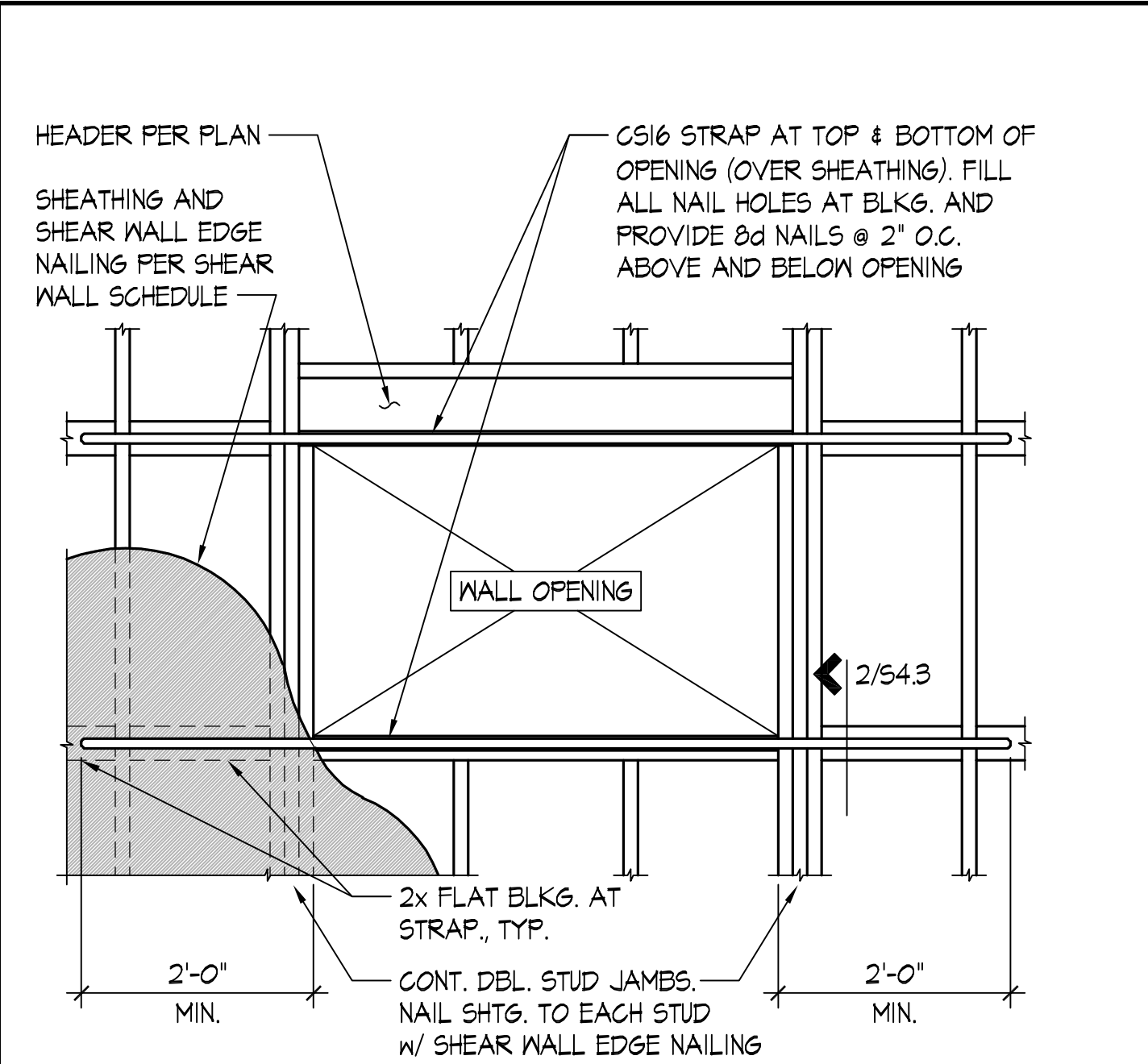
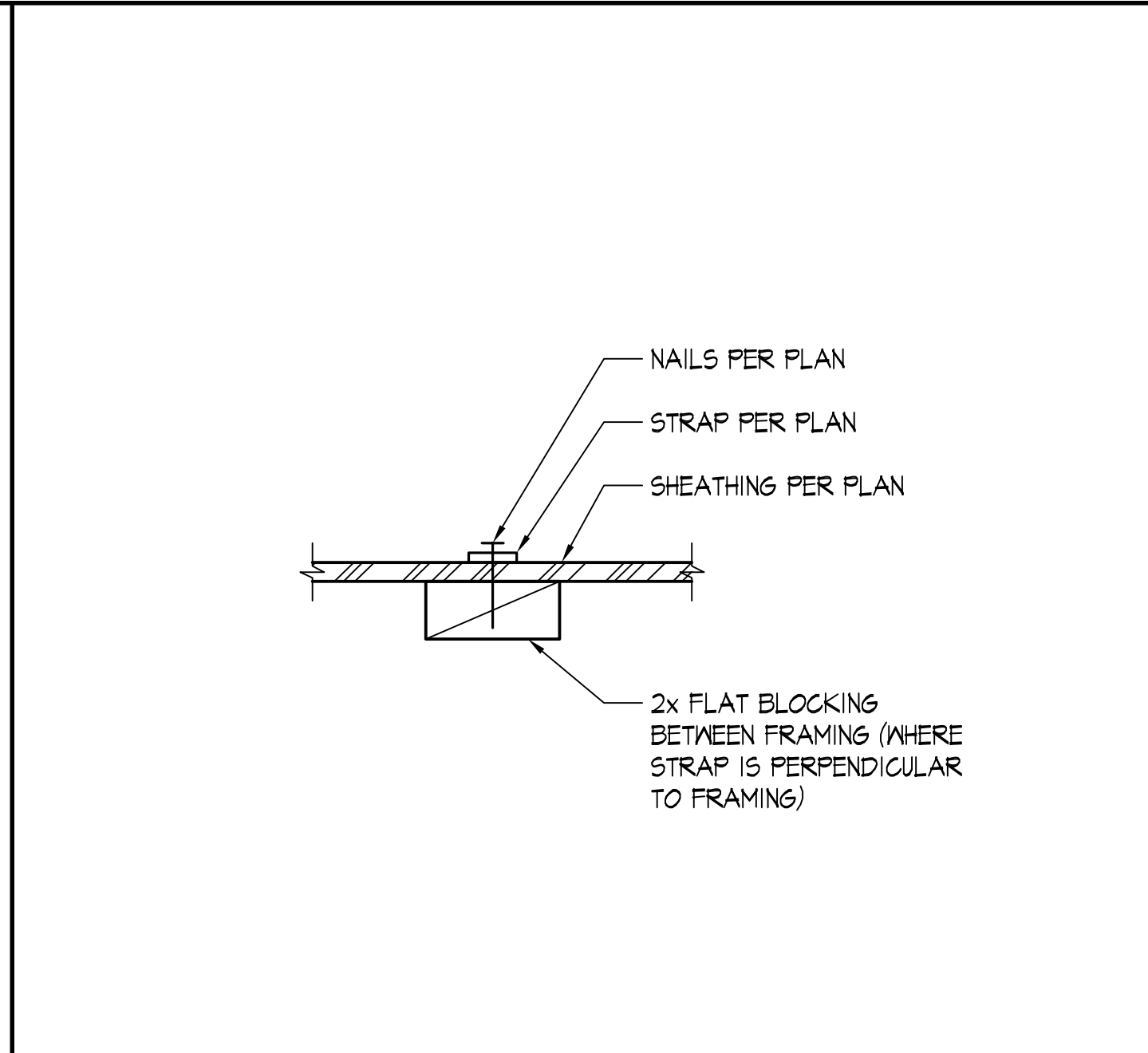

