

LUMEN FIELD

800 OCCIDENTAL AVE S., SEATTLE, WASHINGTON

LUMEN FIELD SECURITY BOOTH

THESE DOCUMENTS ARE INTENDED FOR PERMIT REVIEW, LOCATION AND PLACEMENT OF THE NORTH AND SOUTH GUARD SHACKS

OCTOBER 20, 2026

DRAWING SET CONTAINS THE FOLLOWING DESIGN DOCUMENTS:
GENERAL INFORMATION
GUARD SHACK LAYOUT AND DETAILS

NOTE TO CONTRACTORS, SUB-CONTRACTORS, FABRICATORS, AND INSTALLERS:
PLEASE READ, REVIEW, AND REFER TO ALL DRAWINGS FOR COMPLETE BASIS OF DESIGN.

PARCEL NUMBER: 766620-4876

LEGAL DESCRIPTION:
LOTS 5-35, BLK 285 & LOTS 5-35, BLK 325, SEATTLE TIDE LANDS & VAC 3RD AVE S (VO # 10552) EXCEPT POR OF LOT 5, BLK 325 LYING N OF ADJUSTED LINE PER LBA # 9806721 & EXCEPT POR OF LOT 5, BLK 285, & VAC 3RD AVE S (VO # 10552) LYING N OF THE ADJUSTED LINE PER LBA # 9806720.

INDEX OF DRAWINGS

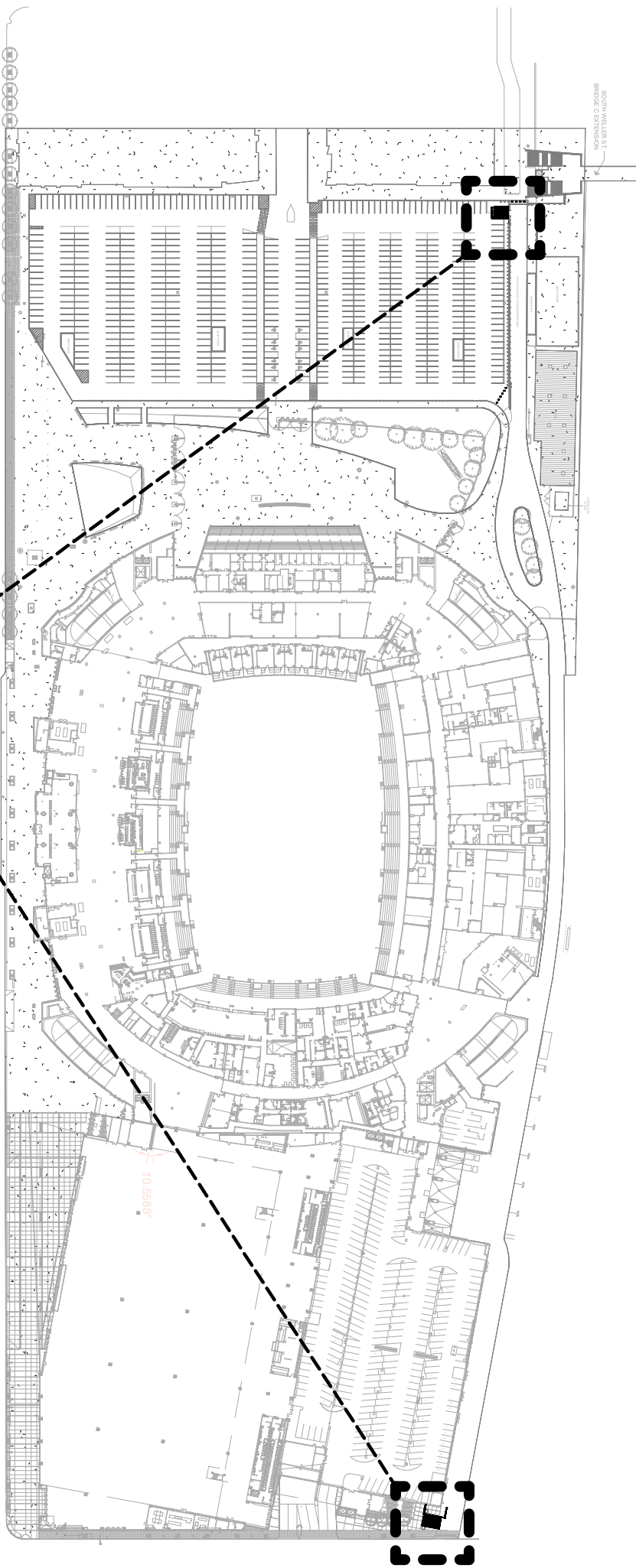
00 - GENERAL	
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CODE REVIEW INFORMATION

GOVERNING CODES:
2021 SEATTLE BUILDING CODE
2021 SEATTLE EXISTING BUILDING CODE
2021 SEATTLE ENERGY CODE
2021 NATIONAL ELECTRICAL CODE
2021 SEATTLE MECHANICAL CODE
2021 SEATTLE PLUMBING CODE
2021 SEATTLE FIRE CODE



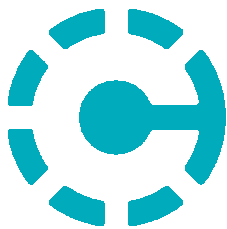
PROJECT
LOCATION



1 STADIUM REFERENCE PLAN
1" = 200'-0"



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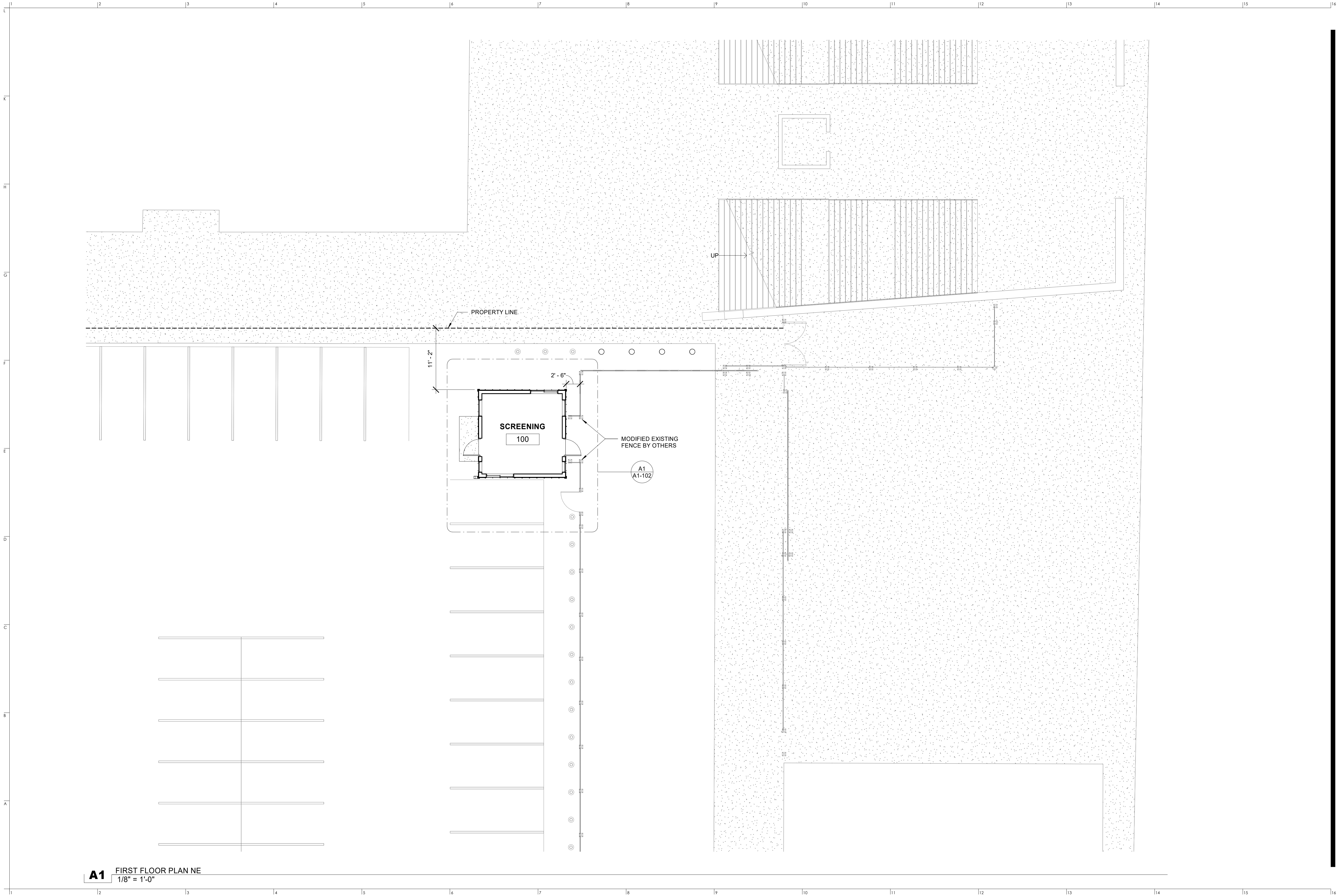
DRAWINGS ISSUED FOR:
Issued for Permit

title :

subcontract no. : 25050012-31541

G000

sheet no.
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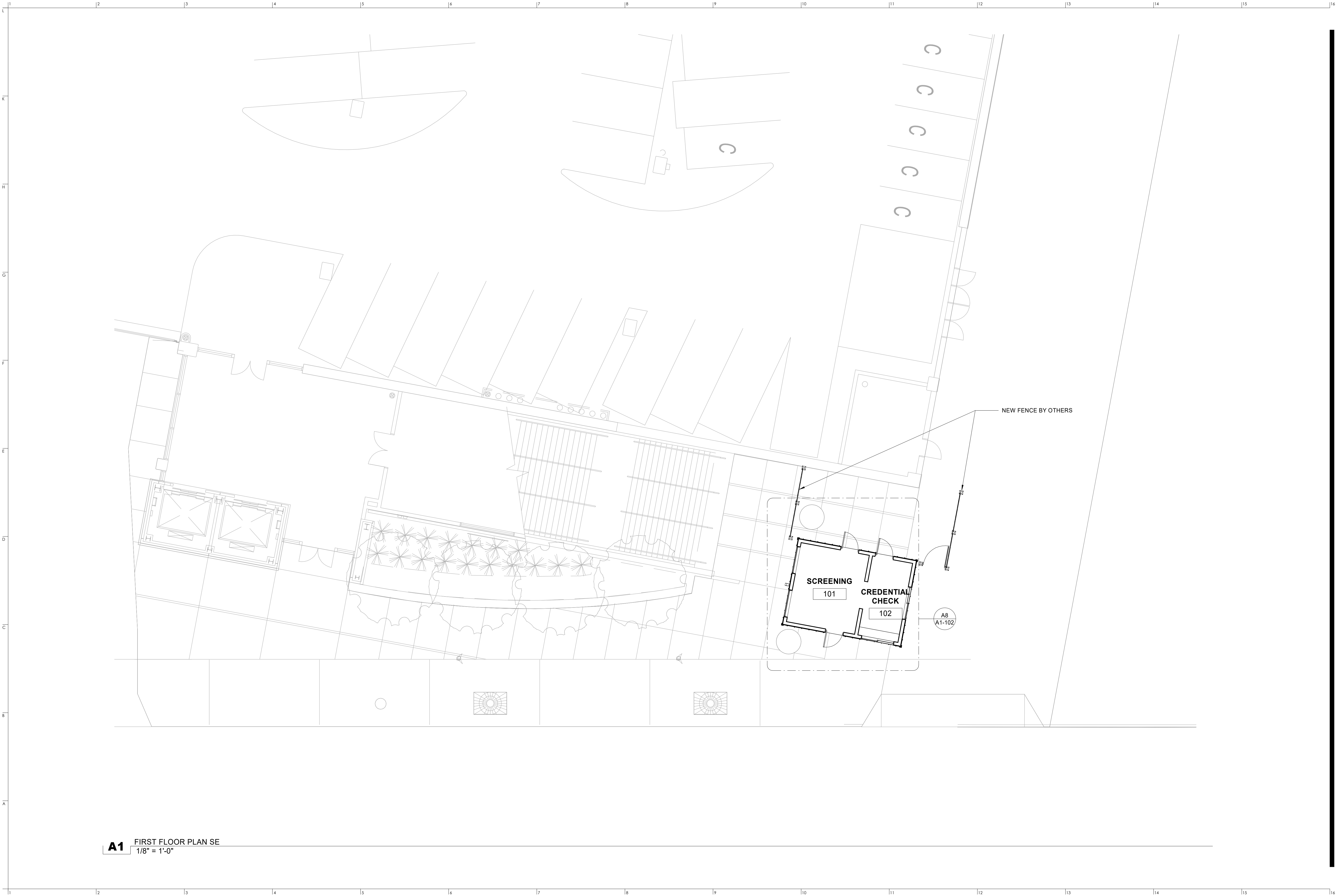
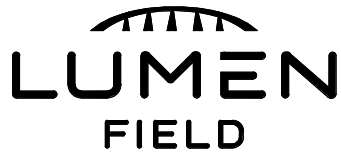
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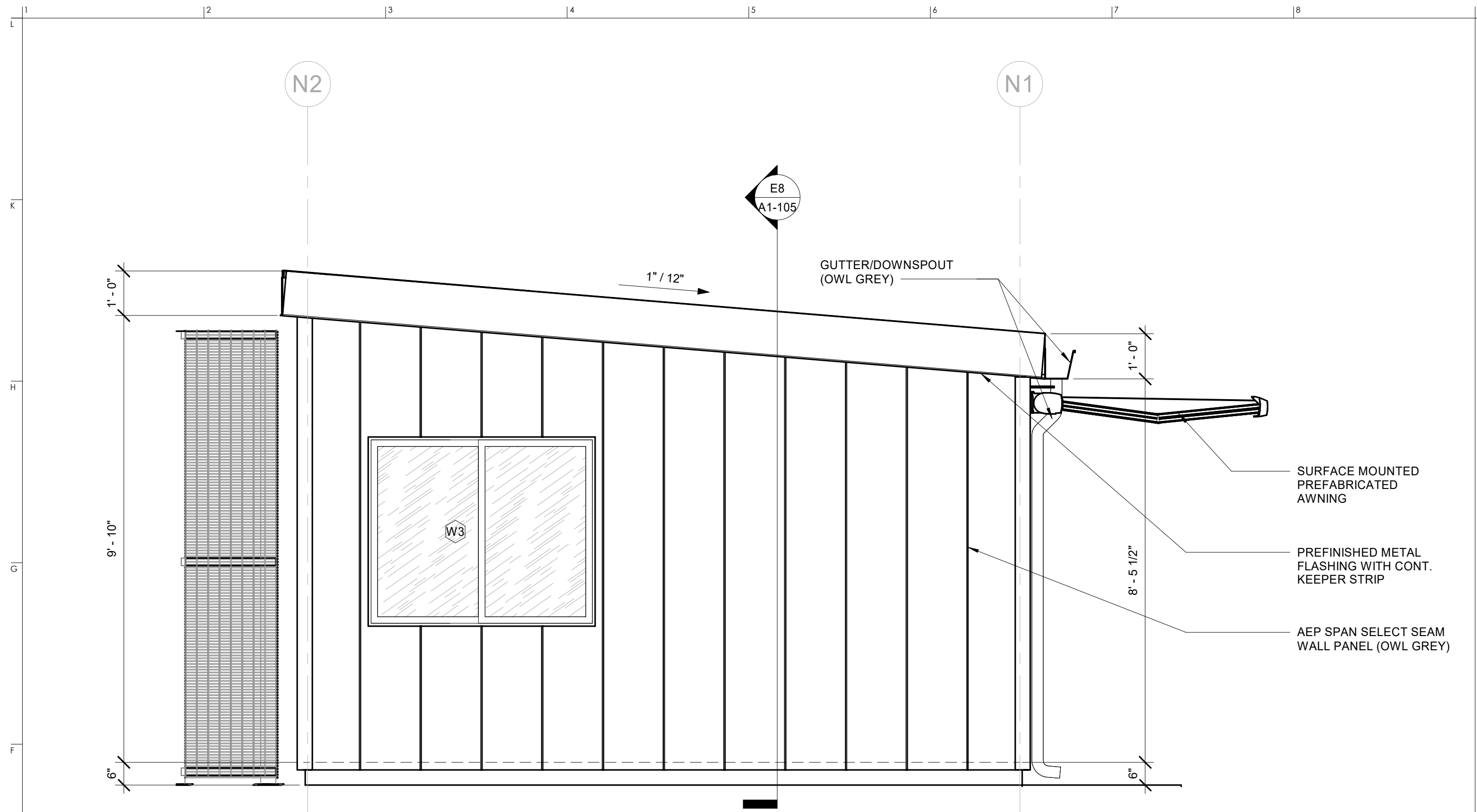
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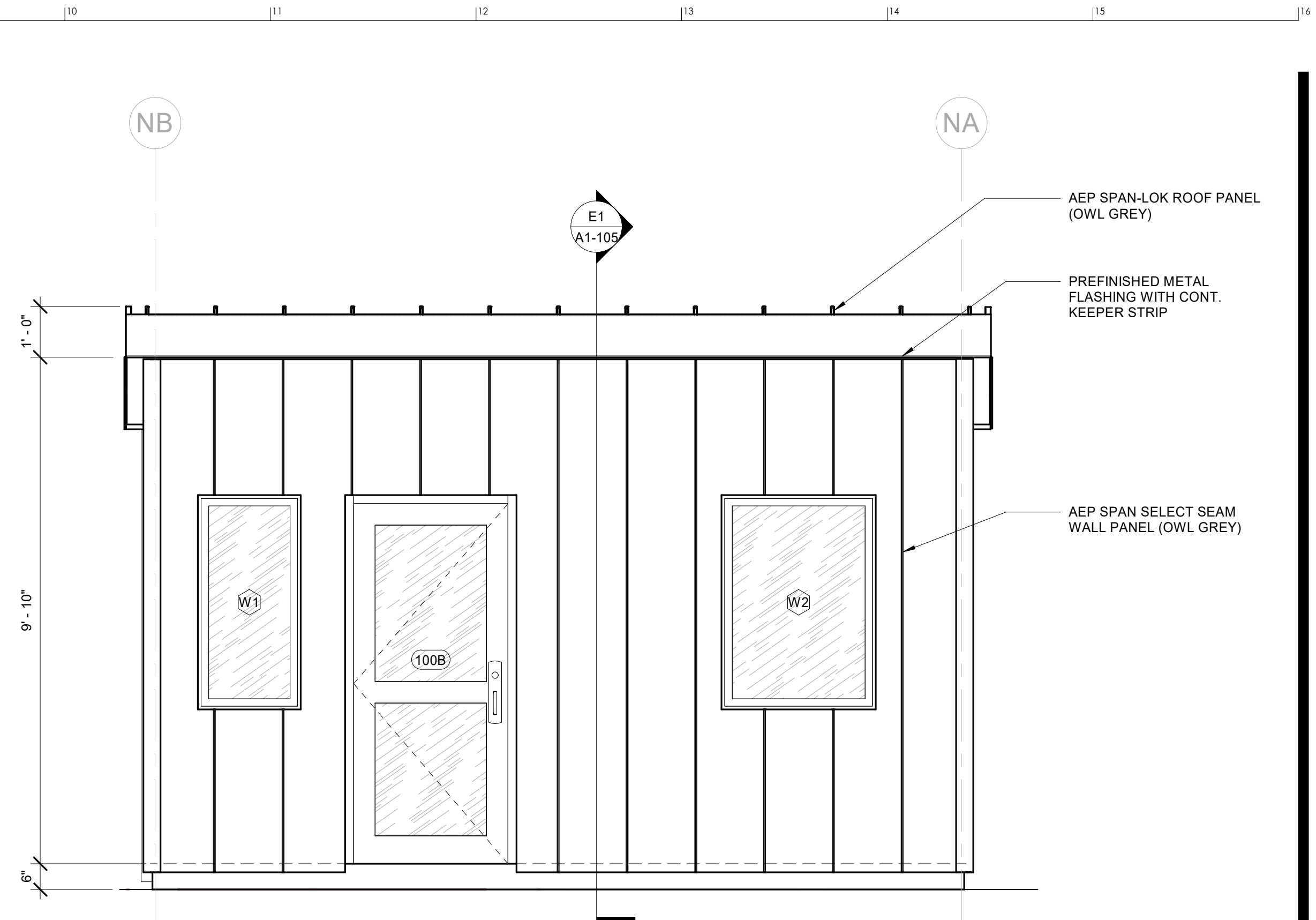


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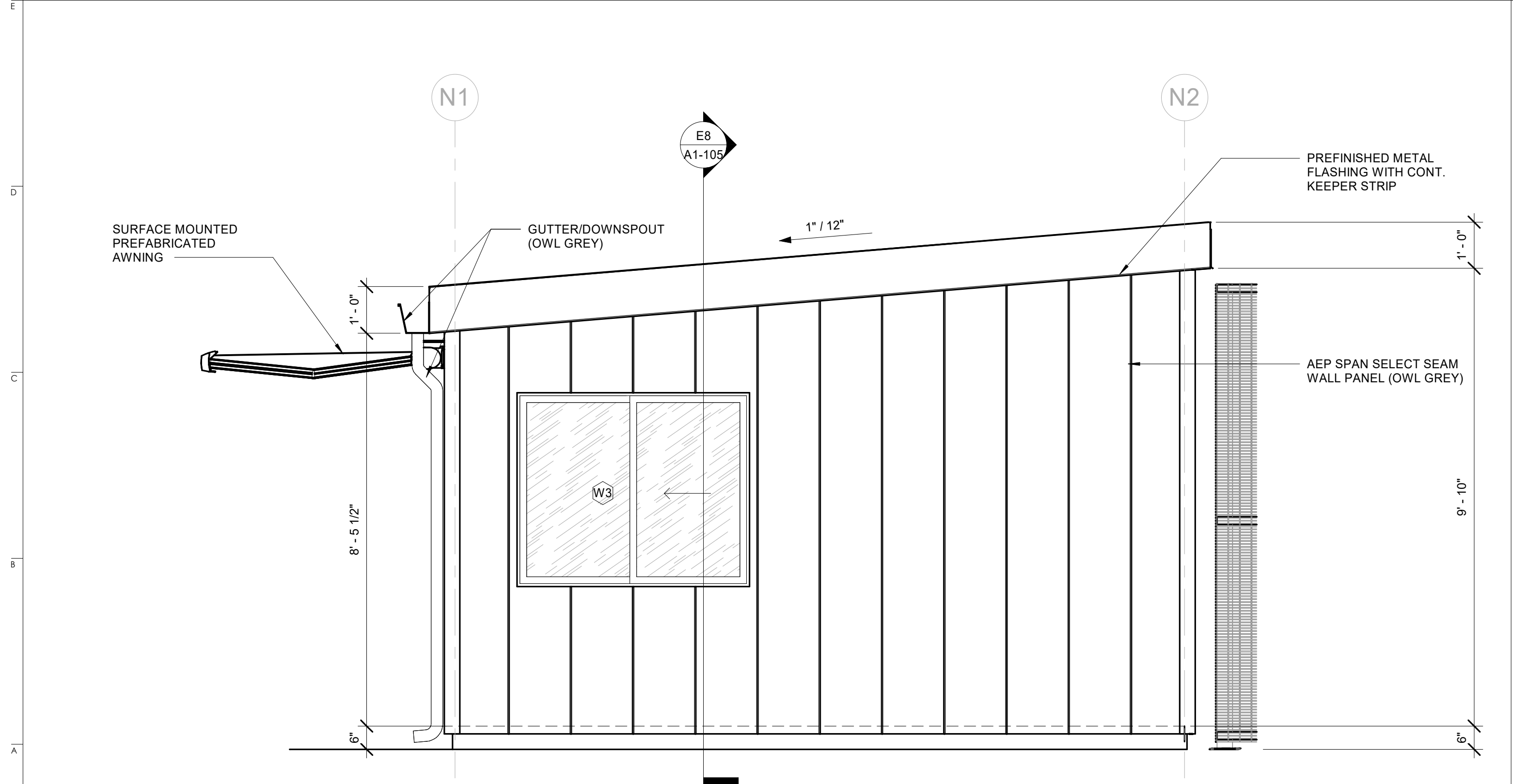
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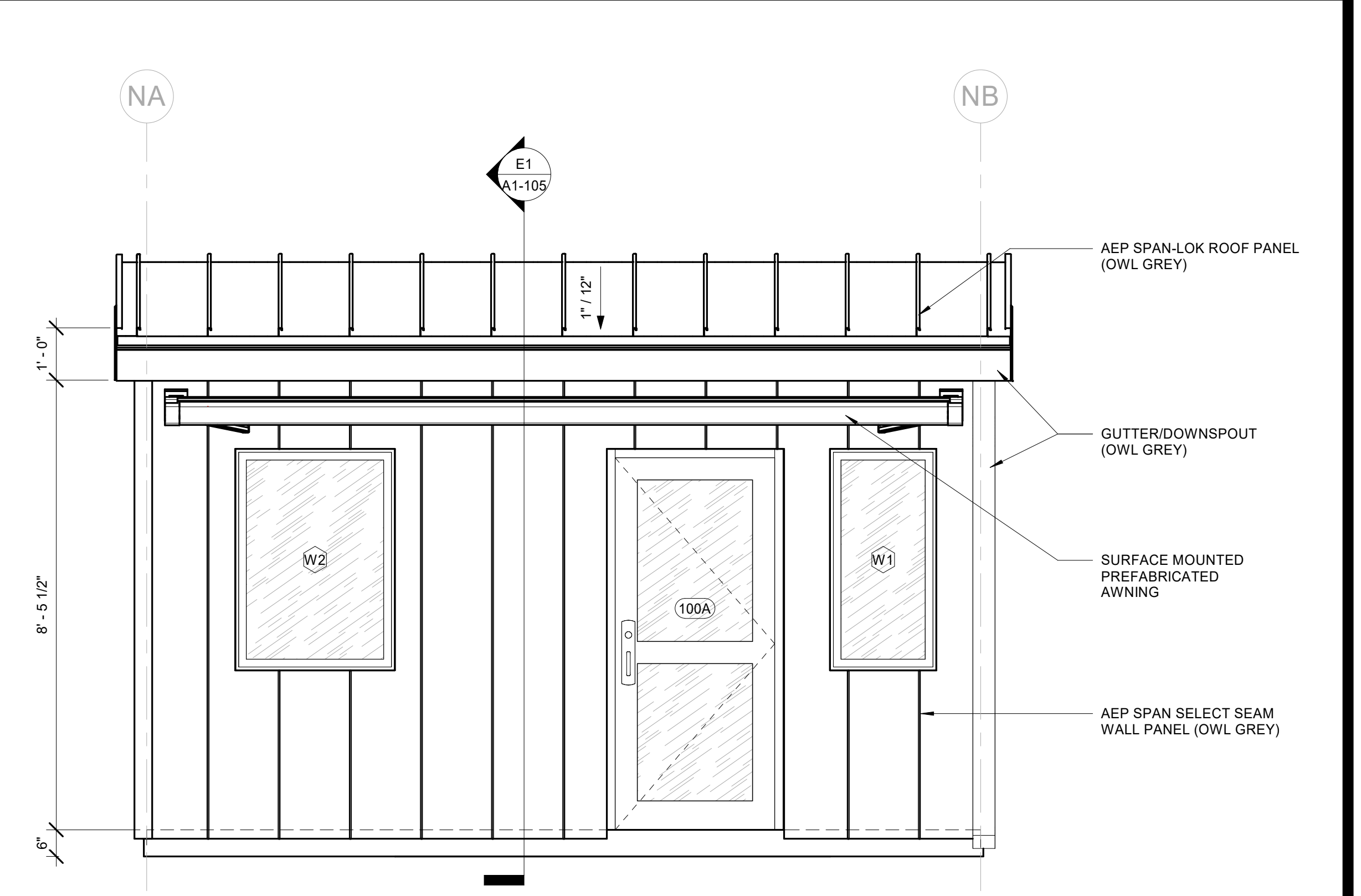
E1 NORTH ELEVATION - NORTH GUARD BOOTH
1/2" = 1'-0"



E9 EAST ELEVATION - NORTH GUARD BOOTH
1/2" = 1'-0"



A1 SOUTH ELEVATION - NORTH GUARD BOOTH
1/2" = 1'-0"

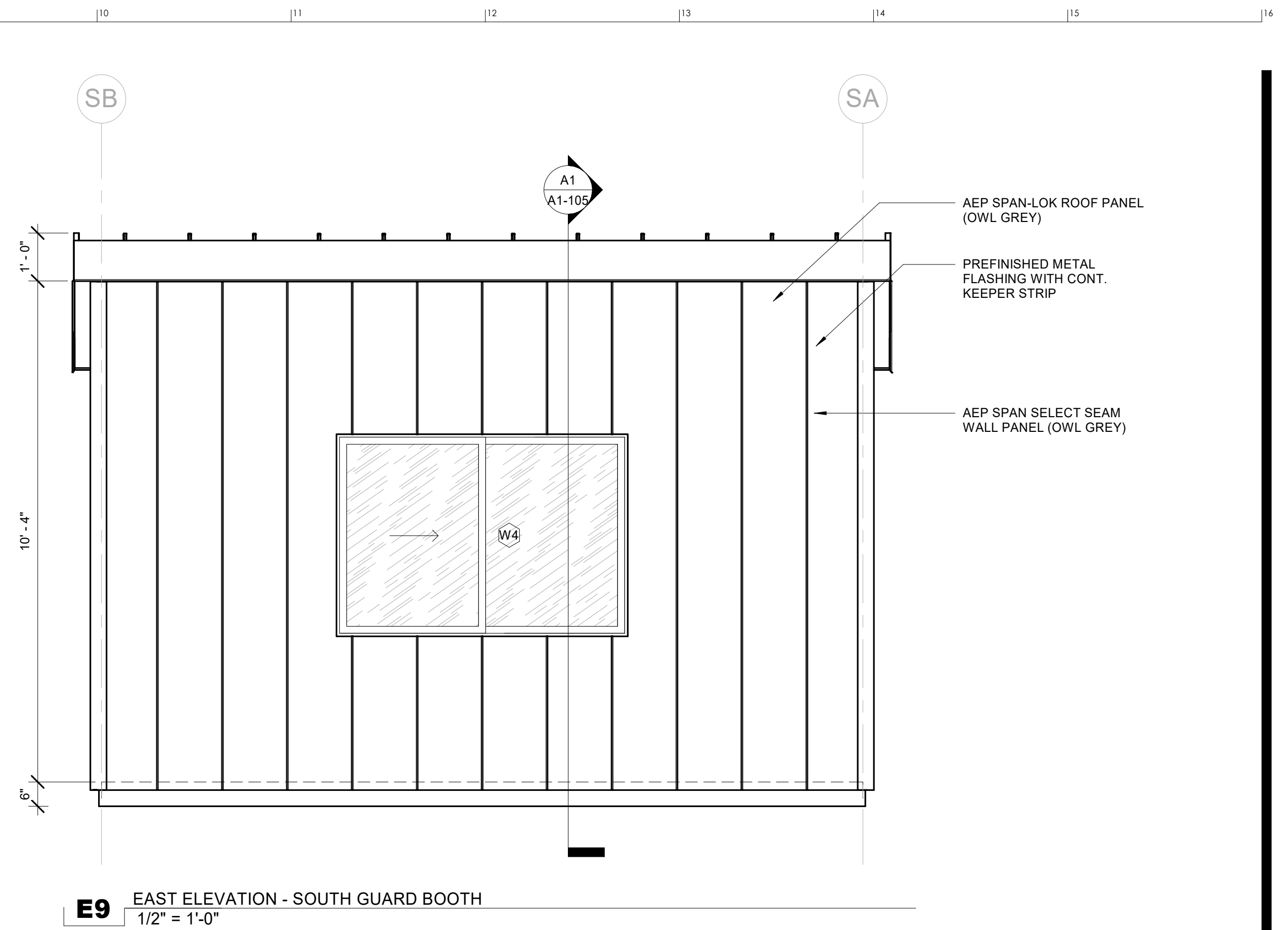
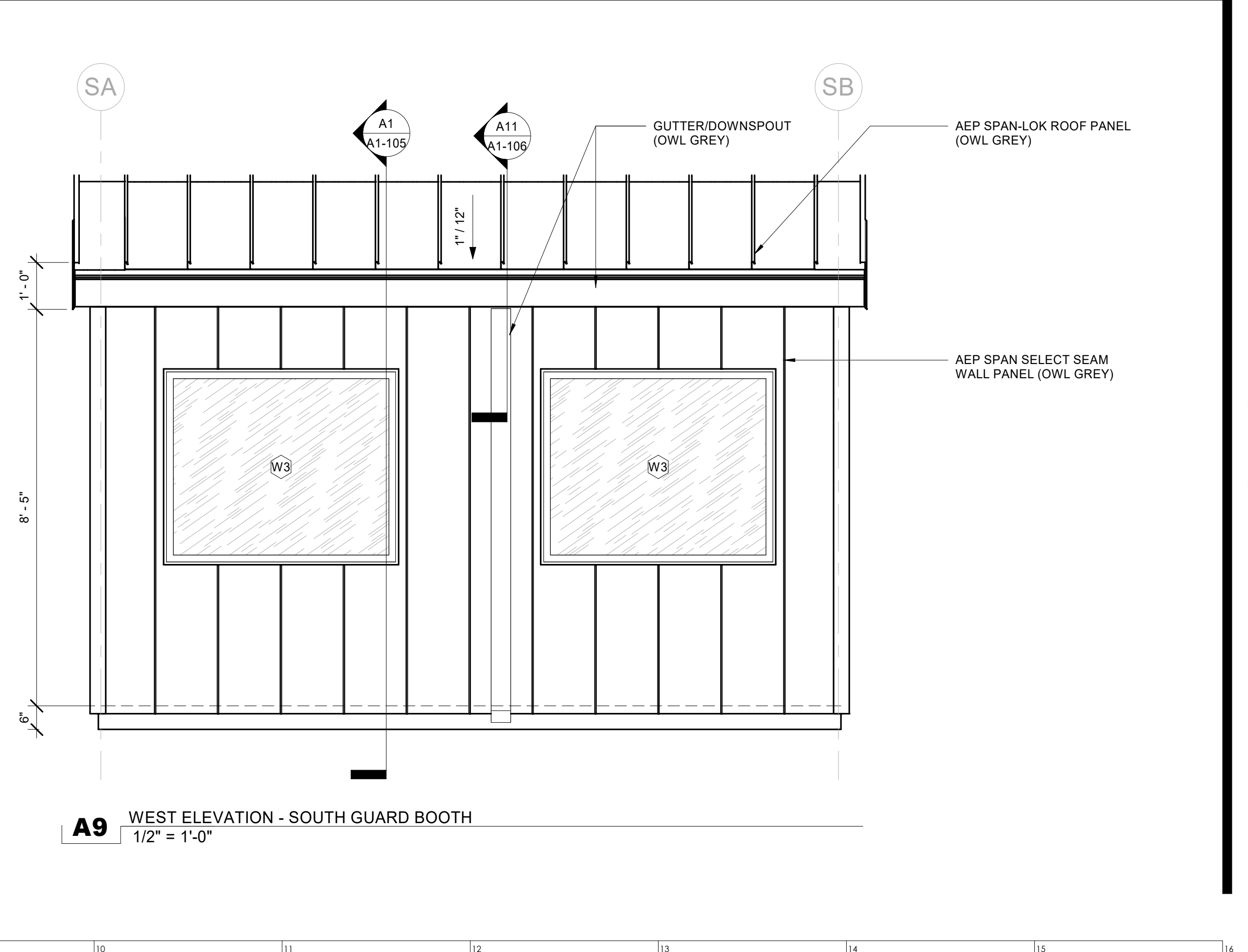


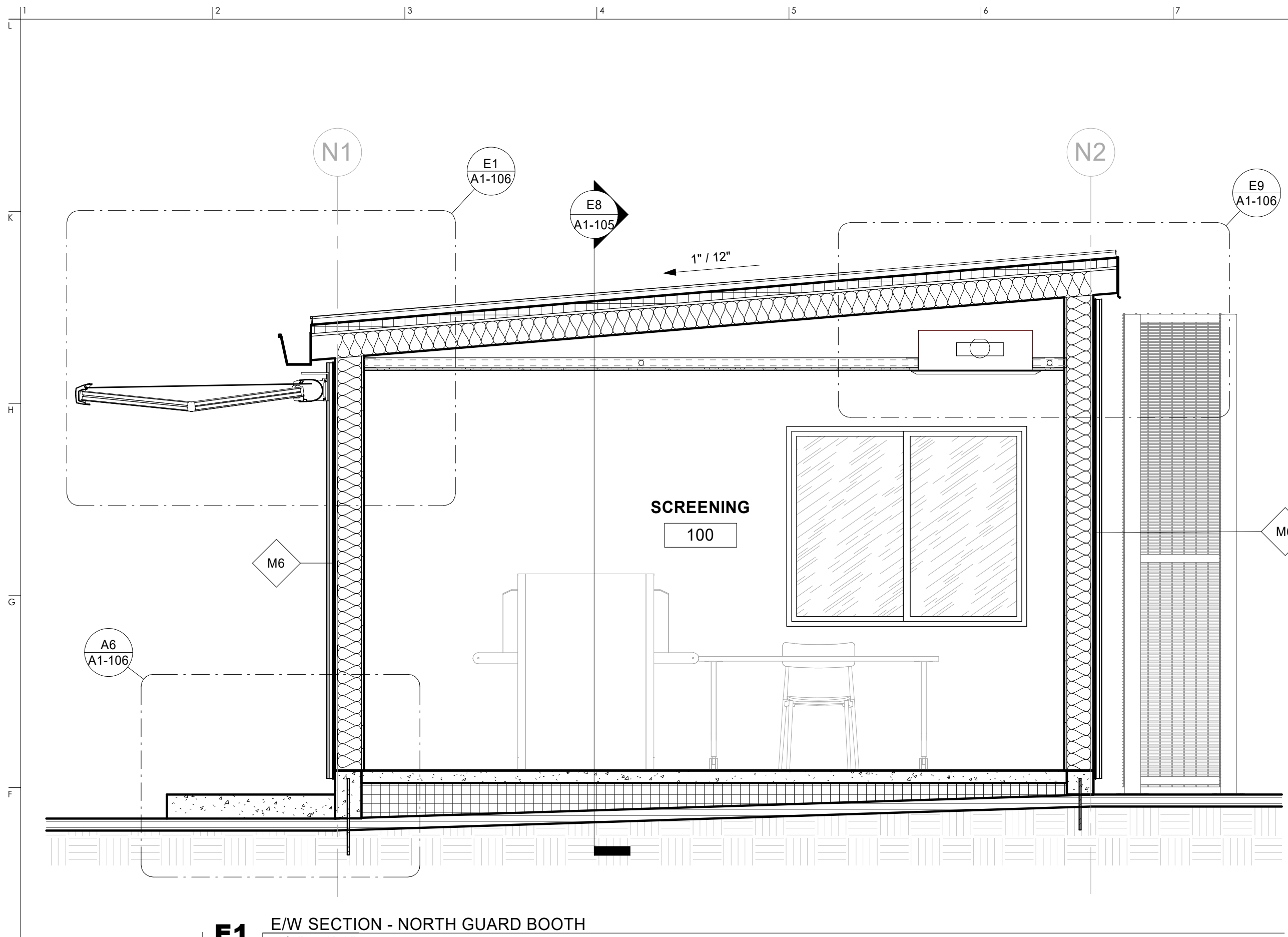
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1/2" = 1'-0"



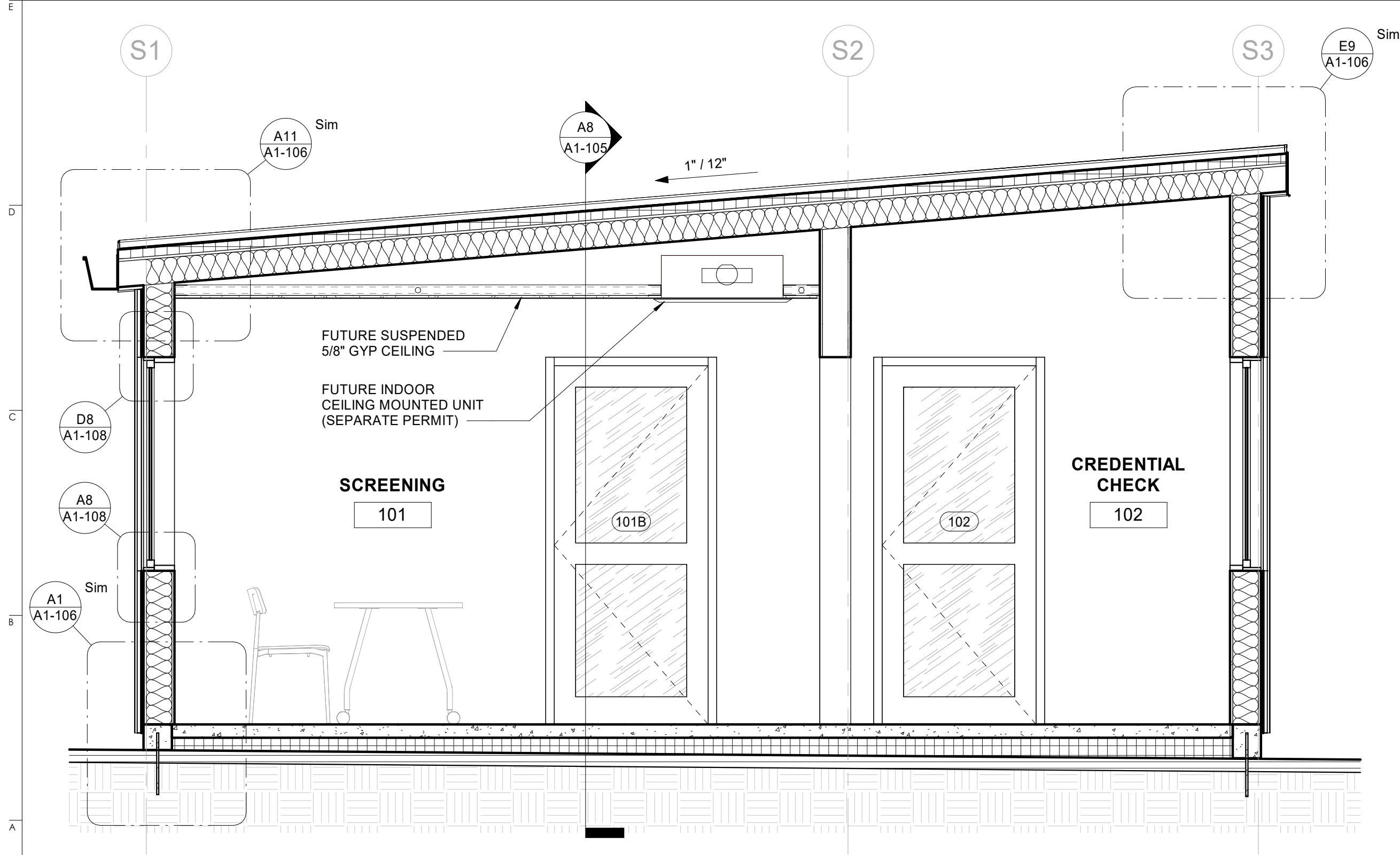
8995
REGISTERED
ARCHITECT
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STATE OF WASHINGTON
David Murphy
10/20/2026

sheet no.