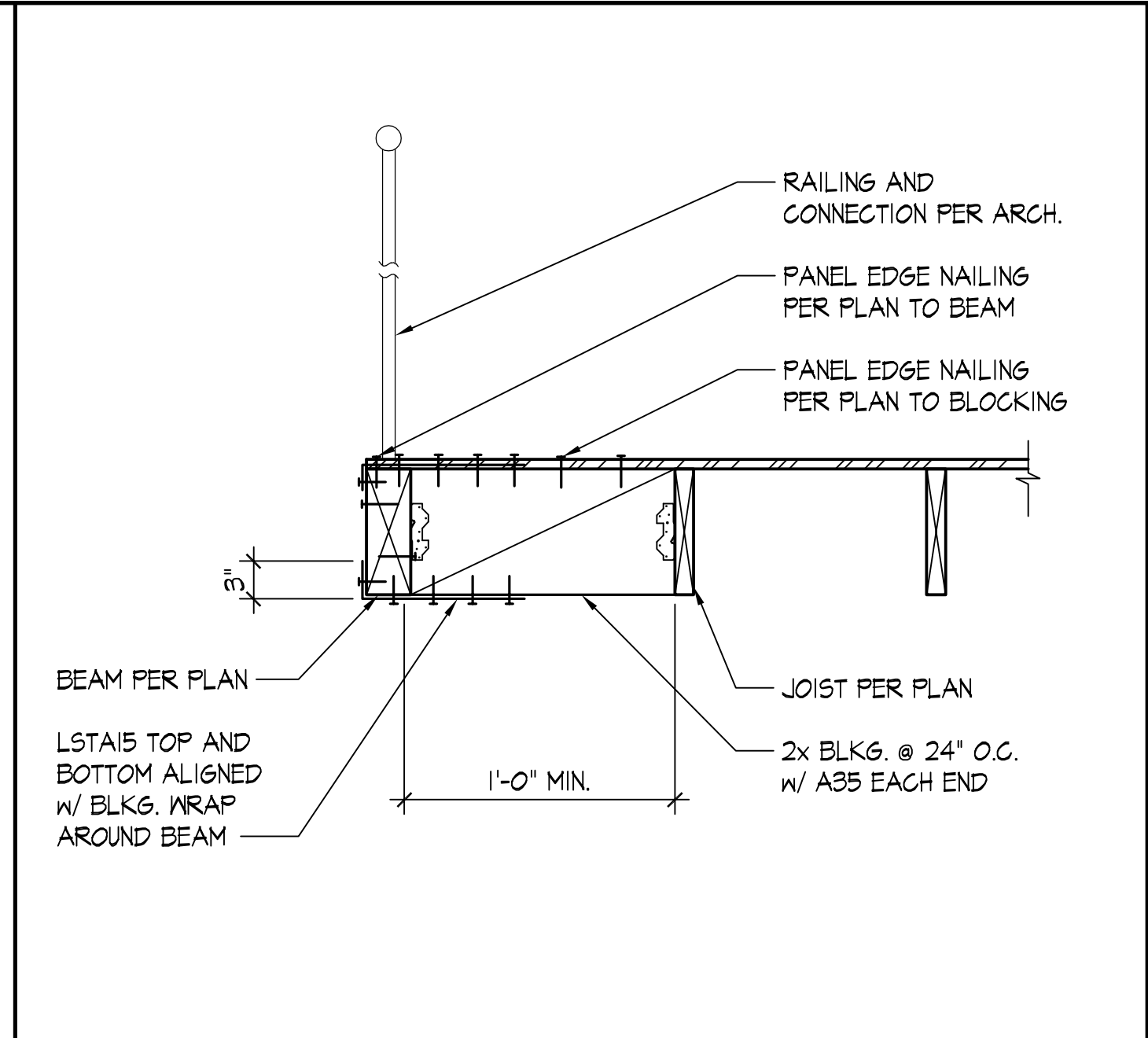



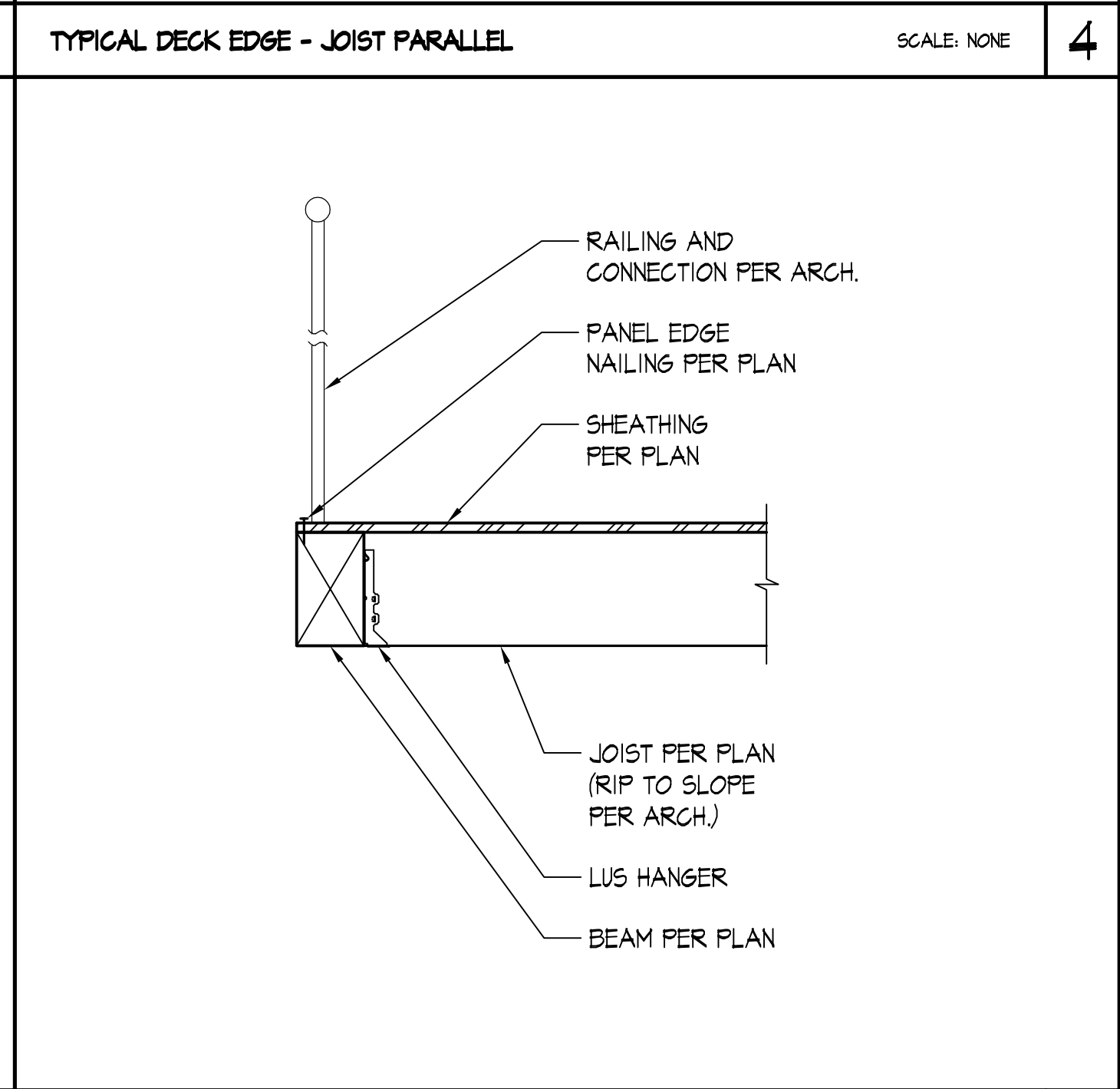