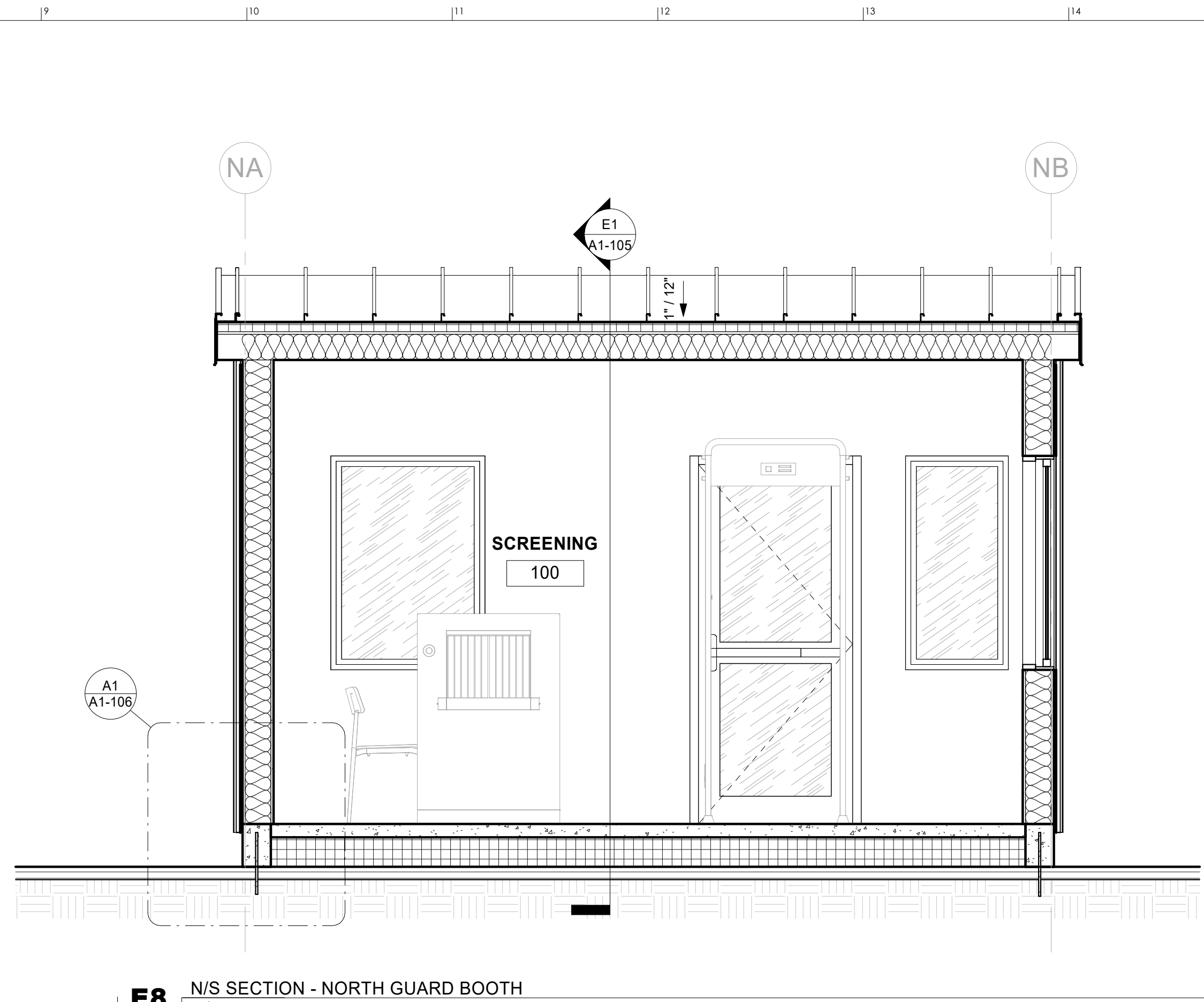




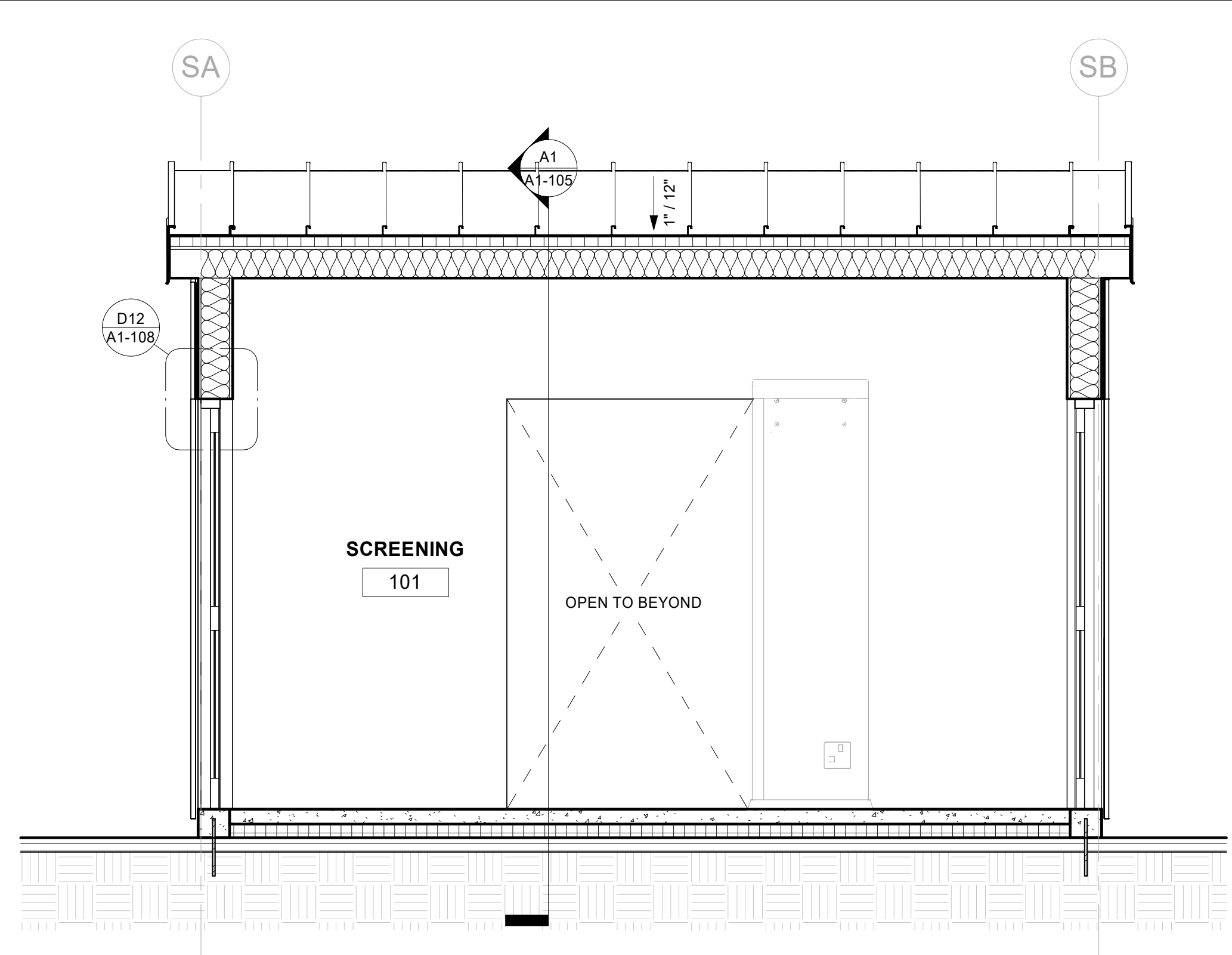
E1 E/W SECTION - NORTH GUARD BOOTH
1/2" = 1'-0"



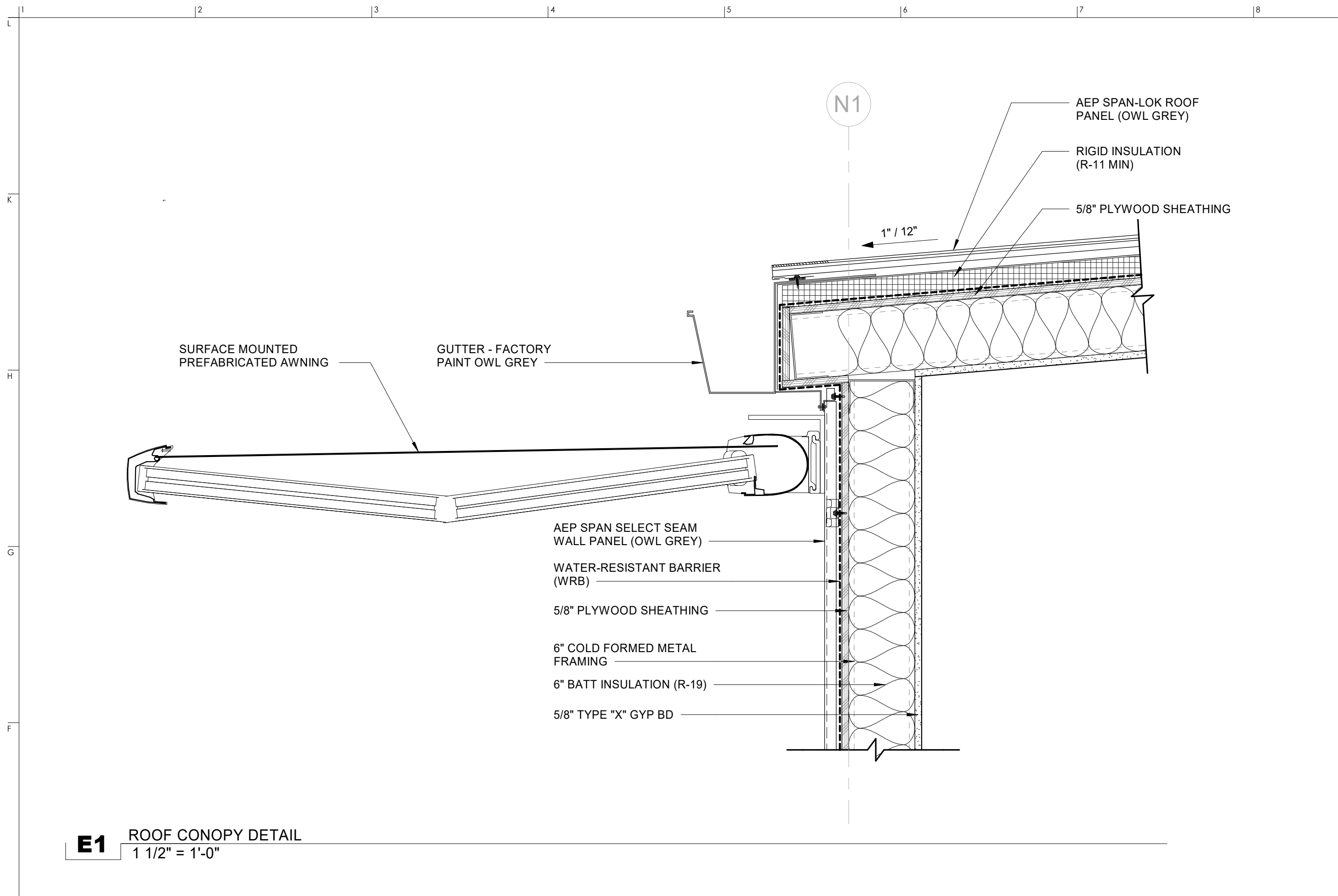
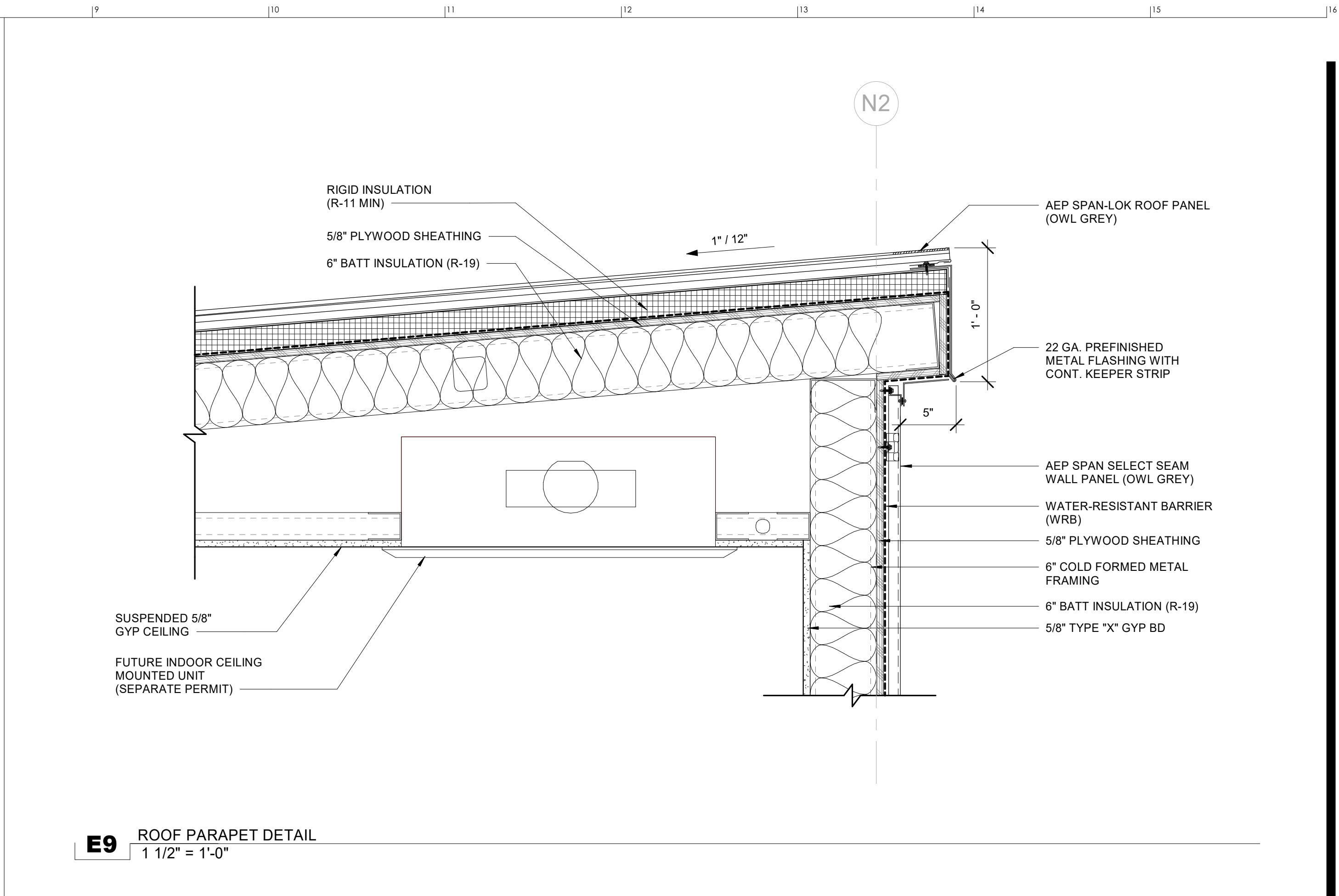
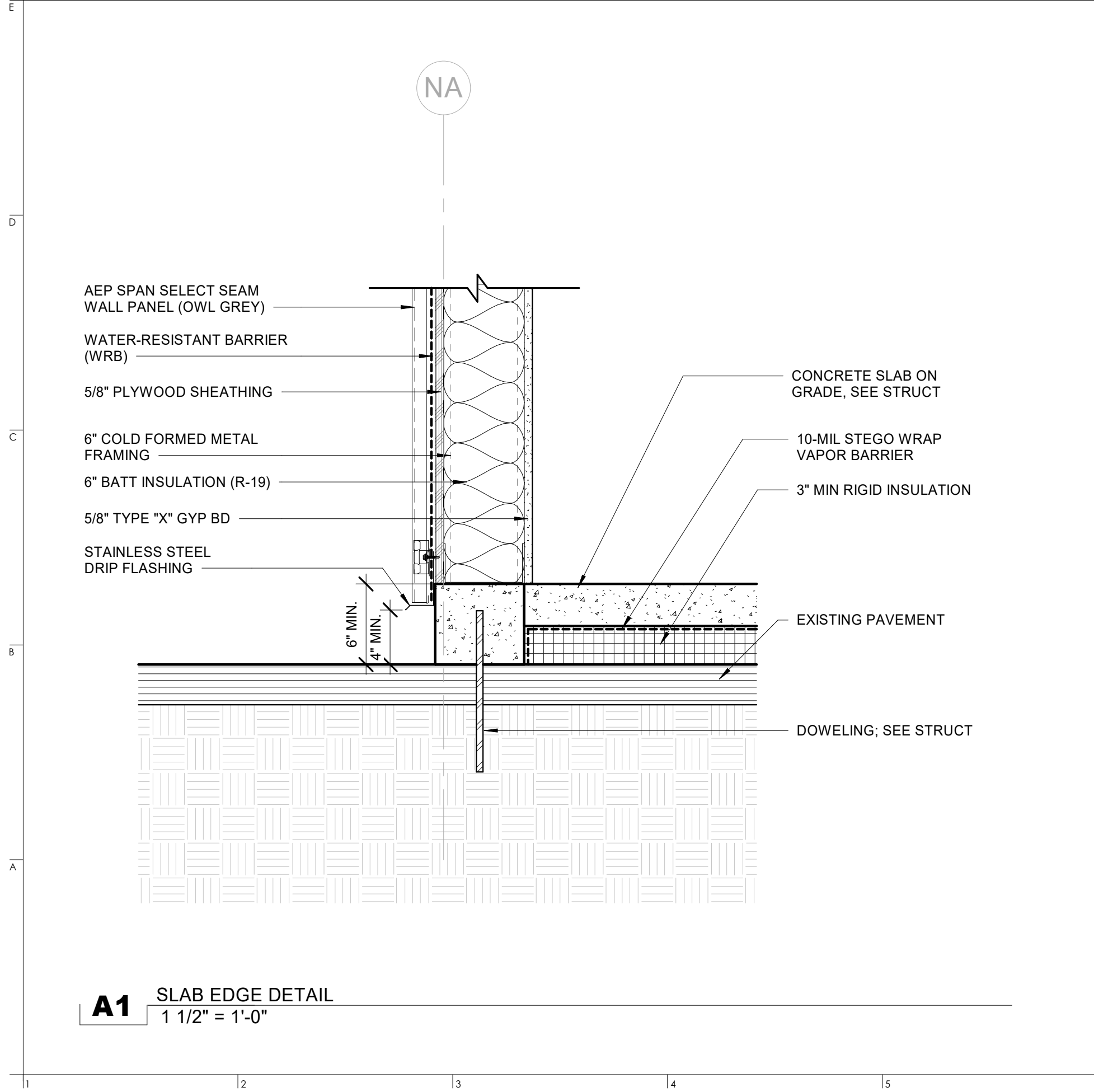
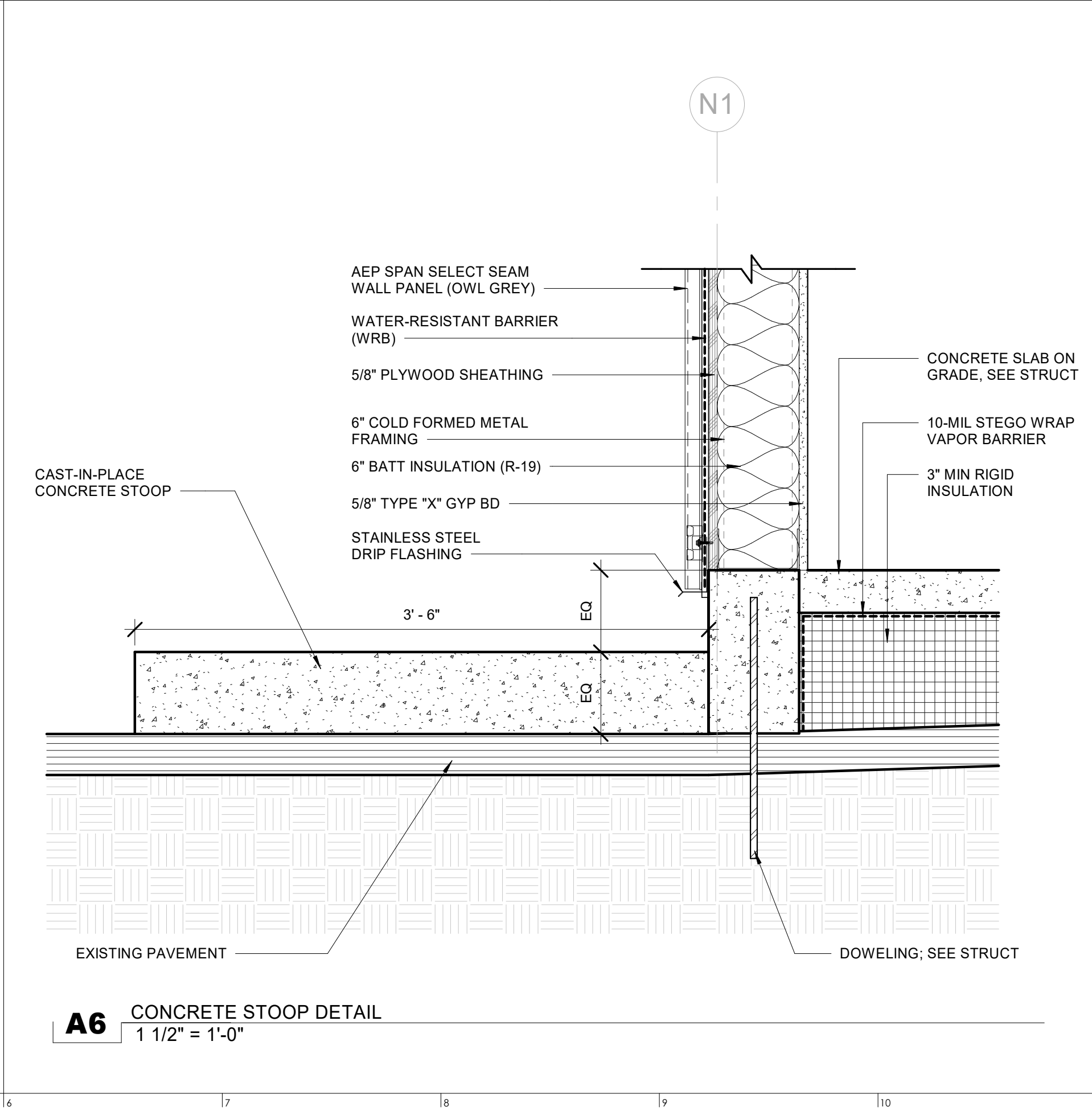
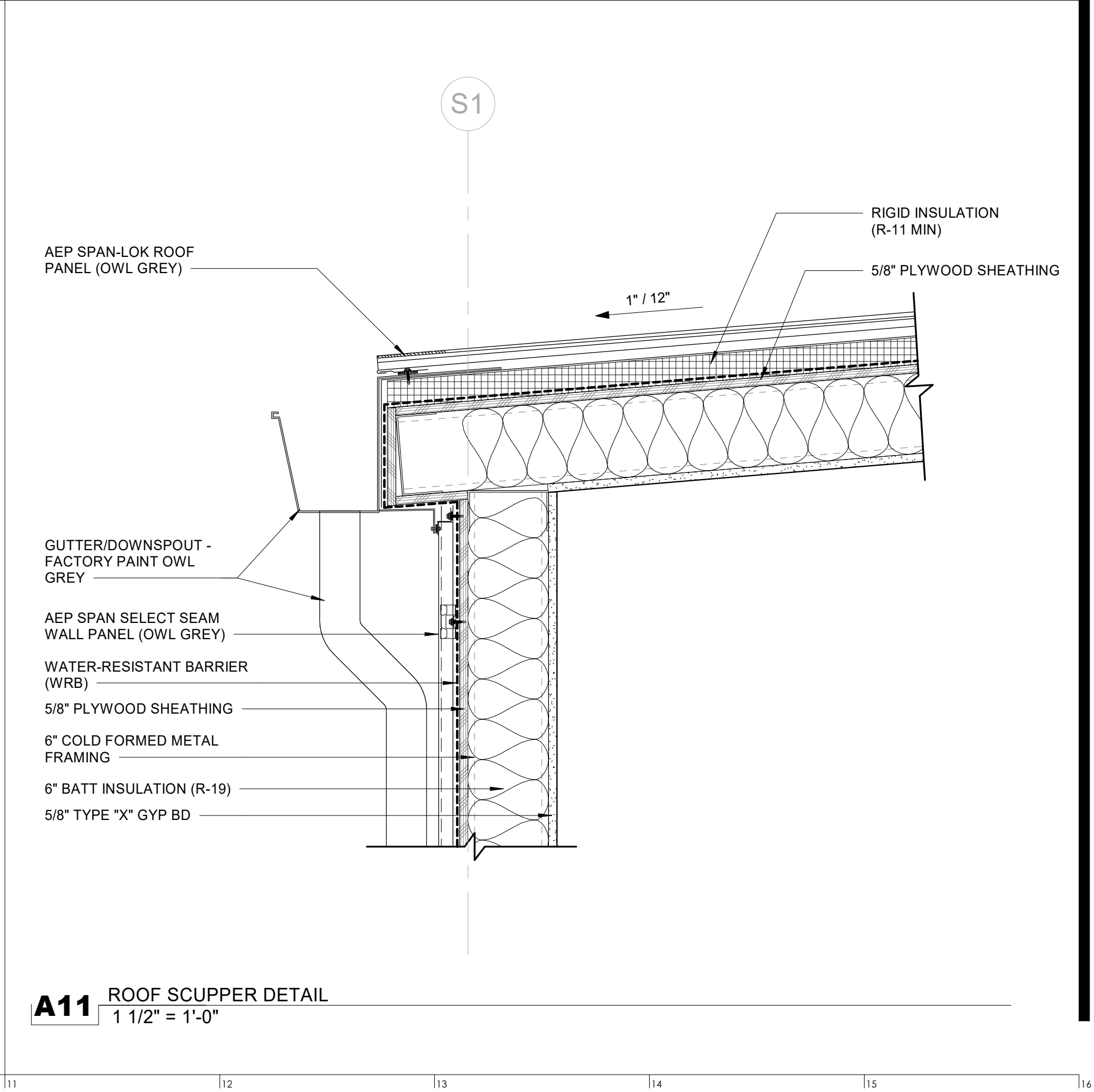
A1 E/W SECTION - SOUTH GUARD BOOTH
1/2" = 1'-0"



E8 N/S SECTION - NORTH GUARD BOOTH
1/2" = 1'-0"



A8 N/S SECTION - SOUTH GUARD BOOTH
1/2" = 1'-0"



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8995 REGISTERED ARCHITECT
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David Merle Murphy
10/20/2026

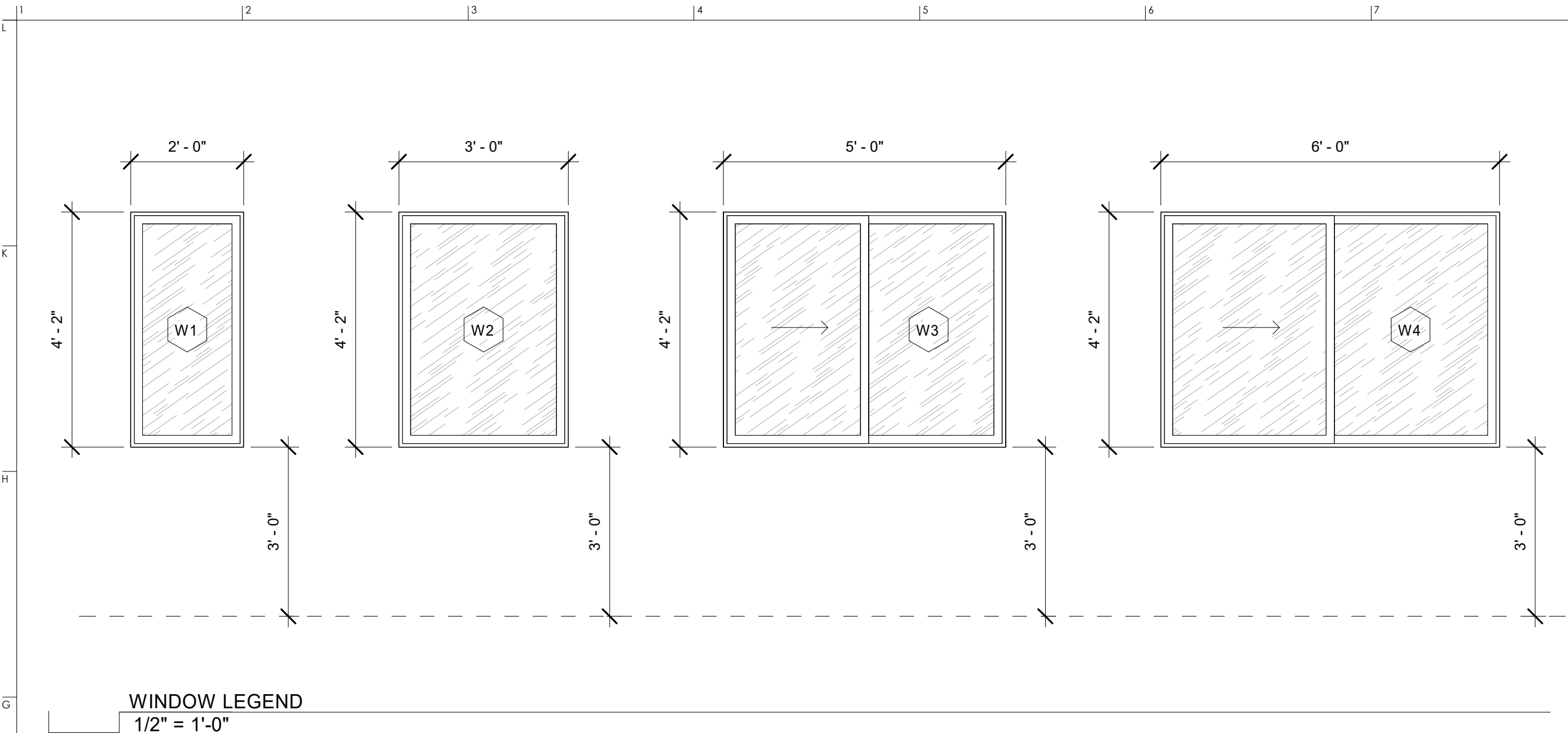
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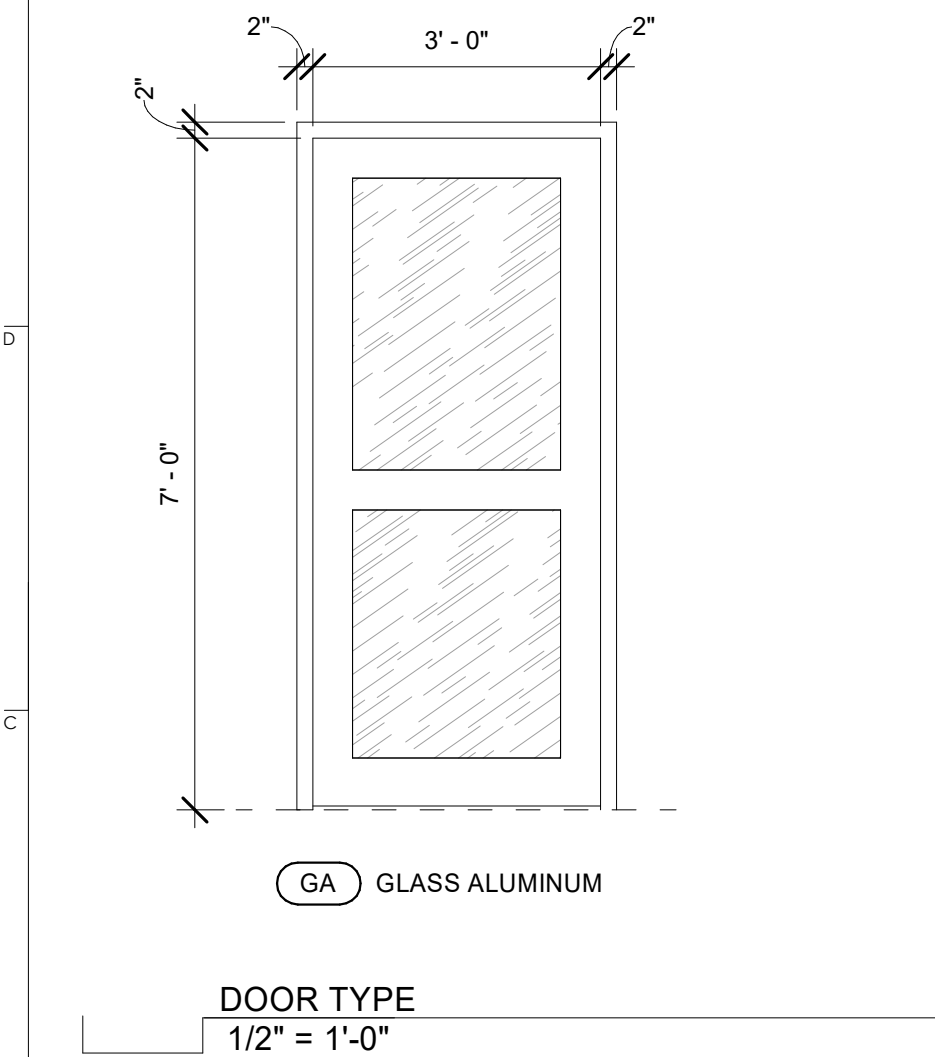
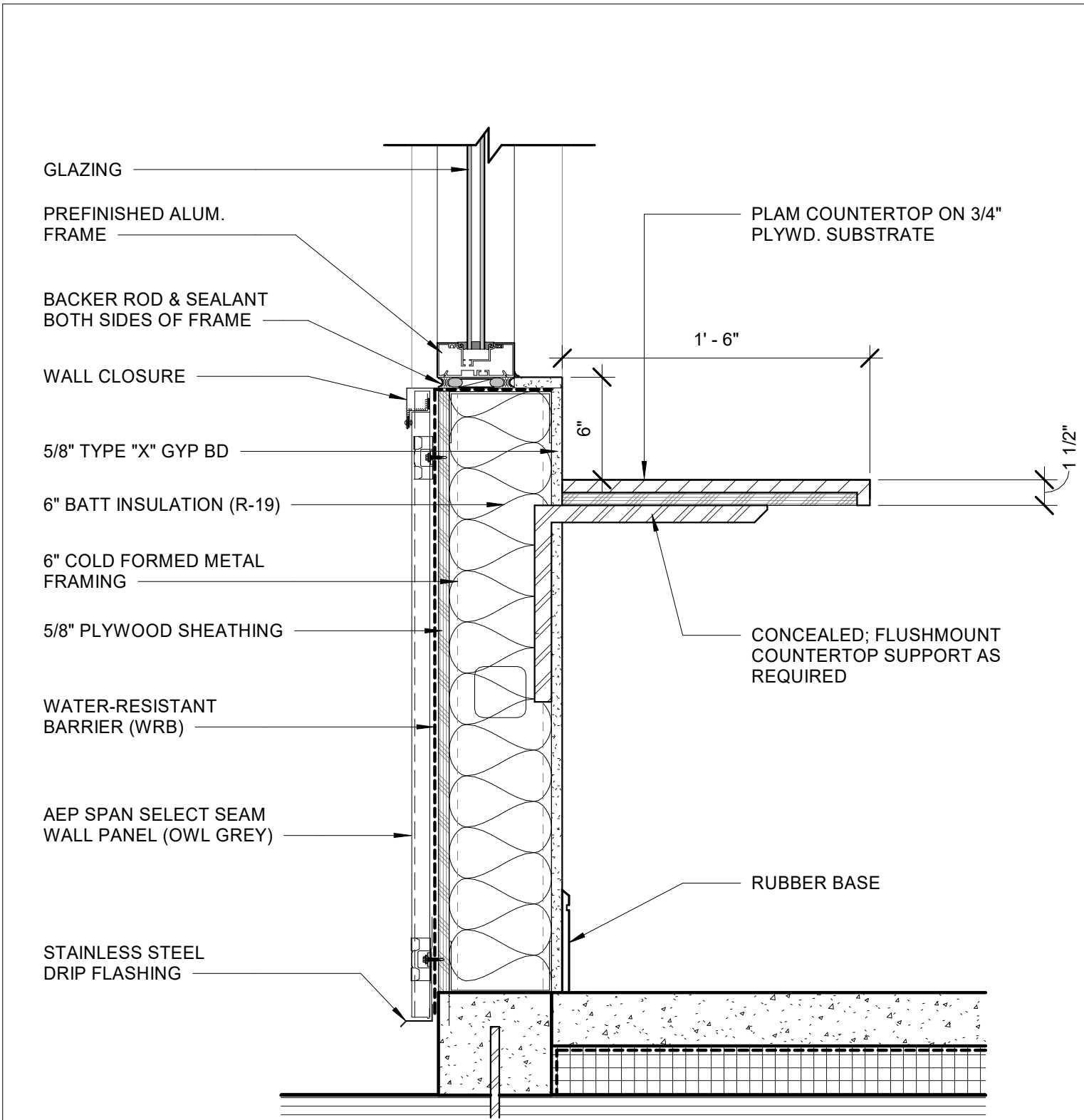
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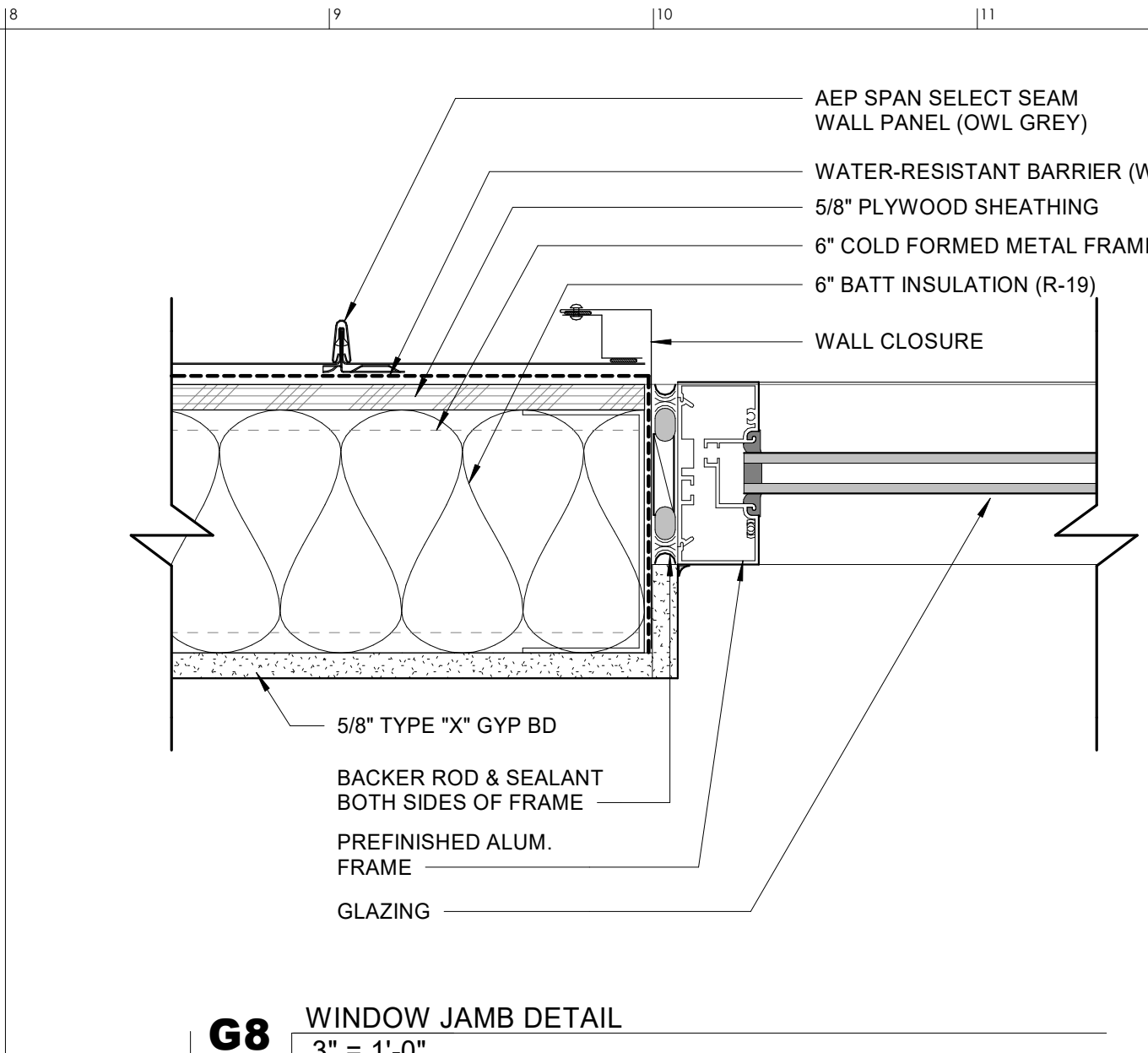
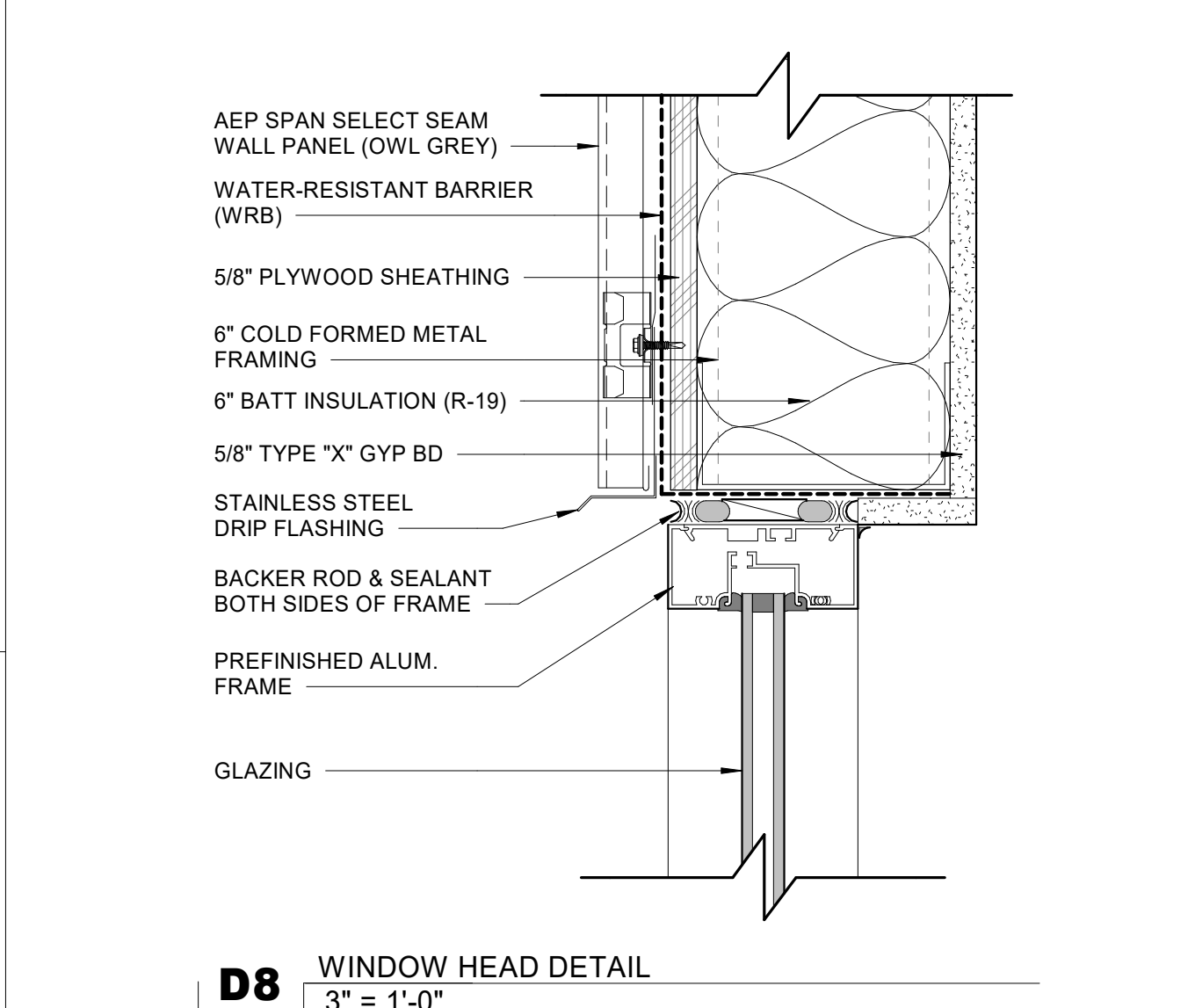
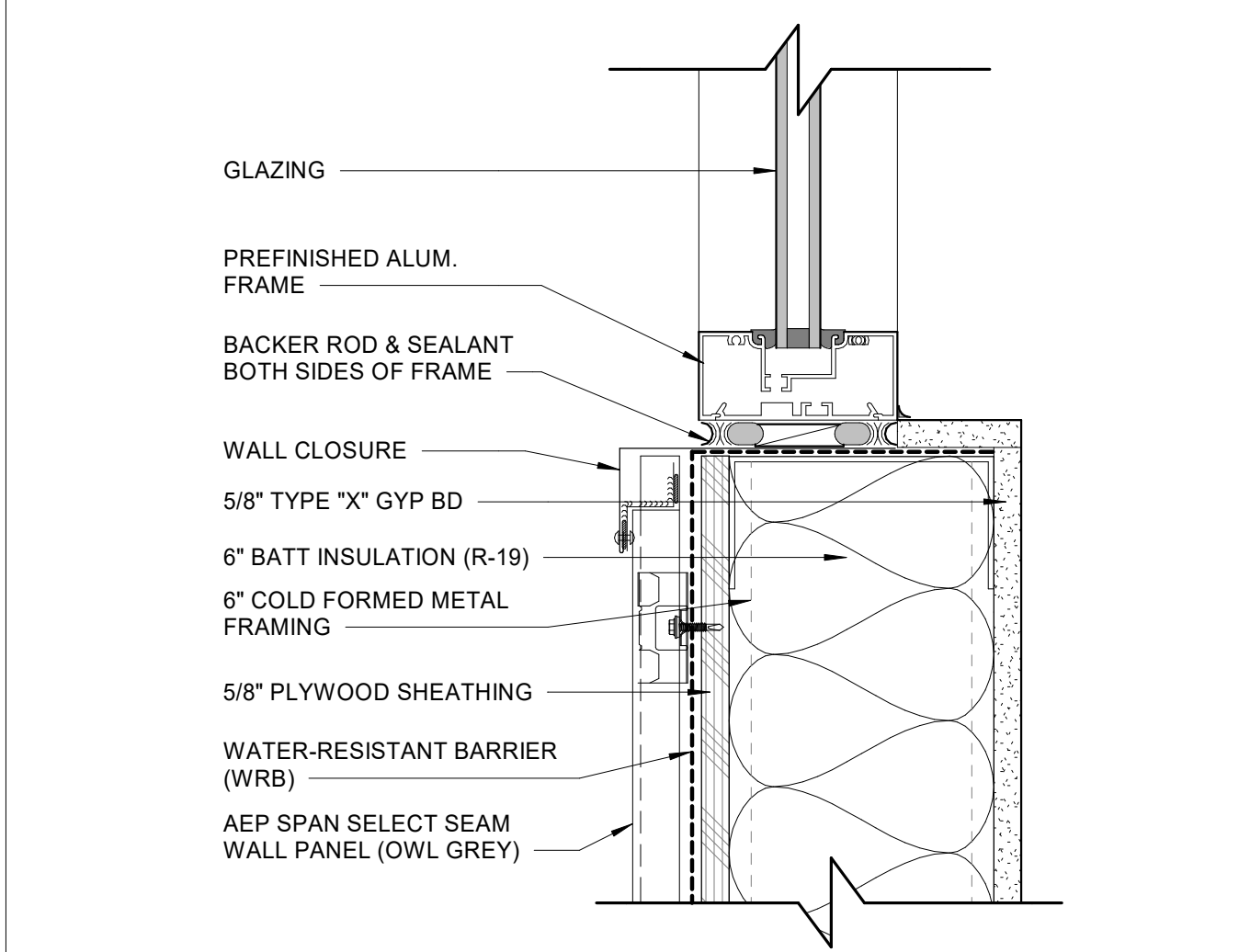
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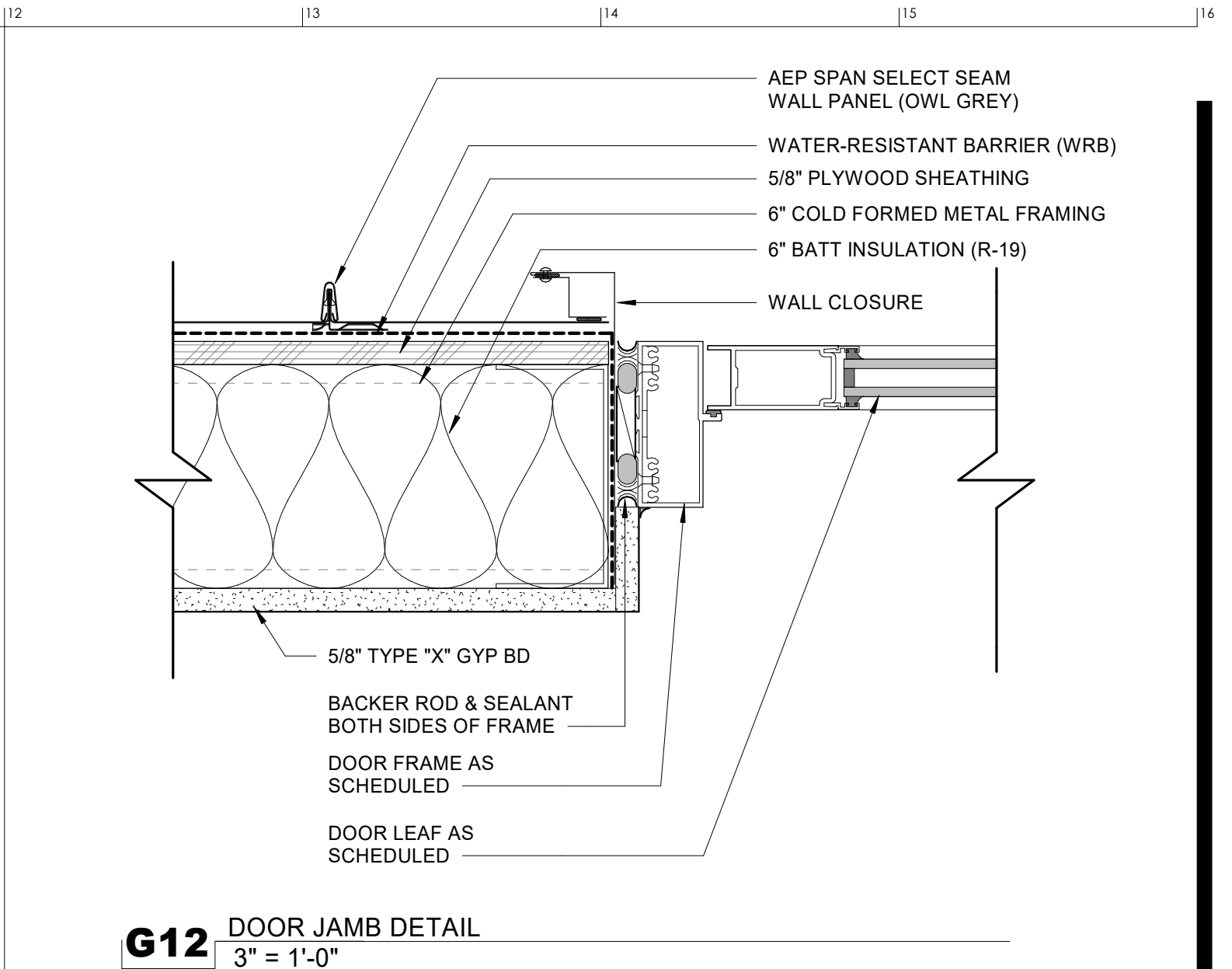