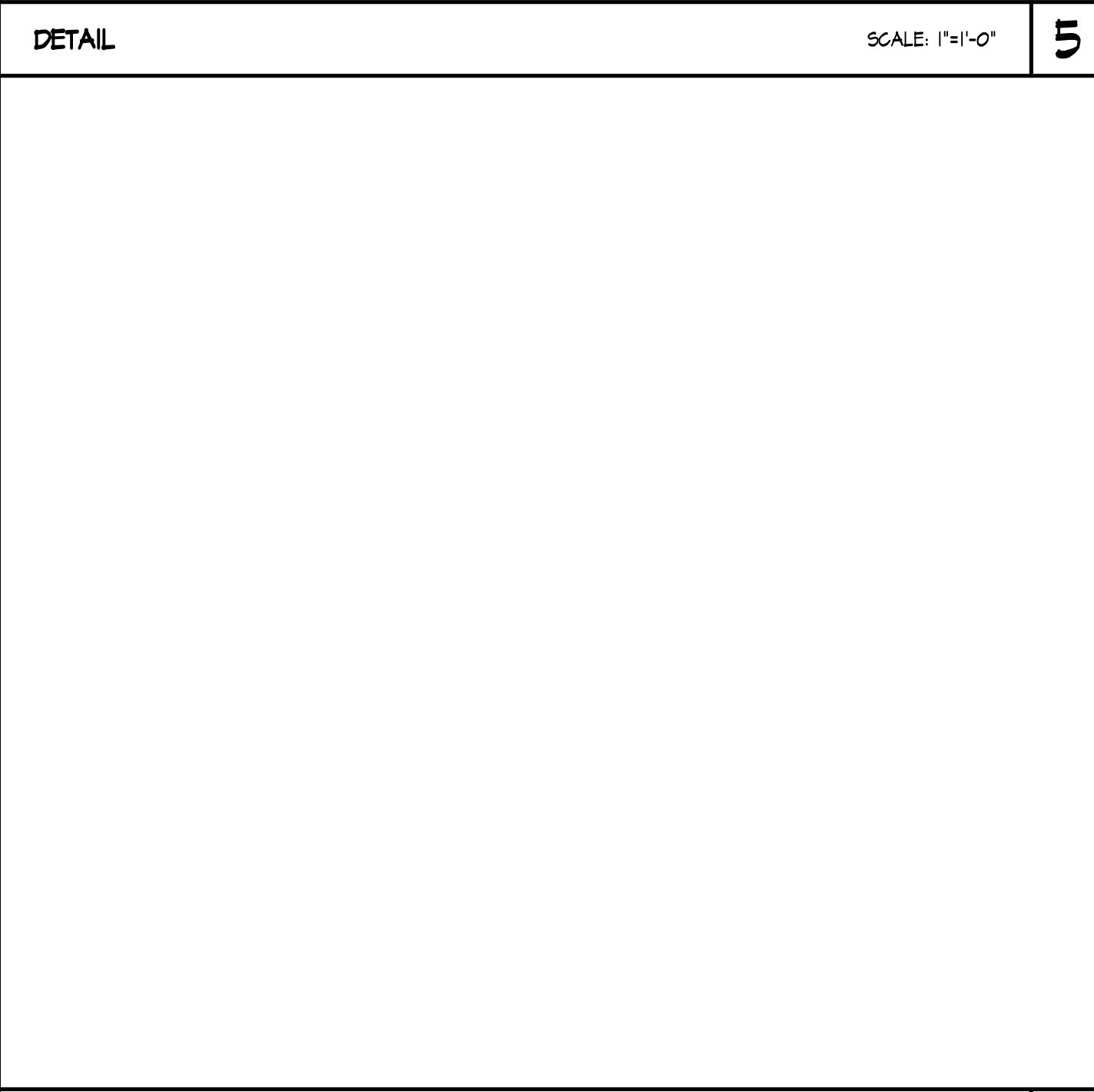
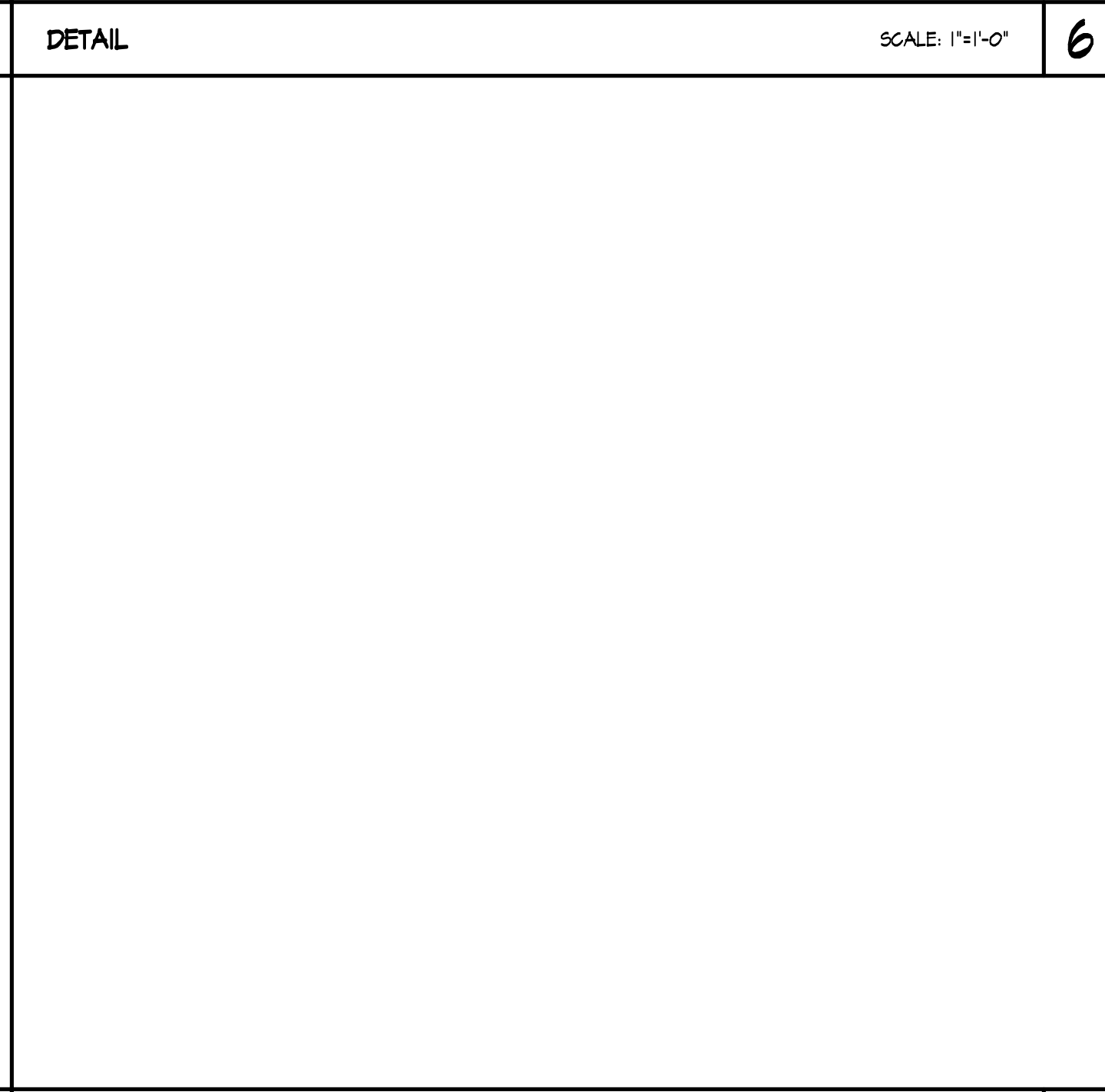