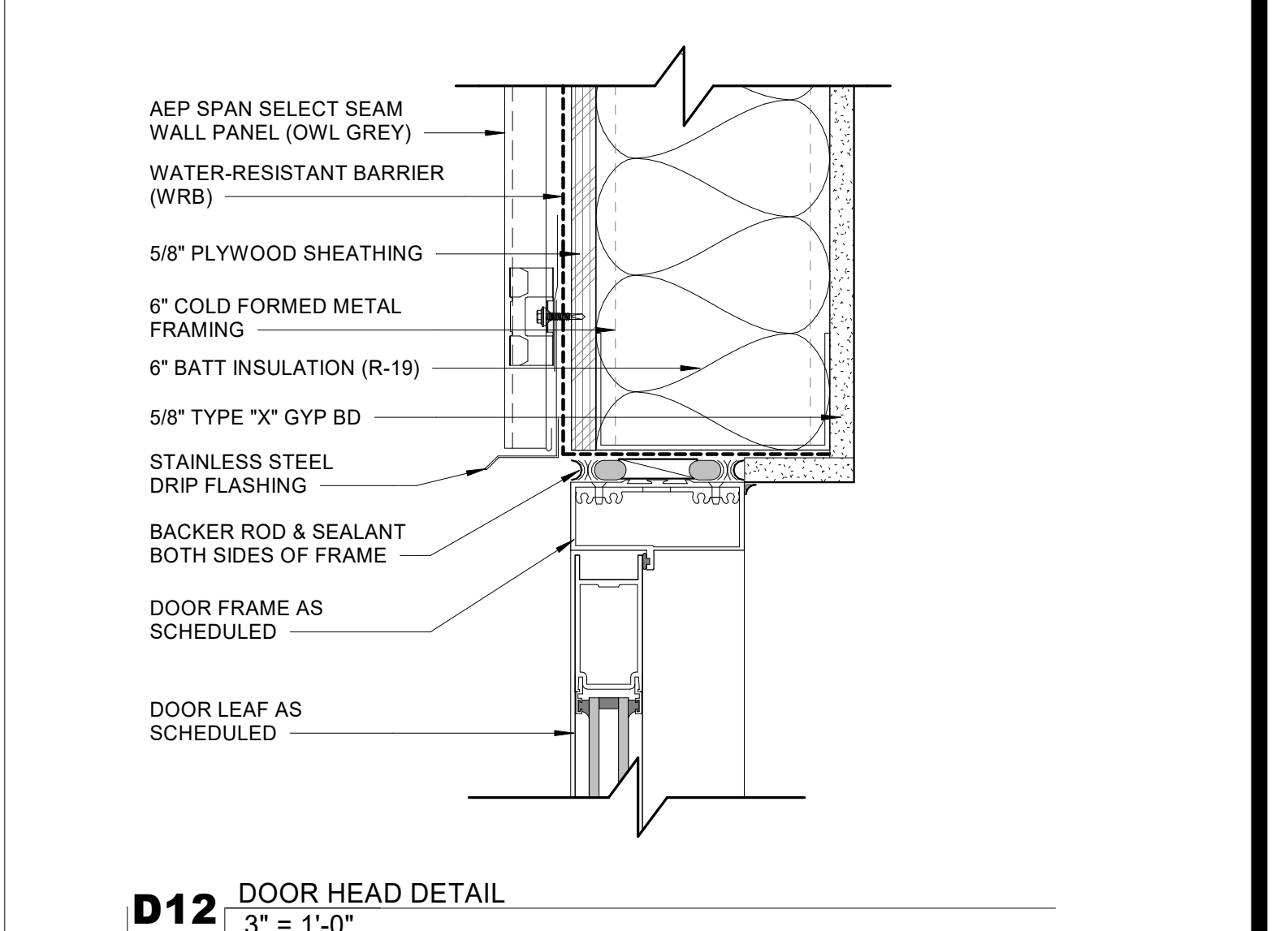
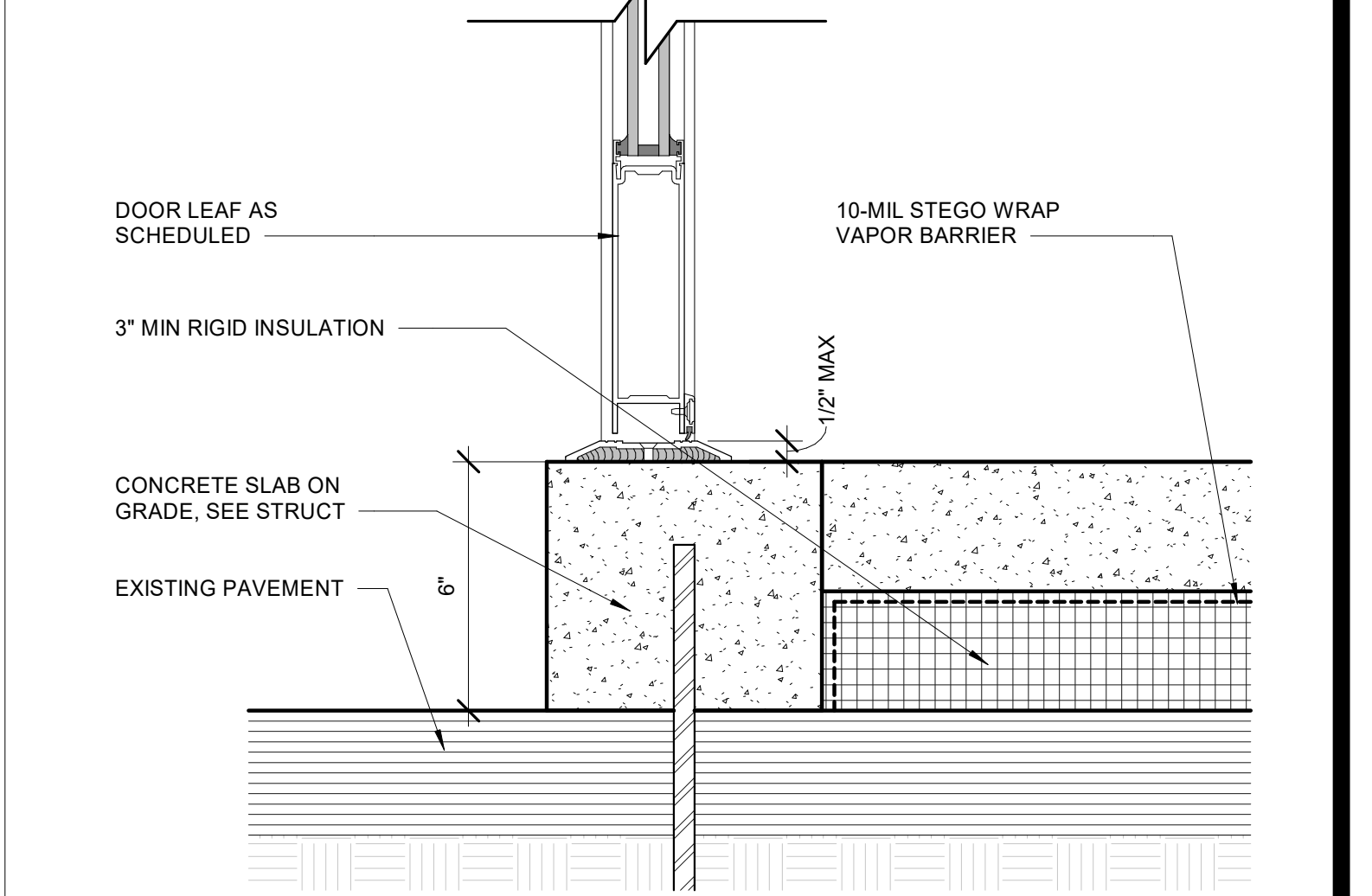
DOOR SCHEDULE												
#	Type	Door						Frame		Details		
		Width	Height	Thick.	Mat.	Finish	Glazing	Mat.	Finish	Head	Jamb	Sill
FIRST FLOOR												
100A	GA	3' - 0"	7' - 0"	1 3/4"	ALUM	GREY	GL-1	ALUM	BLACK	D12 / A1-108	G12 / A1-108	A4 / A1-108
100B	GA	3' - 0"	7' - 0"	1 3/4"	ALUM	GREY	GL-1	ALUM	BLACK	D12 / A1-108	G12 / A1-108	A12 / A1-108
101A	GA	3' - 0"	7' - 0"	1 3/4"	ALUM	GREY	GL-1	ALUM	BLACK	D12 / A1-108	G12 / A1-108	A12 / A1-108
101B	GA	3' - 0"	7' - 0"	1 3/4"	ALUM	GREY	GL-1	ALUM	BLACK	D12 / A1-108	G12 / A1-108	A12 / A1-108
102	GA	3' - 0"	7' - 0"	1 3/4"	ALUM	GREY	GL-1	ALUM	BLACK	D12 / A1-108	G12 / A1-108	A12 / A1-108



A4 SECTION THROUGH COUNTERTOP
1 1/2" = 1'-0"



A8 WINDOW SILL DETAIL
3" = 1'-0"



A12 DOOR SILL DETAIL
3" = 1'-0"

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LUMEN FIELD SECURITY BOOTHS

800 OCCIDENTAL AVE S
SEATTLE, WA 98134

PERMIT SET

10/19/2026

TYPICAL ABBREVIATIONS

AB	ANCHOR BOLT	ICF	INSULATED CONCRETE FORM
ADD'L	ADDITIONAL	IN	INCHES
ADJ	ADJACENT	INT	INTERIOR
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	JST	JOIST
ALT	ALTERNATE	JT	JOINT
APPROX	APPROXIMATE	K	KIPS = 1000 LBS
ARCH	ARCHITECT	KSI	KIPS PER SQUARE INCH
ASD	ALLOWABLE STRESS DESIGN		
BF	BRACED FRAME	L	ANGLE
BLDG	BUILDING	LBS (or LB)	POUNDS (or POUND)
BLKG	BLOCKING	LFRS	LATERAL FORCE RESISTING SYSTEM
BM	BEAM	LL	LIVE LOAD
BN	BOUNDARY NAILING	LLH	LONG LEG HORIZONTAL
BOT	BOTTOM	LLV	LONG LEG VERTICAL
BP	BASE PLATE	LONGIT	LONGITUDINAL
BRB	BUCKLING RESTRAINED BRACES	LSH	LONG SIDE HORIZONTAL
BRG	BEARING	LSL	LAMINATED STRAND LUMBER
BTWN	BETWEEN	LVL	LAMINATED VENEER LUMBER
CIP	CAST-IN-PLACE	MB	MACHINE BOLT
CJ	CONSTRUCTION/CONTROL JOINT	MTL	METAL
CL	CENTERLINE	MWFRS	MAIN WIND FORCE RESISTING SYSTEM
CLR	CLEAR	NS	NEAR SIDE
CLT	CROSS-LAMINATED TIMBER	NTS	NOT TO SCALE
CMU	CONCRETE MASONRY UNIT	OC	ON CENTER
COL	COLUMN	OD	OUTSIDE DIAMETER
CONC	CONCRETE	OPNG	OPENING
CONN	CONNECTION	OPP	OPPOSITE
CONST	CONSTRUCTION	OWSJ	OPEN WEB STEEL JOISTS
CONT	CONTINUOUS		
D	DEEP	PAF	POWER ACTUATED FASTENER
DBA	DEFORMED BAR ANCHOR	PCF	POUNDS PER CUBIC FOOT
DBL	DOUBLE	PCMB	PRE-ENGINEERED METAL BUILDING
DF	DOUGLAS-FIR	PERP	PERPENDICULAR
DIAG	DIAGONAL	PL	PLATE
DIAPH	DIAPHRAGM	PLF	POUNDS PER LINEAR FOOT
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DL	DEAD LOAD	PSI	POUNDS PER SQUARE INCH
DT	DRAG TRUSS	PSL	PARALLEL STRAND LUMBER
(E)	EXISTING	PT	POST-TENSIONED
EA	EACH	PT	PRESSURE TREATED
EL OR ELEV	ELEVATION	RT	ROOF TRUSS
EMBED	EMBEDMENT		
EN	EDGE NAILING	SEOR	STRUCTURAL ENGINEER OF RECORD
ENGR	ENGINEER	SF	SQUARE FEET
EOR	ENGINEER OF RECORD	SFRS	SEISMIC FORCE RESISTING SYSTEM
EQ	EQUAL	SHTG	SHEATHING
EQUIV	EQUIVALENT	SIM	SIMILAR
EXP	EXPANSION	SOG	SLAB ON GRADE
EXT	EXTERIOR	SQ	SQUARE
FDN	FOUNDATION	SS	STAINLESS STEEL
FF	FINISH FLOOR	STD	STANDARD
FFE	FINISH FLOOR ELEVATION	STL	STEEL
FLR	FLOOR	SW	SHEAR WALL
FN	FIELD NAILING		
F/O	FACE OF	T&B	TOP & BOTTOM
FS	FAR SIDE	T&G	TONGUE AND GROOVE
FT	FEET	T/O	TOP OF
FTG	FOOTING	TRANSV	TRANSVERSE
FT-LB	FOOT POUNDS	TYP	TYPICAL
GA	GAGE/GAUGE	UNO	UNLESS NOTED OTHERWISE
GALV	GALVANIZED		
GB	GRADE BEAM	VERT	VERTICAL
GC	GENERAL CONTRACTOR	VIF	VERIFY IN FIELD
GLB	GLUE LAMINATED BEAM		
GLC	GLUE LAMINATED COLUMN	W	WIDE
GR	GRADE	W/	WITH
GT	GIRDER TRUSS	W/O	WITHOUT
GWB	GYP SUM WALL BOARD	WHS	WELDED HEADED STUD
GWP	GLOBAL WARMING POTENTIAL	WP	WORK POINT
		WTS	WELDED THREADED STUD
HDR	HEADER	WWF	WELDED WIRE FABRIC
HD	HOLDOWN		
HF	HEM-FIR		
HGR	HANGER		
HORIZ	HORIZONTAL		
HSS	HOLLOW STRUCTURAL SECTION		
HT	HEIGHT		

SHEET INDEX	
SHEET #	SHEET NAME
S101	STRUCTURAL TITLE SHEET
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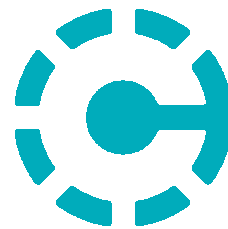
GRAPHIC SYMBOL LEGEND

SURFACE PATTERNS	PLAN (UNO)		OVERFRAMING
	SECTION (UNO)		CONCRETE
CUT PATTERNS	PLAN ONLY (UNO)		BLOCKED DIAPHRAGM
	SECTION ONLY (UNO)		BRICK VENEER
EXISTING & BY OTHERS	PLAN ONLY (UNO)		PLYWOOD
	SECTION ONLY (UNO)		STEEL
	PLAN ONLY (UNO)		TOP OF WALL (ALL MATERIALS)
	SECTION ONLY (UNO)		GROUT
	PLAN ONLY (UNO)		HIDDEN WALL/COLUMN BELOW (ALL MATERIALS)
	SECTION ONLY (UNO)		STRUCTURAL FOAM
	PLAN ONLY (UNO)		CMU WALL (ELEVATION SURFACE)
	SECTION ONLY (UNO)		SAND - LEAN FILL
	PLAN ONLY (UNO)		CONCRETE WALL OR COLUMN
	SECTION ONLY (UNO)		EARTH
	PLAN ONLY (UNO)		WOOD/CFS SHEAR WALL (LOCATION OF SW MARK INDICATES SIDE OF SHTG)
	SECTION ONLY (UNO)		GRAVEL
	PLAN ONLY (UNO)		WOOD/CFS STUD WALL (SECTION OR PLAN)
	SECTION ONLY (UNO)		INSULATION
	PLAN ONLY (UNO)		CMU WALL (SECTION OR PLAN)
	SECTION ONLY (UNO)		MECH UNIT (SECTION OR PLAN)
	PLAN ONLY (UNO)		WOOD/CFS STUD WALL (SECTION OR PLAN)
	SECTION ONLY (UNO)		CONSTRUCTION ELEMENTS (SECTION OR PLAN)
	PLAN ONLY (UNO)		EXISTING SCOPE (SECTION OR PLAN)
	SECTION ONLY (UNO)		HIDDEN CONSTRUCTION ELEMENTS BELOW OR BEYOND (SECTION OR PLAN)
	PLAN ONLY (UNO)		EXISTING FRAMING (PLAN ONLY)
	SECTION ONLY (UNO)		HIDDEN EXISTING FRAMING BELOW (PLAN ONLY)

WOOD COLUMN (CUT)		EXTENT NOTES	
WOOD BLOCKING (CUT)		SHEATHING SPAN	
WOOD COLUMN (BELOW)		SURFACE SLOPE PER ARCHITECT	
STEEL HSS COLUMN (CUT)		JOIST SPAN	
STEEL HSS COLUMN (BELOW)		DETAIL REFERENCE #	
STEEL WIDE FLANGE COLUMN (CUT)		SECTION CALLOUT	
STEEL WIDE FLANGE COLUMN (BELOW)		SHEET REFERENCE #	
SIMPSON TENSION TIE HOLDOWN		DETAIL REFERENCE #	
CONCRETE COLUMN TYPE		ELEVATION CALLOUT	
NUMBER OF KINGS PLUS TRIMMERS		SHEET REFERENCE #	
FOOTING STEP		DETAIL REFERENCE #	
DECK SPAN		DETAIL CALLOUT	
EXTENTS (AS DRAWN)		SHEET REFERENCE #	
EXTENTS (FULL EXTENT OF ELEMENT)			



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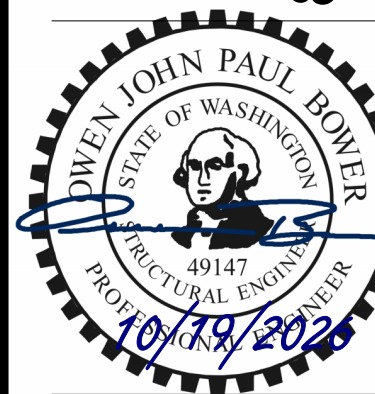


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GENERAL REQUIREMENTS

SUMMARY OF WORK
PROJECT CONSISTS OF NEW CONSTRUCTION AS SHOWN ON THESE CONTRACT DOCUMENTS USED IN COORDINATION WITH THE ARCHITECTURAL AND OTHER DISCIPLINE'S DOCUMENTS.

GOVERNING CODE
ALL DESIGN AND CONSTRUCTION SHALL CONFORM TO THE 2021 SEATTLE BUILDING CODE (SBC).

REFERENCE TO ASTM AND OTHER STANDARDS SHALL REFER TO THE LATEST EDITION DESIGNATED BY SBC CHAPTER 35. REFER TO THE SPECIFICATIONS FOR INFORMATION IN ADDITION TO THAT COVERED BY THESE STRUCTURAL NOTES AND DRAWINGS.

DOCUMENTS
STRUCTURAL DOCUMENTS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DOCUMENTS FOR ALL BIDDING AND CONSTRUCTION.

DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. TYPICAL DETAILS AND GENERAL NOTES SHALL APPLY EVEN IF NOT SPECIFICALLY DENOTED ON PLANS, UNO. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE SEOR.

EXISTING STRUCTURAL INFORMATION, DESIGNATED AS (E) ON THE STRUCTURAL DRAWINGS, HAS BEEN COMPILED FROM INFORMATION FURNISHED BY VARIOUS SOURCES AND IS NOT NECESSARILY FIELD-VERIFIED BY THE ENGINEER. DIMENSIONS RELATING TO EXISTING STRUCTURES ARE INTENDED FOR USE AS GUIDELINES ONLY; ALL DIMENSIONS SHALL BE FIELD-VERIFIED BY THE CONTRACTOR PRIOR TO START OF CONSTRUCTION. NOTIFY THE ARCHITECT OF ANY DISCREPANCIES.

THESE CONTRACT DOCUMENTS AND ANY MATERIALS USED IN PREPARATION OF THEM, INCLUDING CALCULATIONS, ARE THE EXCLUSIVE PROPERTY OF THE SEOR AND CAN BE REPRODUCED ONLY WITH THE PERMISSION OF THE SEOR.

WARRANTY
THE SEOR HAS USED THAT DEGREE OF CARE AND SKILL ORDINARILY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY MEMBERS OF THE PROFESSION IN THIS LOCALE AND NO OTHER WARRANTY, EITHER EXPRESSED OR IMPLIED, IS MADE IN CONNECTION WITH RENDERING PROFESSIONAL SERVICES.

OWNER RESPONSIBILITY
THE OWNER SHALL RETAIN A SPECIAL INSPECTOR TO PERFORM THE SPECIAL INSPECTION REQUIREMENTS REQUIRED BY THE BUILDING OFFICIAL AND AS OUTLINED IN THE SPECIAL INSPECTION SECTION BELOW.

DESIGN CRITERIA

BUILDING CATEGORY
STRUCTURAL RISK CATEGORY II
IMPORTANCE FACTORS FOR SNOW AND SEISMIC ARE LISTED WITH THE LOADING CRITERIA.

LIVE LOADS - SNOW
MINIMUM UNIFORM ROOF SNOW LOAD = 25 PSF

LATERAL LOADS - WIND
1. ULTIMATE DESIGN WIND SPEED (3-SECOND GUST): V_{ult} = 92 MPH
NOMINAL DESIGN WIND SPEED: V_{nd} = 71 MPH
2. RISK CATEGORY: I
3. WIND EXPOSURE: B
4. INTERNAL PRESSURE COEFFICIENT = +/- 0.18
5. COMPONENTS AND CLADDING:
THE FOLLOWING LOADS MAY BE USED FOR DESIGN OF COMPONENTS AND CLADDING WITH AN EFFECTIVE WIND AREA OF 10 SF MINIMUM IN LIEU OF CALCULATIONS (ZONES PER ASCE 7, CHAPTER 30):

5. COMPONENTS AND CLADDING:
THE WORKING LOADS SHOWN ON THE WIND LOADING DIAGRAMS, IF SHOWN, MAY BE USED FOR DESIGN OF COMPONENTS AND CLADDING WITH AN EFFECTIVE WIND AREA OF 10 SF MINIMUM IN LIEU OF CALCULATIONS.

ADDITIONAL INFO:
6. TOPOGRAPHIC FACTOR: K_{zt} = 1.0
7. DIRECTIONALITY FACTOR: K_d = 0.85
8. GROUND ELEVATION FACTOR: K_e = 1.0
9. ENCLOSURE CLASSIFICATION: ENCLOSED
10. GUST EFFECT FACTOR: G = 0.85
11. DESIGN BASE SHEAR: V = 1.76 KIPS (NE SHACK)
2.46 KIPS (SE SHACK)

12. ANALYSIS PROCEDURE: DIRECTIONAL

LATERAL LOADS - EARTHQUAKE
1. RISK CATEGORY: I
2. SEISMIC IMPORTANCE FACTOR: I_s = 1.0
3. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS: S_s = 1.424 g; S₁ = 0.496 g
4. SITE CLASS: D_s F_s = 1.2;
5. DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS: S_{os} = 1.136 g
6. SEISMIC DESIGN CATEGORY: D
7. BASIC SEISMIC FORCE RESISTING SYSTEMS:
VERTICAL ELEMENTS: COLD-FORMED STEEL WALLS SHEATHED WITH WOOD STRUCTURAL PANELS
DIAPHRAGMS: PLYWOOD SHEATHING ON COLD-FORMED STEEL JOISTS;
8. DESIGN BASE SHEAR: 0.88 KIPS (NE SHACK)
1.21 KIPS (SE SHACK)
9. SEISMIC RESPONSE COEFFICIENT: C_s = 0.175
10. RESPONSE MODIFICATION COEFFICIENT: R = 6.5
11. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

CONTRACTOR PERFORMANCE REQUIREMENTS

DESIGN DOCUMENTS
CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ALL CONDITIONS AT THE JOB SITE, INCLUDING BUILDING AND SITE CONDITIONS BEFORE COMMENCING WORK, AND BE RESPONSIBLE FOR SAME. ALL DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH WORK. ANY ERRORS, AMBIGUITIES AND/OR OMISSIONS IN THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT IMMEDIATELY, IN WRITING. NO WORK IS TO BE STARTED BEFORE CORRECTION IS MADE.

CONTRACTOR SHALL VERIFY AND/OR COORDINATE ALL DIMENSIONED OPENINGS AND SLAB EDGES SHOWN ON THE CONTRACT DOCUMENTS. SOME DIMENSIONS, OPENINGS AND EMBEDDED ITEMS ARE SHOWN ON THE STRUCTURAL DRAWINGS. OTHERS MAY BE REQUIRED. REFER TO ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF CURBS, EQUIPMENT PADS, WALL AND FLOOR OPENINGS, ARCHITECTURAL TREATMENT, EMBEDS REQUIRED FOR ARCHITECTURAL ITEMS AND DIMENSIONS. REFER TO MECHANICAL, PLUMBING, ELECTRICAL AND FIRE PROTECTION DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS FOR DUCTS, PIPING, CONDUITS, ETC. SUBMIT OPENINGS TO ARCHITECT FOR REVIEW.

DO NOT SCALE DRAWINGS. USE ONLY FIELD VERIFIED DIMENSIONS. WHEN ELECTRONIC PLAN FILES ARE PROVIDED FOR THE CONTRACTOR'S DETAILING CONVENIENCE, IT SHALL BE NOTED THAT THE ELECTRONIC FILES ARE NOT GUARANTEED TO BE DIMENSIONALLY ACCURATE. THE CONTRACTOR USES THEM AT THEIR OWN RISK. THE PUBLISHED PAPER DOCUMENTS ARE THE CONTROLLING CONTRACT DOCUMENTS. ELECTRONIC FILES OF DETAIL SHEETS AND NOTES WILL NOT BE PROVIDED.

CONTRACTOR PERFORMANCE REQUIREMENTS (CONT'D)

DESIGN DOCUMENTS
CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ALL CONDITIONS AT THE JOB SITE, INCLUDING BUILDING AND SITE CONDITIONS BEFORE COMMENCING WORK, AND BE RESPONSIBLE FOR SAME. ALL DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH WORK. ANY ERRORS, AMBIGUITIES AND/OR OMISSIONS IN THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT IMMEDIATELY, IN WRITING. NO WORK IS TO BE STARTED BEFORE CORRECTION IS MADE.

CONTRACTOR SHALL VERIFY AND/OR COORDINATE ALL DIMENSIONED OPENINGS AND SLAB EDGES SHOWN ON THE CONTRACT DOCUMENTS. SOME DIMENSIONS, OPENINGS AND EMBEDDED ITEMS ARE SHOWN ON THE STRUCTURAL DRAWINGS. OTHERS MAY BE REQUIRED. REFER TO ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF CURBS, EQUIPMENT PADS, WALL AND FLOOR OPENINGS, ARCHITECTURAL TREATMENT, EMBEDS REQUIRED FOR ARCHITECTURAL ITEMS AND DIMENSIONS. REFER TO MECHANICAL, PLUMBING, ELECTRICAL AND FIRE PROTECTION DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS FOR DUCTS, PIPING, CONDUITS, ETC. SUBMIT OPENINGS TO ARCHITECT FOR REVIEW.

DO NOT SCALE DRAWINGS. USE ONLY FIELD VERIFIED DIMENSIONS. WHEN ELECTRONIC PLAN FILES ARE PROVIDED FOR THE CONTRACTOR'S DETAILING CONVENIENCE, IT SHALL BE NOTED THAT THE ELECTRONIC FILES ARE NOT GUARANTEED TO BE DIMENSIONALLY ACCURATE. THE CONTRACTOR USES THEM AT THEIR OWN RISK. THE PUBLISHED PAPER DOCUMENTS ARE THE CONTROLLING CONTRACT DOCUMENTS. ELECTRONIC FILES OF DETAIL SHEETS AND NOTES WILL NOT BE PROVIDED.

SHOP DRAWINGS AND SUBMITTALS

SHOP DRAWING & SUBMITTAL REVIEW (INCLUDING DEFERRED STRUCTURAL COMPONENTS)
THE CONTRACTOR MUST REVIEW AND STAMP THE SHOP DRAWINGS & SUBMITTALS FOR REVIEW. SEOR WILL ONLY REVIEW SUBMITTALS FOR ITEMS SHOWN ON SEOR DOCUMENTS. SUBMITTALS FOR DEFERRED STRUCTURAL COMPONENTS WILL RECEIVE CURSORY REVIEW BY SEOR FOR LOADS IMPOSED ON PRIMARY STRUCTURE. SEOR WILL REVIEW SHOP DRAWINGS FOR GENERAL CONFORMANCE WITH DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE INFORMATION GIVEN IN THE STRUCTURAL CONTRACT DOCUMENTS. REVIEW OF SUBMITTALS DOES NOT CONSTITUTE APPROVAL OR ACCEPTANCE OF UNAUTHORIZED DEVIATION FROM CONTRACT DOCUMENTS.

CORRECTIONS OR COMMENTS MADE ON SHOP DRAWINGS DURING THIS REVIEW DO NOT RELIEVE CONTRACTOR FROM COMPLIANCE WITH THE REQUIREMENTS OF THE PLANS AND SPECIFICATIONS.