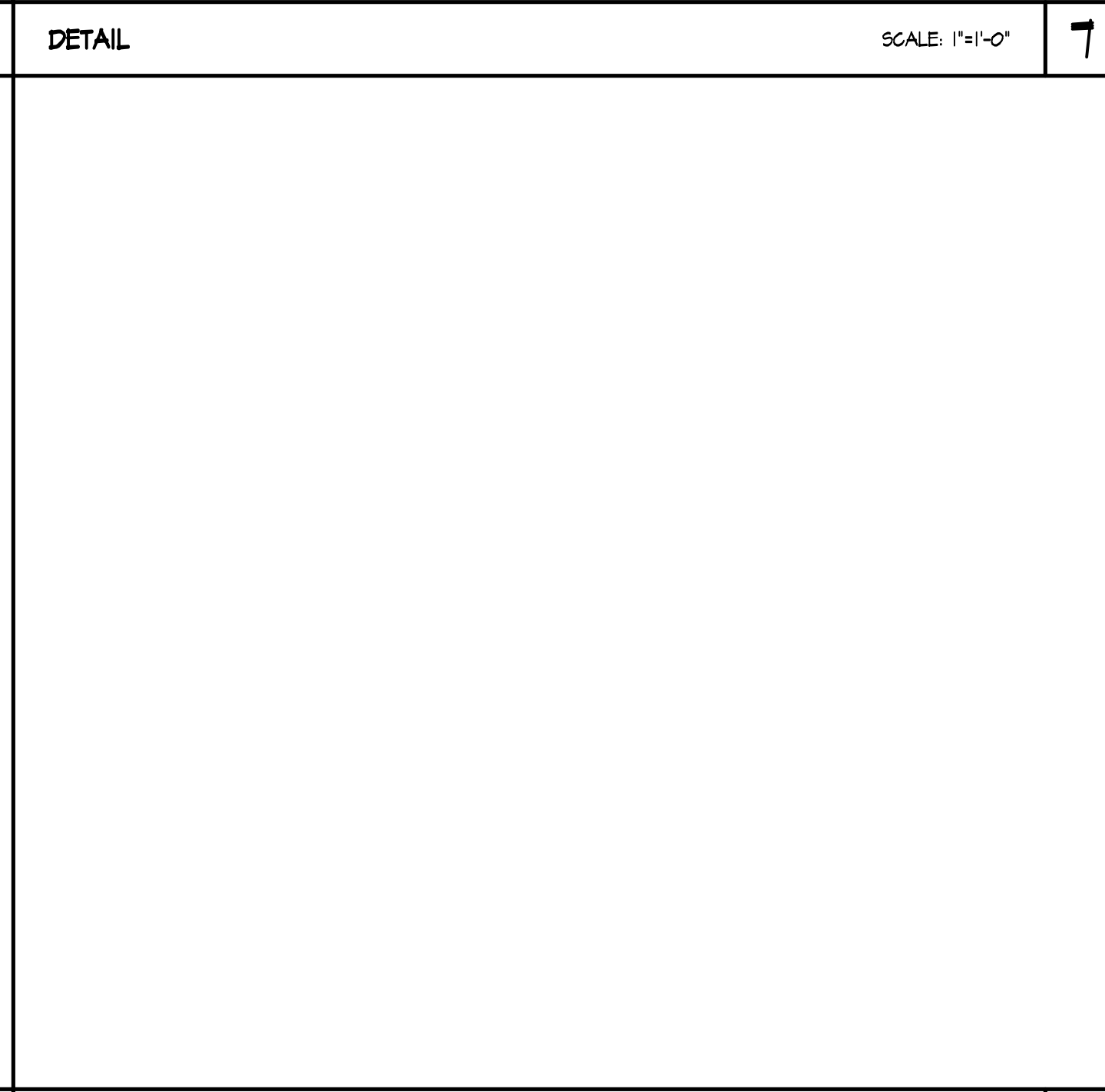
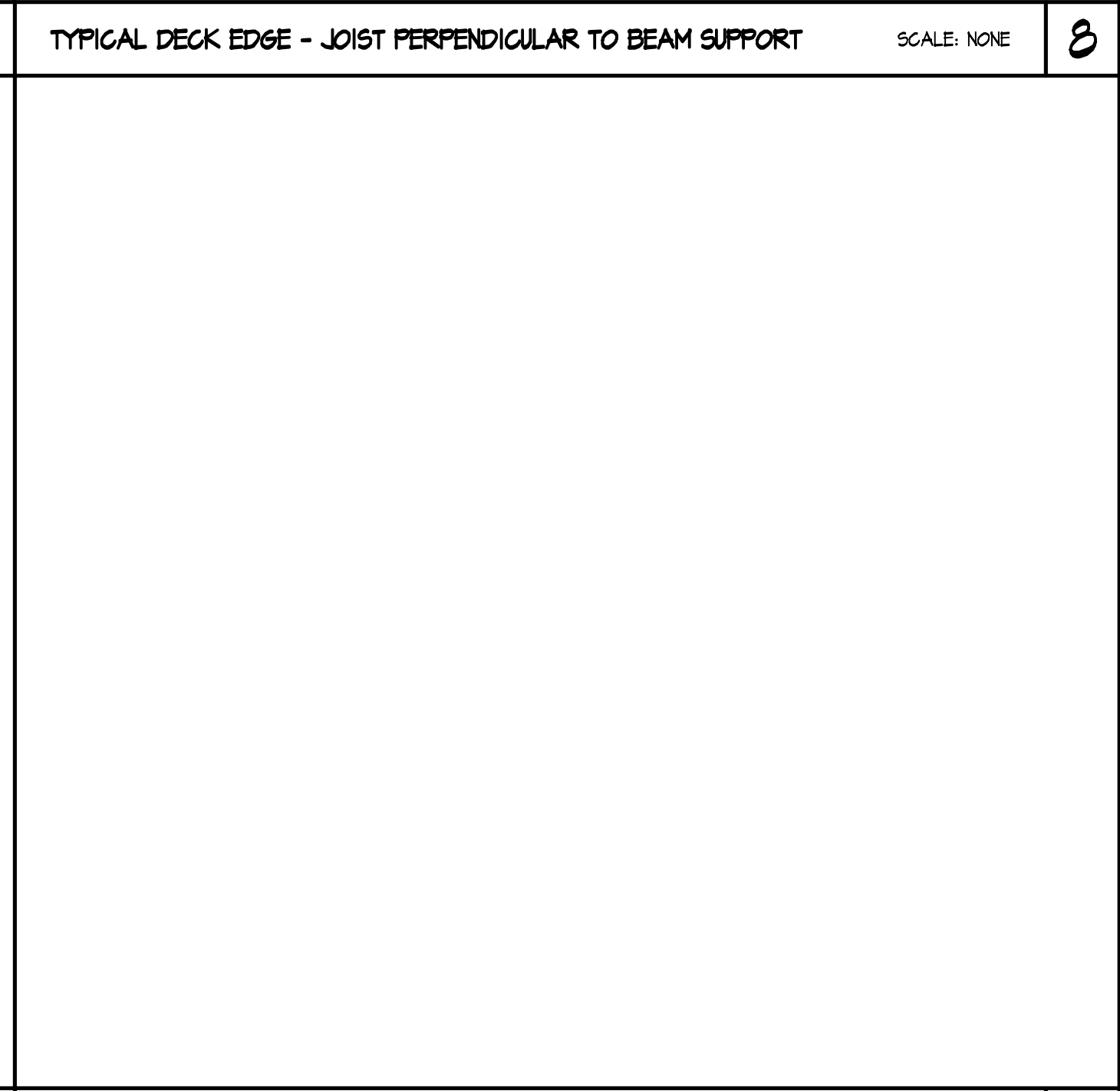
DETAILS

S4.2

Scale

AS NOTED

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|  | |  | |  | |  | | | | | | | | | |
| STRAPPING AROUND SHEAR WALL OPENING | | SCALE: NONE | 1 | STRAP TO BLOCKING DETAIL | | SCALE: NONE | 2 | DETAIL | | SCALE: 1"=1'-0" | 3 | TYPICAL DECK EDGE - JOIST PARALLEL | | SCALE: NONE | 4 |
|  | |  | |  | |  | | | | | | | | | |
| DETAIL | | SCALE: 1"=1'-0" | 5 | DETAIL | | SCALE: 1"=1'-0" | 6 | DETAIL | | SCALE: 1"=1'-0" | 7 | TYPICAL DECK EDGE - JOIST PERPENDICULAR TO BEAM SUPPORT | | SCALE: NONE | 8 |
|  | |  | |  | |  | | | | | | | | | |
| DETAIL | | SCALE: 1"=1'-0" | 9 | DETAIL | | SCALE: 1"=1'-0" | 10 | DETAIL | | SCALE: 1"=1'-0" | 11 | DETAIL | | SCALE: 1"=1'-0" | 12 |

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REVISIONS

| NO. | DESCRIPTION | DATE |
|-----|-------------|-----------|
| 1 | PERMIT SET | 6/29/2020 |

DETAILS

Project number
20196.01

Date
6/29/2020

Project Manager
SKK

Drawn by
SC

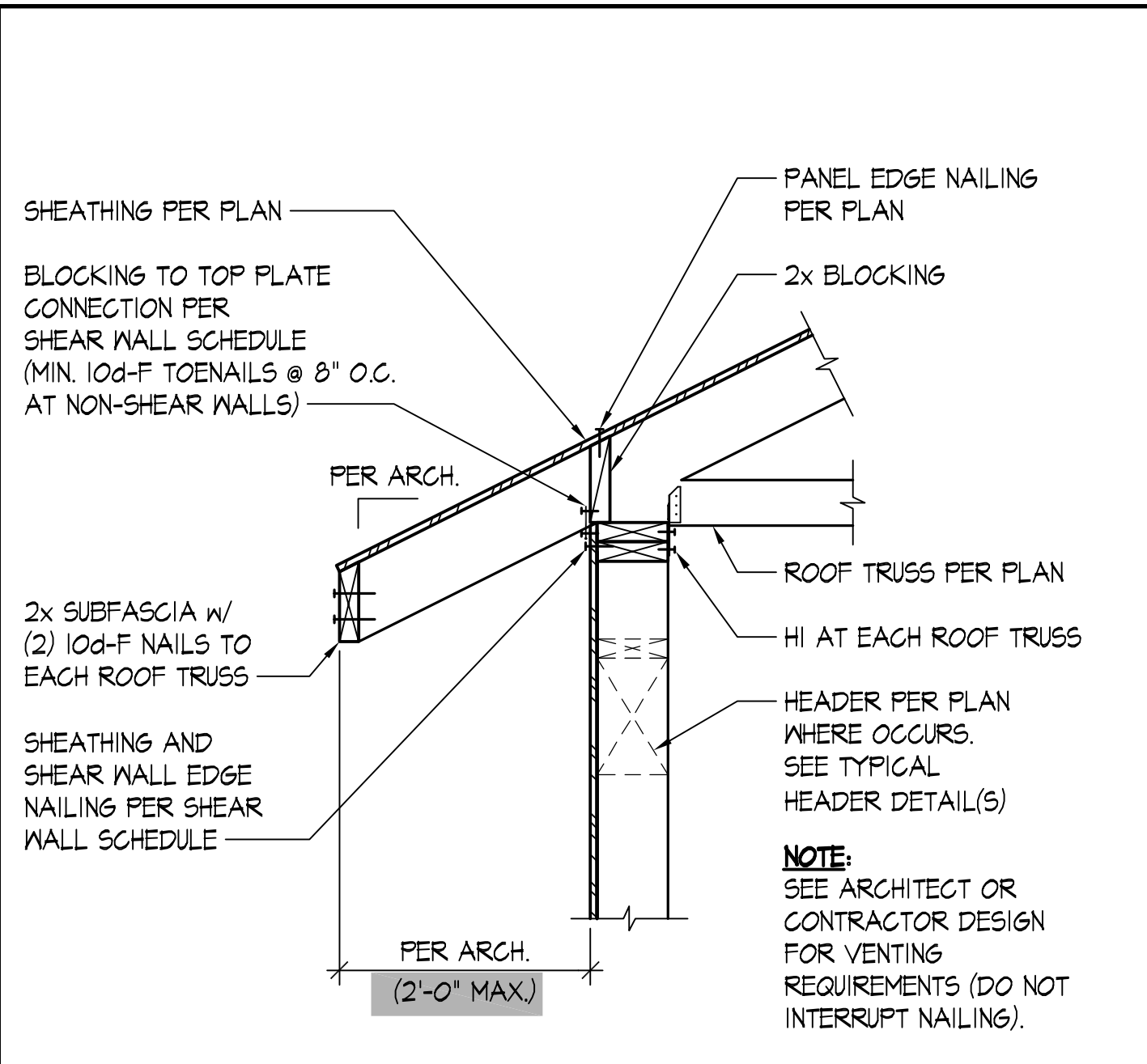
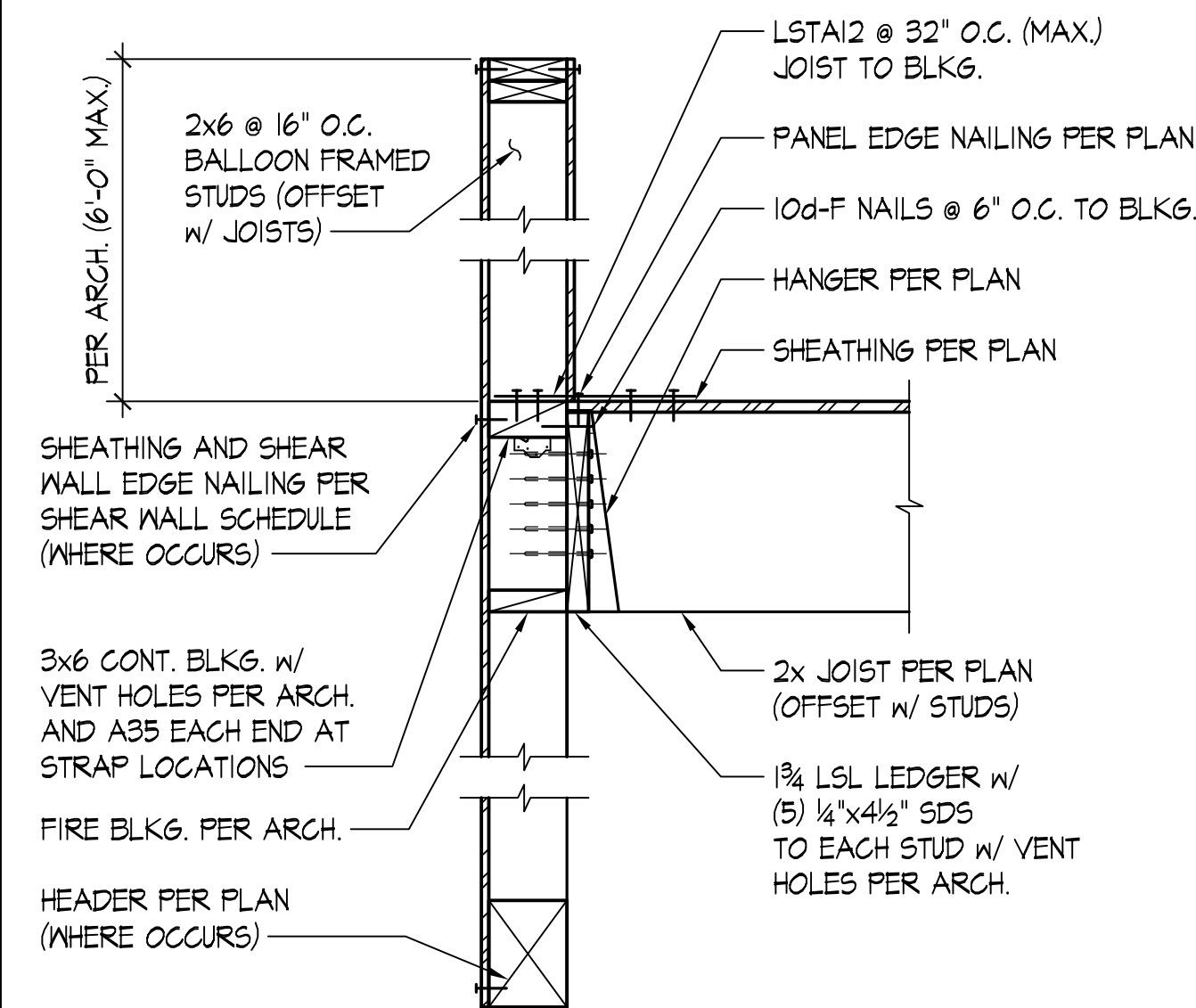
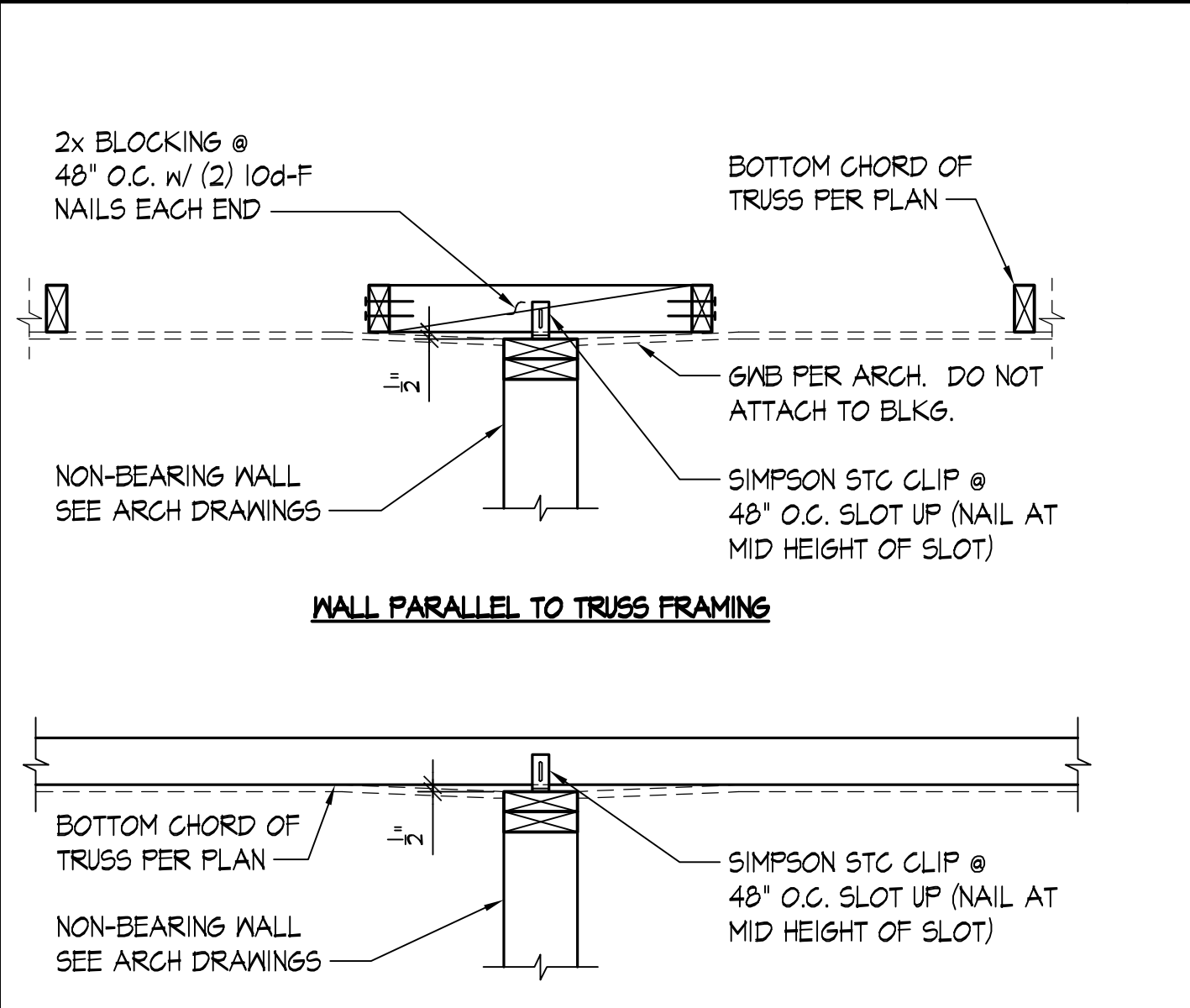
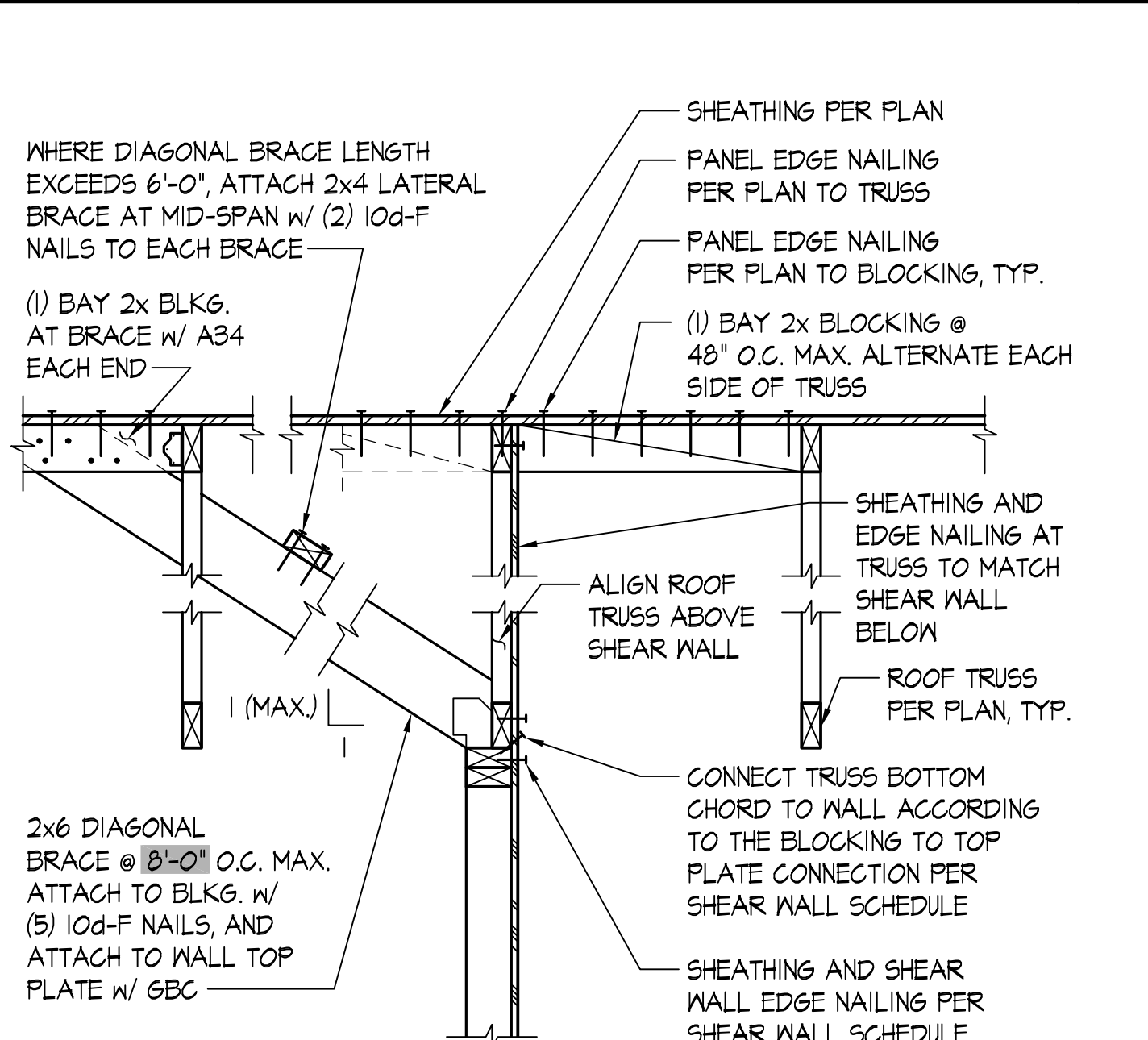
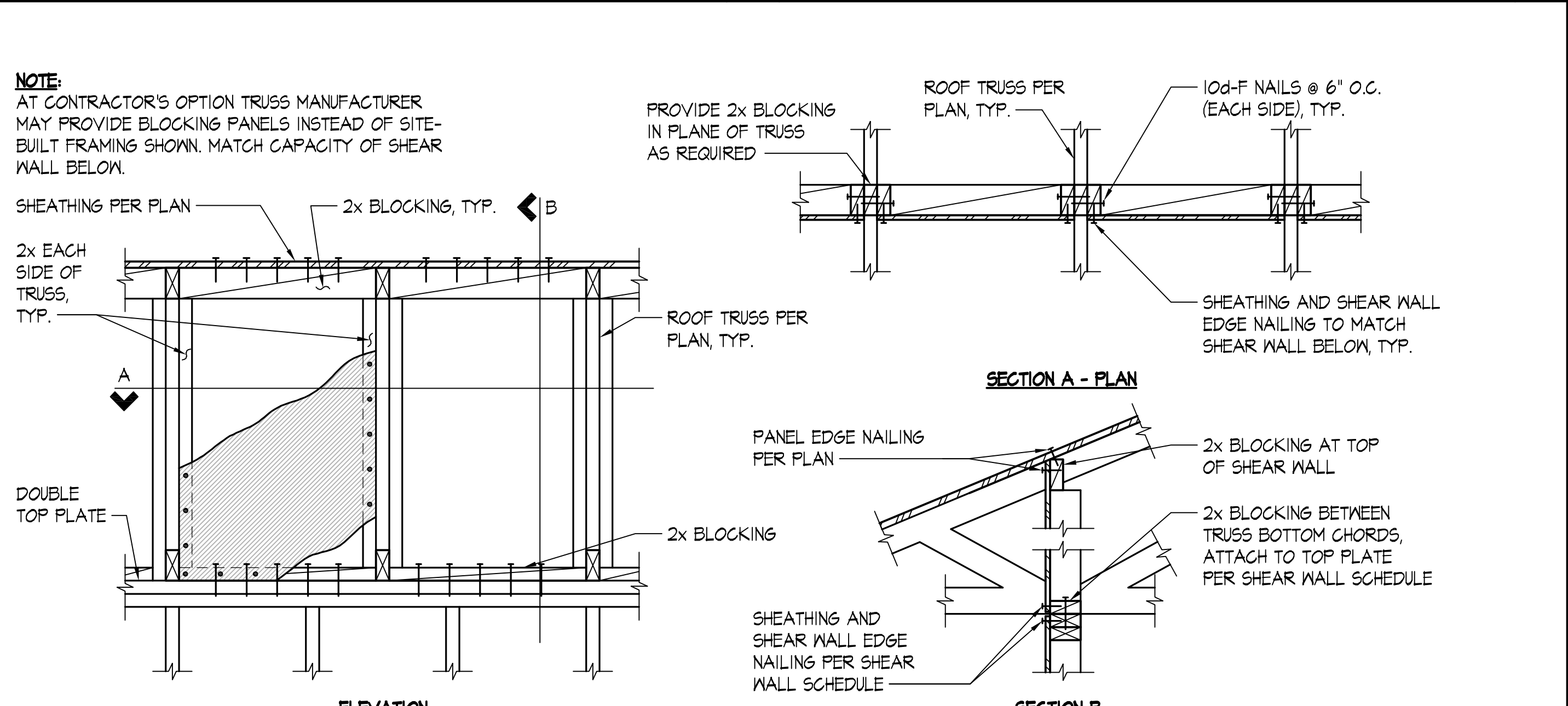
Checked by
TON