CONTRACTOR RESPONSIBLE FOR:
• REVIEWING, APPROVING, STAMPING AND SIGNING SUBMITTALS PRIOR TO SUBMITTAL TO ARCHITECT AND SEOR
• TIMING SUBMITTALS TO ALLOW TWO WEEKS OF REVIEW TIME FOR THE SEOR AND TIME FOR CORRECTIONS AND/OR RESUBMITTAL
• CONFORMANCE TO REQUIREMENTS OF THE CONTRACT DOCUMENTS
• DIMENSIONS AND QUANTITIES
• VERIFYING INFORMATION TO BE CONFIRMED OR COORDINATED
• INFORMATION SOLELY FOR FABRICATION, SAFETY, MEANS, METHODS, TECHNIQUES AND SEQUENCES OF CONSTRUCTION
• COORDINATION OF ALL TRADES

RESUBMITTALS SHALL BE CLOUDED AND DATED FOR ALL CHANGES TO THE SUBMITTAL. ONLY CLOUDED PORTIONS OF RESUBMITTAL WILL BE REVIEWED AND SEOR'S REVIEW STAMP APPLIES TO ONLY THESE AREAS.

SUBSTITUTIONS
SUBSTITUTIONS SHALL BE SUBMITTED IN WRITING PRIOR TO SUBMITTAL OF SHOP DRAWINGS. SHOP DRAWINGS BEARING SUBSTITUTIONS WILL BE REJECTED. SUBMIT ENGINEERING DATA TO SUBSTANTIATE THE EQUIVALENCE OF THE PROPOSED ITEMS. THE SEOR'S BASIC SERVICES CONTRACT DOES NOT INCLUDE REVIEW OF SUBSTITUTIONS THAT REQUIRE RE-ENGINEERING OF THE ITEM OR ADJACENT STRUCTURE. NOR DOES THE SEOR'S CONTRACT COVER EXCESSIVE REVIEW OF PROPOSED SUBSTITUTIONS. THE FEES FOR MAKING THESE REVIEWS AND/OR REDESIGN SHALL BE PAID BY THE CONTRACTOR. REVIEWS AND APPROVALS SHALL NOT BE MADE UNTIL AUTHORIZATION IS RECEIVED.

SUBMITTALS
SHOP DRAWINGS AND MATERIAL SUBMITTALS SHALL BE SUBMITTED TO THE ARCHITECT AND SEOR PRIOR TO ANY FABRICATION OR CONSTRUCTION FOR THE FOLLOWING STRUCTURAL ITEMS. SUBMITTALS SHALL BE ELECTRONICALLY SHARED, IN PDF FORMAT. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWINGS SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE SEOR, THE CONTRACT DOCUMENTS CONTROL AND SHALL BE FOLLOWED.

• CONSTRUCTION SEQUENCE DESCRIPTION
• CONTRACTOR QUALITY CONTROL TESTING PROCEDURES WHEN REQUIRED IN SPECIFICATIONS
• CONCRETE MIX DESIGNS
• CONCRETE CONSTRUCTION CONTROL JOINT PLANS AND ELEVATIONS
• CONCRETE ACCESSORIES MATERIAL SPECIFICATION, SIZE AND LOCATION
• REINFORCING BAR SHOP DRAWINGS AND PLACING PLANS
• REINFORCING BAR MILL CERTIFICATES SHALL BE AVAILABLE UPON REQUEST
• NON-SHRINK GROUT MATERIAL SPECIFICATIONS AND MANUFACTURER'S INSTALLATION RECOMMENDATIONS
• COLD-FORMED STEEL SHOWN ON STRUCTURAL DRAWINGS
• DEFERRED STRUCTURAL COMPONENTS LISTED BELOW

DEFERRED STRUCTURAL COMPONENTS
COMPONENTS REFERRED TO AS DEFERRED STRUCTURAL COMPONENTS SHALL COMPLY WITH THESE NOTES. THESE ELEMENTS HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. THE CONTRACTOR WILL BE REQUIRED TO SUBMIT THE COMPONENT SYSTEM DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL. THE DOCUMENTS SHALL BE STAMPED AND SIGNED BY AN ENGINEER LICENSED BY THE STATE WHERE THE PROJECT IS LOCATED. THE DEFERRED STRUCTURAL COMPONENTS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

PRIOR TO BUILDING DEPARTMENT SUBMITTAL, THE DEFERRED STRUCTURAL COMPONENTS SUBMITTALS SHALL RECEIVE CURSORY REVIEW BY SEOR FOR LOADS IMPOSED ON PRIMARY STRUCTURE AND GENERAL CONFORMANCE WITH DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE INFORMATION GIVEN IN THE STRUCTURAL CONTRACT DOCUMENTS. REVIEW OF SUBMITTALS DOES NOT CONSTITUTE APPROVAL OR ACCEPTANCE OF UNAUTHORIZED DEVIATION FROM CONTRACT DOCUMENTS. SUBMITTALS OF CONTRACTOR-DESIGNED COMPONENTS SHALL INCLUDE THE DESIGNING PROFESSIONAL ENGINEER'S STAMP AND SIGNATURE, AS NOTED ABOVE. THE SUBMITTAL SHALL BE APPROVED BY THE COMPONENT VENDOR PRIOR TO REVIEW BY THE SEOR. THE DESIGNING PROFESSIONAL IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL CONTRACT DOCUMENTS.

SUBMITTALS SHALL INCLUDE DETAILS OF CONNECTIONS TO PRIMARY STRUCTURE THAT INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED AT POINT OF CONNECTION. DESIGN CRITERIA SHALL BE PROVIDED WITH SUBMITTAL AND CALCULATIONS SHALL BE MADE AVAILABLE UPON REQUEST.

THE FOLLOWING LIST INCLUDES THE ITEMS THAT ARE DEFINED AS DEFERRED STRUCTURAL COMPONENTS. REFER TO OTHER DISCIPLINE'S CONTRACT DOCUMENTS FOR ADDITIONAL DEFERRED COMPONENTS THAT MAY REQUIRE STRUCTURAL DESIGN AND DETAILS. CONNECTIONS OF THESE ELEMENTS SHALL NOT INDUCE TORSION ON STRUCTURAL MEMBERS. DEFERRED STRUCTURAL COMPONENTS SHALL BE MANUFACTURED, DELIVERED, HANDLED, STORED, AND FIELD ERECTED IN CONFORMANCE WITH INSTRUCTIONS PREPARED BY THE COMPONENT VENDOR.

DEFERRED STRUCTURAL COMPONENTS:
• MECHANICAL UNIT AND EQUIPMENT ANCHORAGE AND SUPPORTS WHERE REQ'D AND NOT DETAILED
• MEP DISTRIBUTION SUPPORT AND BRACING, OTHER STRUT SYSTEMS
• LIGHT-GAGE METAL STUD SYSTEMS

INSPECTIONS & STRUCTURAL OBSERVATIONS

INSPECTIONS BY BUILDING OFFICIAL
THE BUILDING OFFICIAL, UPON NOTIFICATION, SHALL MAKE STRUCTURAL INSPECTIONS AS REQUIRED BY LOCAL ORDINANCE. THE INSPECTION BY THE BUILDING OFFICIAL PER IBC SECTION 110 WILL BE SEPARATE FROM, AND IN ADDITION TO, THE STRUCTURAL OBSERVATION(S) AND SPECIAL INSPECTION(S) MENTIONED SUBSEQUENTLY.

STRUCTURAL OBSERVATION
WHERE REQUIRED BY IBC SECTION 1704.6 THE OWNER OR THE OWNER'S AUTHORIZED AGENT SHALL EMPLOY A STRUCTURAL OBSERVER TO PERFORM STRUCTURAL OBSERVATIONS. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR INSPECTIONS OR SPECIAL INSPECTIONS REQUIRED BY THE BUILDING CODE AND MENTIONED HEREIN. THE STRUCTURAL OBSERVER SHALL BE ONE OF THE FOLLOWING INDIVIDUALS: THE SEOR, OR A REGISTERED DESIGN PROFESSIONAL DESIGNATED BY THE SEOR.

AT THE CONCLUSION OF THE WORK INCLUDED IN THE PERMITTED CONTRACT DOCUMENTS, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT THAT THE SITE VISITS HAVE BEEN MADE AND SHALL IDENTIFY ANY REPORTED DEFICIENCIES THAT, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN RESOLVED.

THE FOLLOWING LIST INCLUDES THE ITEMS REQUIRING STRUCTURAL OBSERVATION:

FOUNDATIONS:
• CONCRETE FOUNDATIONS REINFORCEMENT PRIOR TO CONCRETE PLACEMENT
CFS:
• COLD-FORMED STEEL SHEAR WALLS (HARDWARE AND FASTENERS INSTALLED) PRIOR TO FINISHES INSTALLED
• COLD-FORMED STEEL ROOF FRAMING

SPECIAL INSPECTIONS
A SPECIAL INSPECTOR SHALL BE HIRED BY THE OWNER TO PERFORM THE FOLLOWING SPECIAL INSPECTIONS PER IBC SECTION 1704. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR SPECIAL INSPECTION AND TESTING. THE ARCHITECT, STRUCTURAL ENGINEER, AND BUILDING DEPARTMENT SHALL BE FURNISHED WITH COPIES OF ALL INSPECTION REPORTS AND TEST RESULTS.

EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A SEISMIC FORCE RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM, OR COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE WRITTEN STATEMENT SHALL BE IN ACCORDANCE WITH IBC SECTION 1704.4.

SEE IBC CHAPTER 17: "SPECIAL INSPECTIONS AND TESTS" FOR MORE DETAILED REQUIREMENTS.

SPECIAL INSPECTIONS OF COLD-FORMED STEEL (PER IBC 1705.2.2, 1705.12.2 & 1705.13.3)

VERIFICATION AND INSPECTION	FREQUENCY		REFERENCE
	CONT.	PERIODIC	
INSTALLATION OF TEMPORARY AND PERMANENT RESTRAINT/BRACING OF COLD-FORMED STEEL TRUSSES SPANNING 60 FEET OR GREATER		X	1705.2.4
QUALIFICATION OF WELDING AND SPECIAL INSPECTION OF COLD-FORMED STEEL FLOOR AND ROOF DECK		X	1705.2.2 SDI QA/QC
WELDING OPERATIONS OF ELEMENTS OF THE MAIN WIND AND SEISMIC FORCE RESISTING SYSTEM AND SCREW INSTALLATION, BOLTING, ANCHORING AND OTHER FASTENING TO COMPONENTS (SPACED 4" OC OR CLOSER) WITHIN THE MAIN WIND AND SEISMIC FORCE RESISTING SYSTEM, INCLUDING DRAG STRUTS, BRACES, HOLDDOWNS, SHEAR WALLS, AND DIAPHRAGMS		X	1705.12.2 1705.13.3

SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION (PER IBC 1705.3)

VERIFICATION AND INSPECTION	FREQUENCY		REFERENCE
	CONT.	PERIODIC	
INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT		X	ACI 318: 20, 25.2-3, 26.6.1-3
REINFORCING BAR WELDING: • VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706 • INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16" • INSPECT ALL OTHER WELDS		X	AWS D1.4 ACI 318: 26.6.4
INSPECTION OF ANCHORS CAST IN CONCRETE		X	ACI 318: 26.13.3
INSPECTION OF ANCHORS AND REINFORCING BAR POST-INSTALLED IN HARDENED CONCRETE MEMBERS: • ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS • MECHANICAL ANCHORS, ADHESIVE ANCHORS AND REINFORCING BAR NOT DEFINED ABOVE		X	ACI 318: 26.13.3.2 ACI 318: 26.13.3
VERIFYING USE OF REQUIRED DESIGN MIX		X	IBC 1904.1, 1904.2, ACI 318: 19, 26.4.3, 26.4.4
PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X		ACI 318: 26.5, 26.12 ASTM C172, C31
INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X		ACI 318: 26.5
VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X	ACI 318: 26.5.3-5
FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR HDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E, OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD FOR: • INSTALLATION OF THE EMBEDDED PARTS • COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS • COMPLETION OF CONNECTIONS IN THE FIELD		X	ACI 318: 26.13.1.3, ACI 550.5
INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE BEING FORMED		X	ACI 318: 26.11.1.2(B)

GEOTECHNICAL

GENERAL CRITERIA
ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE ASSUMED AND THEREFORE MUST BE VERIFIED BY A GEOTECHNICAL INSPECTOR OR THE BUILDING OFFICIAL. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED, NOTIFY THE STRUCTURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.

UNLESS NOTED OTHERWISE, FOOTINGS SHALL BE CENTERED BELOW COLUMNS OR WALLS.

INSPECTIONS
ALL PREPARED SOIL-BEARING SURFACES SHALL BE INSPECTED BY THE OWNERS GEOTECHNICAL INSPECTOR (OR BUILDING OFFICIAL) PRIOR TO PLACEMENT OF REINFORCING STEEL AND CONCRETE. INSPECTIONS SHALL BE MADE PER IBC TABLE 1705.6.

BEARING VALUES
ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL AND SHALL BE LOWERED TO FIRM BEARING IF SUITABLE SOIL IS NOT FOUND AT ELEVATIONS SHOWN. EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 18" BELOW THE FINISHED GROUND SURFACE. FOOTING ELEVATIONS SHOWN ON PLANS (OR IN DETAILS) ARE MINIMUM DEPTHS AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS OF FOOTINGS MUST BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD WORKING WITH THE GEOTECHNICAL INSPECTOR.

SOIL ALLOWABLE BEARING PRESSURE = 1,500 PSF

SOIL ALLOWABLE BEARING PRESSURE MAY BE INCREASED BY 1/3 UNDER TRANSIENT LOADS (WIND, SEISMIC, IMPACT).

SUBGRADE PREPARATION
PREPARE SUBGRADE, SUMMARIZED AS FOLLOWS: ALL FOOTINGS SHALL BE CAST ON UNDISTURBED FIRM NATURAL SOILS THAT ARE FREE OF ORGANIC MATERIALS. FOOTING EXCAVATION SHALL BE FREE OF LOOSE SOILS, SLOUGHS, DEBRIS AND FREE OF WATER AT ALL TIMES. IF ORGANIC SILT AND/OR FILL MATERIAL IS ENCOUNTERED AT SUBGRADE ELEVATIONS, OVEREXCAVATE A MINIMUM OF 2'-0" BELOW THE DESIGN FOUNDATION SUBGRADE ELEVATION PRIOR TO PLACING FOOTINGS. THE OVEREXCAVATED AREAS SHALL BE BACKFILLED WITH STRUCTURAL FILL COMPACTED TO 95% PROCTOR PER ASTM D-1557 OR D698 OR A LEAN CONCRETE MIX.

EXISTING UTILITIES
THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO ANY EXCAVATION, SHORING, PILE DRIVING, OR PIER DRILLING. ANY UTILITY INFORMATION SHOWN ON THE PLANS AND DETAILS ARE APPROXIMATE AND NOT VERIFIED BY THE SEOR. CONTRACTOR IS TO PROVIDE PROTECTION OF ANY UTILITIES OR UNDERGROUND STRUCTURES DURING CONSTRUCTION.

DRAINAGE
DRAINAGE SYSTEMS, INCLUDING FOUNDATION, ROOF AND SURFACE DRAINS, SHALL BE INSTALLED AS DIRECTED BY THE CIVIL DRAWINGS. VAPOR RETARDER PLACED BELOW SLAB-ON-GRADE SHALL CONFORM TO ASTM E1643 AND ASTM E1745.

CONCRETE

CAST-IN-PLACE CONCRETE
CONCRETE MATERIALS SHALL CONFORM TO THE FOLLOWING:
PORTLAND CEMENT: TYPE I, ASTM C150 OR TYPE 1L, ASTM C595
FLY ASH (IF USED): ASTM C618 CLASS F OR C
SLAG CEMENT (IF USED): ASTM C989
LIGHTWEIGHT AGGREGATES: LIGHTWEIGHT AGGREGATES SHALL NOT BE USED WITHOUT PRIOR APPROVAL OF SEOR AND BUILDING DEPARTMENT
NORMAL WEIGHT AGGREGATES: ASTM C33
SAND EQUIVALENT: ASTM C33
WATER: POTABLE PER ASTM C94
AIR ENTRAINING ADMIXTURES: ASTM C260
CHEMICAL ADMIXTURES: ASTM C494
FLOWABLE CONCRETE ADMIXTURES: ASTM C1017

DURABILITY REQUIREMENTS OF CONCRETE MIXES SHALL CONFORM TO BUILDING CODE. THESE REQUIREMENTS INCLUDE WATER-CEMENTITIOUS MATERIAL RATIOS, MINIMUM COMPRESSIVE STRENGTHS, AIR ENTRAINMENT, TYPE OF CEMENT, AND MAXIMUM CHLORIDE ION CONTENT.

CONCRETE STRENGTH REQUIREMENTS TABLE

LOCATION	STRENGTH ¹ F _c (PSI)	MAX AGG SIZE	MAX W/C RATIO	TOTAL AIR CONTENT	EXPOSURE CATEGORIES AND CLASSES			
					F	S	W	C
FOUNDATIONS, GRADE BEAMS AND INTERIOR STEM WALLS	4,000	1"	0.44	4.5%	F1	S0	W0	C1

¹ 28-DAY CONCRETE STRENGTH, UNLESS NOTED OTHERWISE
* 28-DAY CONCRETE STRENGTH
^ 3-DAY CONCRETE STRENGTH

CONCRETE MIXTURES
MIXES SHALL BE PROPORTIONED TO MEET COMPLIANCE REQUIREMENTS OF ACI 318 SECTION 26.4.3. SLUMP, W/C RATIO, ADMIXTURES AND AGGREGATE SIZE WILL BE DETERMINED BY THE CONTRACTOR. SUBMIT DOCUMENTATION OF CONCRETE MIXTURE CHARACTERISTICS FOR REVIEW BY THE SEOR BEFORE THE MIXTURE IS USED AND BEFORE MAKING CHANGES TO MIXTURES ALREADY IN USE. DOCUMENTATION SHALL COMPLY WITH ACI 318 SECTION 26.4.4 OR CITY OF SEATTLE DIRECTOR'S RULE 11-2014.

ALL CONCRETE, INCLUDING SLAB ON GRADE, SHALL CONTAIN AN ACCEPTABLE WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494 AND BE USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

ALL CONCRETE WHICH IS EXPOSED TO FREEZING AND THAWING IN A MOIST CONDITION OR EXPOSED TO DEICING CHEMICALS SHALL CONTAIN AN AIR ENTRAINING AGENT, CONFORMING TO ASTM C260. TOTAL AIR CONTENT SHALL BE ADJUSTED PER ACI 318 FOR MIX DESIGNS WITH SMALLER NOMINAL AGGREGATE SIZE. THE AMOUNT OF ENTRAINED AIR SHALL BE MEASURED AT THE DISCHARGE END OF THE PLACING NOZZLE. ENTRAINED AIR SHALL BE AS NOTED ± 1.0% BY VOLUME. AIR-ENTRAINMENT SHALL NOT BE USED AT SLABS THAT WILL RECEIVE A SMOOTH, DENSE, HARD-TROWELED FINISH.

TRUCKS HAULING PLANT-MIXED CONCRETE SHALL ARRIVE ON-SITE WITH A FIELD TICKET INDICATING THE MAXIMUM GALLONS OF WATER THAT CAN BE ADDED AT THE SITE NOT TO EXCEED THE TOTAL WATER CONTENT IN THE APPROVED MIX DESIGN.

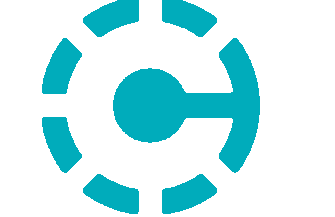
CONCRETE SHALL BE DEPOSITED AS NEARLY AS PRACTICABLE IN ITS FINAL POSITION TO AVOID SEGREGATION DUE TO REHANDLING OR FLOWING. CONCRETE SHALL BE THOROUGHLY CONSOLIDATED BY SUITABLE MEANS DURING PLACEMENT AND SHALL BE THOROUGHLY WORKED AROUND REINFORCEMENT, EMBEDDED ITEMS, AND INTO CORNERS OF FORMS.

FORMWORK AND ACCESSORIES
CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE" AND THE BUILDING CODE, INCLUDING TESTING PROCEDURES. SEE SPECIFICATIONS AND/OR ARCHITECTURAL DOCUMENTS FOR FORMWORK REQUIREMENTS. INSTALLATION SHALL ADHERE TO ACI 301. CONDUITS AND PIPES OF ALUMINUM SHALL NOT BE EMBEDDED IN CONCRETE CONSTRUCTION.

SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS OF DOOR AND WINDOW OPENINGS IN ALL CONCRETE WALLS. SEE MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF MECHANICAL OPENINGS THROUGH CONCRETE WALLS. SEE ARCHITECTURAL DRAWINGS FOR ALL GROOVES, NOTCHES, CHAMFERS, FEATURE STRIPS, COLOR, TEXTURE, AND OTHER FINISH DETAILS AT ALL EXPOSED CONCRETE SURFACES, BOTH CAST-IN-PLACE AND PRECAST. SEE STRUCTURAL DETAILS FOR REINFORCING AROUND OPENINGS.