AS NOTED

S4.3

Scale

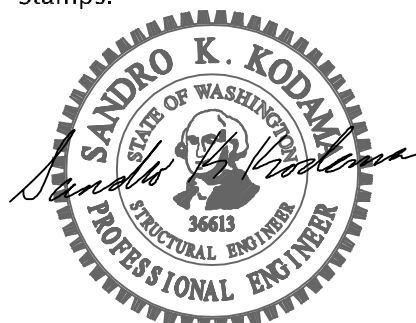
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|--|--|-----------------|---|--|--|---|----|--|--|---|----|---|--|-----------------|---|
|  <p>SHEATHING PER PLAN</p> <p>BLOCKING TO TOP PLATE CONNECTION PER SHEAR WALL SCHEDULE (MIN. 10d-F TOENAILS @ 8" O.C. AT NON-SHEAR WALLS)</p> <p>PER ARCH.</p> <p>2x SUBFASCIA w/ (2) 10d-F NAILS TO EACH ROOF TRUSS</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>2x BLOCKING</p> <p>ROOF TRUSS PER PLAN</p> <p>HI AT EACH ROOF TRUSS</p> <p>HEADER PER PLAN WHERE OCCURS. SEE TYPICAL HEADER DETAIL(S)</p> <p>NOTE: SEE ARCHITECT OR CONTRACTOR DESIGN FOR VENTING REQUIREMENTS (DO NOT INTERRUPT NAILING).</p> <p>PER ARCH. (2'-0" MAX.)</p> | | | | | | | | | | | | | | | |
| TYPICAL ROOF TRUSS TO EXTERIOR WALL - TRUSS PERPENDICULAR | | SCALE: NONE | 1 | DETAIL | | SCALE: 1"=1'-0" | 2 | DETAIL | | SCALE: 1"=1'-0" | 3 | DETAIL | | SCALE: 1"=1'-0" | 4 |
| | | | | | | | | | |  <p>PER ARCH. (6'-0" MAX.)</p> <p>2x6 @ 16" O.C. BALLOON FRAMED STUDS (OFFSET w/ JOISTS)</p> <p>LSTAI2 @ 32" O.C. (MAX.) JOIST TO BLKG.</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>10d-F NAILS @ 6" O.C. TO BLKG.</p> <p>HANGER PER PLAN</p> <p>SHEATHING PER PLAN</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE (WHERE OCCURS)</p> <p>3x6 CONT. BLKG. w/ VENT HOLES PER ARCH. AND A35 EACH END AT STRAP LOCATIONS</p> <p>FIRE BLKG. PER ARCH.</p> <p>HEADER PER PLAN (WHERE OCCURS)</p> <p>2x JOIST PER PLAN (OFFSET w/ STUDS)</p> <p>1 3/4 LSL LEDGER w/ (5) 1/4"x4 1/2" SDS TO EACH STUD w/ VENT HOLES PER ARCH.</p> | | | | | |
| DETAIL | | SCALE: 1"=1'-0" | 5 | DETAIL | | SCALE: 1"=1'-0" | 6 | DETAIL | | SCALE: 1"=1'-0" | 7 | BALLOON FRAMED PARAPET - 2x JOIST PERPENDICULAR | | SCALE: NONE | 8 |
|  <p>2x BLOCKING @ 48" O.C. w/ (2) 10d-F NAILS EACH END</p> <p>BOTTOM CHORD OF TRUSS PER PLAN</p> <p>NON-BEARING WALL SEE ARCH DRAWINGS</p> <p>6x6 PER ARCH. DO NOT ATTACH TO BLKG.</p> <p>SIMPSON STC CLIP @ 48" O.C. SLOT UP (NAIL AT MID HEIGHT OF SLOT)</p> <p>WALL PARALLEL TO TRUSS FRAMING</p> <p>BOTTOM CHORD OF TRUSS PER PLAN</p> <p>NON-BEARING WALL SEE ARCH DRAWINGS</p> <p>SIMPSON STC CLIP @ 48" O.C. SLOT UP (NAIL AT MID HEIGHT OF SLOT)</p> <p>WALL PERP. TO TRUSS FRAMING</p> | | | |  <p>WHERE DIAGONAL BRACE LENGTH EXCEEDS 6'-0", ATTACH 2x4 LATERAL BRACE AT MID-SPAN w/ (2) 10d-F NAILS TO EACH BRACE</p> <p>(1) BAY 2x BLKG. AT BRACE w/ A34 EACH END</p> <p>SHEATHING PER PLAN</p> <p>PANEL EDGE NAILING PER PLAN TO TRUSS</p> <p>PANEL EDGE NAILING PER PLAN TO BLOCKING, TYP.</p> <p>(1) BAY 2x BLOCKING @ 48" O.C. MAX. ALTERNATE EACH SIDE OF TRUSS</p> <p>ALIGN ROOF TRUSS ABOVE SHEAR WALL</p> <p>SHEATHING AND EDGE NAILING AT TRUSS TO MATCH SHEAR WALL BELOW</p> <p>ROOF TRUSS PER PLAN, TYP.</p> <p>2x6 DIAGONAL BRACE @ 8'-0" O.C. MAX. ATTACH TO BLKG. w/ (5) 10d-F NAILS, AND ATTACH TO WALL TOP PLATE w/ 6BC</p> <p>CONNECT TRUSS BOTTOM CHORD TO WALL ACCORDING TO THE BLOCKING TO TOP PLATE CONNECTION PER SHEAR WALL SCHEDULE</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE</p> | |  <p>NOTE: AT CONTRACTOR'S OPTION TRUSS MANUFACTURER MAY PROVIDE BLOCKING PANELS INSTEAD OF SITE-BUILT FRAMING SHOWN. MATCH CAPACITY OF SHEAR WALL BELOW.</p> <p>SHEATHING PER PLAN</p> <p>2x BLOCKING, TYP.</p> <p>2x EACH SIDE OF TRUSS, TYP.</p> <p>DOUBLE TOP PLATE</p> <p>ROOF TRUSS PER PLAN, TYP.</p> <p>PROVIDE 2x BLOCKING IN PLANE OF TRUSS AS REQUIRED</p> <p>10d-F NAILS @ 6" O.C. (EACH SIDE), TYP.</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING TO MATCH SHEAR WALL BELOW, TYP.</p> <p>SECTION A - PLAN</p> <p>PANEL EDGE NAILING PER PLAN</p> <p>2x BLOCKING AT TOP OF SHEAR WALL</p> <p>2x BLOCKING BETWEEN TRUSS BOTTOM CHORDS, ATTACH TO TOP PLATE PER SHEAR WALL SCHEDULE</p> <p>SHEATHING AND SHEAR WALL EDGE NAILING PER SHEAR WALL SCHEDULE</p> <p>SECTION B</p> <p>ELEVATION</p> | | | | | | | | | |
| TYPICAL NON-STRUCTURAL WALL SUPPORT (TOP) - TRUSS | | SCALE: NONE | 9 | TYPICAL ROOF TRUSS TO INTERIOR SHEAR WALL - TRUSS PARALLEL | | SCALE: NONE | 10 | TYPICAL ROOF TRUSS TO SHEAR WALL - TRUSS PERPENDICULAR | | SCALE: NONE | 12 | | | | |

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REVISIONS

| NO. | DESCRIPTION | DATE |
|-----------|-------------|-----------|
| 201906.01 | | 6/29/2020 |

PERMIT SET

6/29/2020

DETAILS

S4.4

Scale

AS NOTED

Project number
201906.01

Date
6/29/2020

Project Manager
SKK

Drawn by
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Checked by
TON