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DATE: 2026_1019
DRAWINGS ISSUED FOR: Issued for Permit
title: subcontract no.: 250500012-31541

CONCRETE (CONT'D)

CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE ARCHITECT FOR ACCEPTANCE BEFORE STARTING CONSTRUCTION. ERICO LENTON FORMASAYER (ER-0129) MAY BE USED AS AN ALTERNATE TO THE ROUGHENED JOINT. ALL CONSTRUCTION, CONTROL, AND ISOLATION JOINTS FOR SLABS ON GROUND SHALL BE IN ACCORDANCE WITH THE TYPICAL DETAILS.

CONCRETE ACCESSORIES AND EMBEDDED ITEMS SHALL BE COORDINATED WITH ARCHITECTURAL AND ALL OTHER CONTRACT DOCUMENTS AND SUPPLIERS' DRAWINGS BEFORE PLACING CONCRETE. WET-SETTING OF ANCHOR RODS, REINFORCING, HARDWARE, ETC. IS NOT ALLOWED IN CONCRETE. ANCHOR RODS, REINFORCING, HARDWARE, ETC. SHALL BE FIRMLY TIED IN PLACE PRIOR TO CONCRETE PLACEMENT.

REFER TO ARCHITECTURAL DOCUMENTS FOR WATERSTOPS, DAMP PROOFING, AND SOIL RETAINING WALL DRAINAGE REQUIREMENTS AT CONCRETE AND AT CONCRETE JOINTS (CONSTRUCTION JOINTS, SLAB TO WALL JOINTS, CURB TO SLAB JOINTS, ETC).

CURING AND FINISHES

PROTECT AND CURE FRESHLY PLACED CONCRETE PER ACI 305.1 IN HOT CONDITIONS, ACI 306.1 IN COLD CONDITIONS, AND ACI 308.1 " SPECIFICATION FOR CURING CONCRETE". ALL EXPOSED EDGES AND CORNERS SHALL HAVE 3/4" CHAMFER, UNO. CONCRETE FLATWORK SHALL BE SLOPED TO PROVIDE POSITIVE DRAINAGE. COORDINATE FINISH WITH ARCHITECTURAL CONTRACT DOCUMENTS.

AT THE TIME OF APPLICATION OF FINISH MATERIALS OR SPECIAL TREATMENT TO CONCRETE, MOISTURE CONTENT OF CONCRETE SHALL CONFORM TO REQUIREMENTS IN FINISH MATERIAL SPECIFICATIONS. WHERE VAPOR SENSITIVE COVERINGS ARE TO BE PLACED ON SLABS ON GRADE, CONFORM STRICTLY TO SLAB COVERING MANUFACTURER'S RECOMMENDATIONS REGARDING VAPOR RETARDER AND GRANULAR FILL REQUIREMENTS BELOW THE SLAB.

CONCRETE CRACK MAINTENANCE

CRACKING OCCURS IN CONCRETE STRUCTURES DUE TO INHERENT SHRINKAGE, CREEP, AND THE RESTRAINING EFFECTS OF WALLS AND OTHER STRUCTURAL ELEMENTS. MOST CRACKING DUE TO SHRINKAGE AND CREEP WILL LIKELY OCCUR OVER THE FIRST TWO YEARS OF THE LIFE OF THE STRUCTURE; FURTHER CONCRETE MOVEMENT DUE TO VARIATIONS IN TEMPERATURE MAY PERSIST. CRACKS THAT RESULT IN WATER PENETRATION WILL NEED TO BE REPAIRED TO PROTECT REINFORCING. OTHER CRACKING MAY BE REPAIRED AT THE OWNER'S DISCRETION FOR AESTHETICAL REASONS OR PERFORMANCE OF APPLIED FINISHES. PRIOR TO REPAIRING CRACKS, A STRUCTURAL ENGINEER SHOULD BE CONSULTED TO PROVIDE DIRECTION ON WHICH CRACKS TO REPAIR AND ON WHETHER OBSERVED CRACKS MAY AFFECT THE STRENGTH OF THE STRUCTURE.

REINFORCEMENT IN CONCRETE

REINFORCING STEEL

REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, F_y = 60,000 PSI, UNO.

PROCEDURES

REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT". LAP ALL REINFORCEMENT IN ACCORDANCE WITH "THE REINFORCING SPICE AND DEVELOPMENT LENGTH SCHEDULE" ON THESE DOCUMENTS. IF TABLE IS NOT PROVIDED, LAP ALL REINFORCING BY 40 BAR DIAMETERS. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS.

REINFORCING STEEL SHALL BE ADEQUATELY SUPPORTED TO PREVENT DISPLACEMENT DURING CONCRETE AND GROUT PLACEMENT. BARS SHALL BE BENT COLD.

BARS PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT, UNLESS SPECIFICALLY SO DETAILED OR APPROVED BY THE SEOR.

MECHANICAL CONNECTION OF CONTINUOUS REINFORCING BAR SHALL BE USED WHERE SHOWN ON DOCUMENTS AND MAY BE SUBSTITUTED FOR LAP SPICES IF APPROVED BY THE SEOR. SUCH CONNECTIONS SHALL DEVELOP AT LEAST 125% OF THE SPECIFIED YIELD STRENGTH OF THE BAR. ACCEPTABLE CONNECTORS SHALL BE THE ERICO LENTON STANDARD COUPLER (ER-0129), DAYTON SUPERIOR BAR-LOCK L SERIES (ER-0319), OR APPROVED EQUAL.

MECHANICAL ANCHORAGE OF REINFORCING BAR SHALL BE USED WHERE SHOWN ON DOCUMENTS AND MAY BE SUBSTITUTED FOR STRAIGHT OR HOOKED REINFORCING BAR DEVELOPMENT LENGTHS IF APPROVED BY THE SEOR. SUCH CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF ACI 318. ACCEPTABLE CONNECTORS SHALL BE THE ERICO LENTON TERMINATOR (ER-0188), HEADED BAR COUPLERS MANUFACTURED BY THE HEADED REINFORCEMENT CORP. (ER-177), OR APPROVED EQUAL.

WELDING OR TACK WELDING OF REINFORCING BARS TO OTHER BARS OR TO PLATES, ANGLES, ETC, IS PROHIBITED, EXCEPT WHERE SPECIFICALLY APPROVED BY THE SEOR. WHERE WELDING IS APPROVED, IT SHALL BE DONE BY AWS/WABO-CERTIFIED WELDERS USING E9018 OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.4. ANY GRADE 60 REINFORCING BARS INDICATED ON DRAWINGS TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCEMENT COMPLYING WITH ASTM A615 (S1) MAY BE WELDED ONLY IF MATERIAL PROPERTY REPORTS INDICATING CONFORMANCE WITH WELDING PROCEDURES SPECIFIED IN AWS D1.4 ARE SUBMITTED. WELDING WITHIN 4" OF COLD BENDS IN REINFORCING STEEL IS NOT PERMITTED.

POST-INSTALLED ANCHORS INTO CONCRETE

POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE SEOR PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR; EXISTING REINFORCEMENT SHALL NOT BE CUT OR DAMAGED.

INSTALL ANCHORS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS AND APPLICABLE EVALUATION REPORTS, INCLUDING MINIMUM EMBEDMENT REQUIREMENTS. DRILL AND CLEAN HOLES IN FULL COMPLIANCE WITH THE MANUFACTURER'S REQUIREMENTS. PROVIDE HOLE DIAMETERS AS RECOMMENDED BY THE MANUFACTURER.

SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE SEOR, ALONG WITH APPLICABLE EVALUATION REPORT OR CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE REPORT OR CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S). ANCHORS SHALL HAVE A CURRENT ICC-ES (OR IAPMO-UES EQUIVALENT) REPORT INDICATING THAT THE ANCHOR IS PERMITTED FOR RESISTING SEISMIC LOADS IN CRACKED CONCRETE.

SPECIAL INSPECTION OF INSTALLATION IS REQUIRED. CONTACT MANUFACTURER'S REPRESENTATIVE FOR THE INITIAL TRAINING AND INSTALLATION OF ANCHORS AND FOR PRODUCT-RELATED QUESTIONS AND AVAILABILITY.

MECHANICAL ANCHORAGE IN CONCRETE

MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND/OR ICC-ES ACI 937 FOR CRACKED AND UNCRACKED CONCRETE. PRE-APPROVED PRODUCTS INCLUDE:

EXPANSION ANCHORS:
HILTI "KWIK BOLT-TZ2" (ESR-4266)

ADHESIVE ANCHORAGE IN CRACKED AND UNCRACKED CONCRETE

ADHESIVE ANCHORS REFER TO THREADED RODS OR REINFORCING BAR, AS SPECIFIED ON THE DRAWINGS. ADHESIVE ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED AND UNCRACKED CONCRETE. ADHESIVE ANCHOR EVALUATION MUST ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE, INSTALLATION TEMPERATURE, MOISTURE CONDITION OF CONCRETE, AND DRILLING METHODS. RODS SHALL BE ASTM F1554 GR. 55, UNLESS NOTED OTHERWISE.

HILTI "HIT-HY 200 V3" (ESR-4868)

GENERAL CFS FRAMING CRITERIA

WALL FRAMING

UNLESS NOTED OTHERWISE, STRUCTURAL WALL STUDS SHALL BE ASSUMED TO HAVE A MINIMUM DEPTH OF 3 1/2", MINIMUM FLANGE OF 1 5/8", MINIMUM THICKNESS OF 43 MILS AND MAXIMUM SPACING OF 16" OC. WALL STUDS SHALL BE INSTALLED PLUMB (WITHIN 1/8" OVER 10'-0"). SPICES IN WALL STUDS ARE PROHIBITED. STUDS SHALL BE SEATED TIGHTLY WITHIN TRACKS WITH A MAXIMUM GAP TOLERANCE OF 1/8". THIS TOLERANCE SHALL BE REDUCED TO 1/16" WHEN BEARING STUDS ARE 68 MILS OR THICKER OR WHEN STUDS ARE SUPPORTING MORE THAN TWO STORIES. TRACK SECTIONS SHALL BE UNPUNCHED AND HAVE AT LEAST 1 1/4" FLANGES (1 1/2" FLANGES AT EXTERIOR STUDS) AND SHALL HAVE THE SAME GAUGE THICKNESS AS THE SUPPORTED STUDS, UNLESS NOTED OTHERWISE .

TWO STUDS MINIMUM SHALL BE PROVIDED AT THE ENDS OF ALL WALLS, EACH SIDE OF ALL OPENINGS, AND BELOW BEAM BEARING POINTS. SOLID BLOCKING FOR MULTI-STUD OR STEEL COLUMNS SHALL BE PROVIDED THROUGH INTERMEDIATE LEVELS TO SUPPORTS BELOW. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE WELDED TO EACH OTHER IN ACCORDANCE WITH THE DETAILS. REFER TO THE PLANS AND SHEAR WALL SCHEDULE FOR REQUIRED WALL SHEATHING. UNLESS NOTED OTHERWISE, PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES AND GYPSUM SHEATHING ON EXTERIOR SURFACES SCREWED TO ALL STUDS, TOP AND BOTTOM TRACKS, AND BLOCKING WITH SCREWS @ 7" OC.

PROVIDE CONTINUOUS COLD-ROLLED CHANNEL LATERAL BRACING PER DETAILS AT LESSER SPACING OF: 1/3 POINTS OF WALL HEIGHT OR 48" OC MAXIMUM VERTICALLY AT ALL LOAD BEARING AND ALL EXTERIOR STUD WALLS 6" OR LESS IN WIDTH. AT 8" OR WIDER LOAD BEARING AND/OR EXTERIOR STUD WALLS, SUBSTITUTE CONTINUOUS STRAPS AND INTERMITTENT BLOCKING PER DETAILS FOR COLD ROLLED CHANNEL LATERAL BRACING.

FLOOR FRAMING

JOISTS AND TRUSSES SHALL BE INSTALLED PLUMB AND LEVEL (UNLESS NOTED OTHERWISE) WITH A MINIMUM 1 1/2" BEARING WIDTH. FLOOR JOIST WEBS SHALL NOT BE IN DIRECT CONTACT WITH THE RIM TRACK WEBS WITH AN ALLOWABLE GAP UP TO 1/4". THE BOTTOM FLANGES OF FLOOR JOISTS SPANNING GREATER THAN 12'-0" SHALL BE BRACED WITH GYPSUM BOARD INSTALLED WITH #6 SCREWS @ 12" OC OR 1 1/2" x 33 MILS STRAPS SPACED NO GREATER THEN 12'-0" OC. FLAT STRAPPING SHALL BE ATTACHED TO THE BOTTOM FLANGE OF EACH JOIST WITH ONE #8 SCREW AND TO FULL-DEPTH BLOCKING WITH TWO #8 SCREWS. FULL DEPTH BLOCKING SHALL BE INSTALLED AT THE FIRST AND LAST JOIST BAYS AND AT INTERMEDIATE BAYS WITH MAXIMUM SPACING OF 12'-0" OC.

ROOF AND CEILING FRAMING

CEILING JOISTS AND ROOF TRUSSES SHALL BE INSTALLED PLUMB AND LEVEL (UNLESS NOTED OTHERWISE) WITH A MINIMUM 1 1/2" BEARING WIDTH. THE TOP AND BOTTOM FLANGES OF CEILING JOISTS SPANNING GREATER THAN 12'-0" SHALL BE BRACED WITH GYPSUM BOARD INSTALLED WITH #6 SCREWS @ 12" OC OR 1 1/2" x 33 MILS STRAPS SPACED NO GREATER THEN 12'-0" OC. FLAT STRAPPING SHALL BE ATTACHED TO THE BOTTOM FLANGE OF EACH JOIST WITH ONE #8 SCREW AND TO FULL-DEPTH BLOCKING WITH TWO #8 SCREWS. FULL DEPTH BLOCKING SHALL BE INSTALLED AT THE FIRST AND LAST JOIST BAYS AND AT INTERMEDIATE BAYS WITH MAXIMUM SPACING OF 12'-0" OC. THE TRUSS MANUFACTURER SHALL DESIGN AND PROVIDE ALL PERMANENT BRACING FOR COLD-FORMED STEEL TRUSSES.

PLYWOOD SHEATHING

SEE PLANS FOR THICKNESS, PANEL IDENTIFICATION INDEX AND SCREW ATTACHMENT REQUIREMENTS. UNLESS NOTED OTHERWISE ON PLANS:

- ROOF SHEATHING SHALL BE 19/32" WITH SPAN RATING 32/16.
- FLOOR SHEATHING SHALL BE 23/32" WITH SPAN RATING 48/24.
- WALL SHEATHING SHALL BE 15/32" WITH SPAN RATING 24/0.

WEB HOLES AND FIELD CUTS/NOTCHES

FACTORY INSTALLED OR FIELD CUT WEB HOLES SHALL BE CENTERED ON WEB DEPTH AND SHALL NOT BE SPACED CLOSER THAN 24" OC OR LOCATED CLOSER THAN 10" FROM A BEARING CONDITION OR END OF A MEMBER. WEB HOLE SIZES SHALL BE LIMITED TO ONE-HALF THE WEB DEPTH (2 1/2" MAXIMUM) x 4 1/2" LONG IN THE MEMBER DIRECTION. WEB HOLES NOT CONFORMING TO THESE REQUIREMENTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND POSSIBLE ADDITION OF REINFORCING.

ALL MEMBERS SHALL BE CUT SQUARELY WITH A SAW OR SHEARS. FLAME CUTTING AND WIRE TYING OF MEMBERS ARE PROHIBITED. NO FIELD CUTS OR NOTCHES ARE ALLOWED THROUGH THE FLANGES OR LIPS OF ANY STRUCTURAL MEMBERS.

COLD-FORMED STEEL

MATERIAL

COLD-FORMED STEEL MATERIAL SHALL BE MANUFACTURED AND FORMED, PER ASTM A1003/A1003M, FROM GALVANIZED ASTM A653 SS GRADE 50 STEEL FOR 54, 68 AND 97 MIL BASE THICKNESS MATERIAL AND FROM GALVANIZED ASTM A653 SS GRADE 33 MATERIAL FOR 43 AND 33 MIL BASE THICKNESS MATERIAL, UNO. WHERE NOTED, PAINTED COLD-FORMED STEEL MATERIAL SHALL CONFORM TO ASTM A1011 SS GRADE 80. MINIMUM COLD-FORMED STEEL ACCEPTANCE CRITERIA SHALL BE PER ICC-ES AC46.

ALL GALVANIZED MEMBERS SHALL CONFORM TO ASTM A924 WITH THE FOLLOWING MINIMUM COATING REQUIREMENTS: NON-STRUCTURAL MEMBERS (ASTM C645 - G40 COATING), STRUCTURAL MEMBERS (ASTM C955 - G60 COATING), EXPOSED EXTERIOR MEMBERS (ASTM C955 - G90 COATING).

EACH MEMBER SHALL BEAR A LEGIBLE STICKER, STAMP, STENCIL, OR EMBOSSEMENT, SPACED A MAXIMUM OF 48"OC ON THE WEB OF THE FRAMING MEMBER, INDICATING THE MINIMUM STEEL SHEET THICKNESS, METALLIC COATING DESIGNATION, MINIMUM YIELD STRENGTH, PRODUCT DESIGNATION, AND NAME OF MANUFACTURER. WHERE MEMBERS ARE NOT LABELED, THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THAT THE STEEL IS IN COMPLIANCE WITH THE PROJECT SPECIFICATIONS.

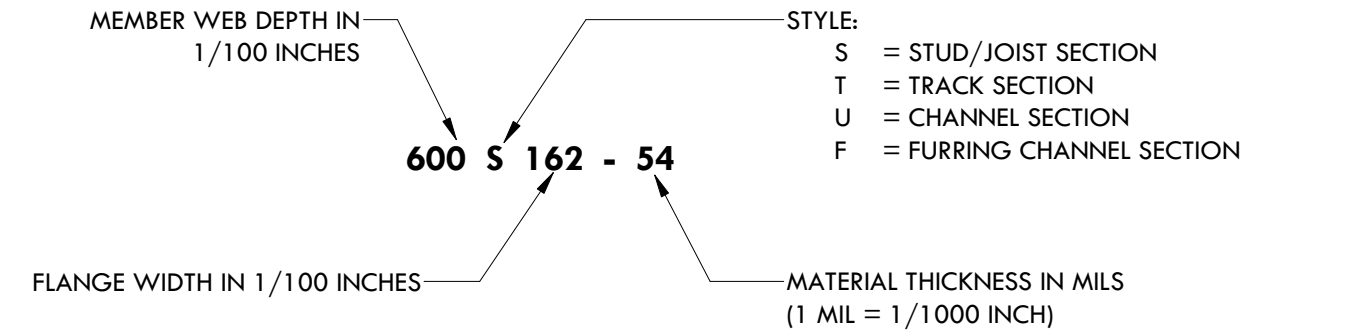
SUBMITTALS

PRODUCT DATA FOR ALL MEMBERS, ACCESSORIES, AND FASTENERS SHALL BE SUBMITTED TO THE ARCHITECT, STRUCTURAL ENGINEER, AND BUILDING DEPARTMENT FOR REVIEW AND ACCEPTANCE PRIOR TO FABRICATION AND ERECTION. FRAMING SUBSTITUTIONS SHALL BE SUBJECT TO REVIEW AND ACCEPTANCE BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION AND INSTALLATION.

COLD-FORMED STEEL FRAMING

ALL COLD-FORMED STEEL FRAMING SHALL BE IN ACCORDANCE WITH AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", AS AMENDED BY THE INTERNATIONAL BUILDING CODE AND SHALL STRICTLY CONFORM WITH ICC REPORT ESR-3064P OR ESR-4205.

ALL COLD-FORMED STEEL PRODUCTS SHALL BE MANUFACTURED BY CURRENT MEMBERS OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) OR STEEL FRAMING INDUSTRY ASSOCIATION (SFIA). MATERIAL DESIGNATIONS NOTED ON THE DRAWINGS, RELATING TO MEMBER TYPES AND SIZES OR MISCELLANEOUS FRAMING ITEMS, REFER TO PRODUCT IDENTIFICATION STANDARDS ADOPTED PER AISI S200 STANDARD "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING — GENERAL PROVISIONS." PRODUCTS SHALL HAVE A FOUR-PART IDENTIFICATION CODE PER AISI S200 AS INDICATED IN THE FOLLOWING EXAMPLE:



COLD-FORMED STEEL (CONT'D)

INSTALLATION

EACH JOIST, RAFTER, TRUSS AND STRUCTURAL WALL STUDS SHALL BE ALIGNED WITHIN 3/4" FROM CENTERLINE OF HORIZONTAL FRAMING MEMBER TO CENTERLINE OF VERTICAL FRAMING MEMBER, UNO, OR AS SPECIFIED IN FIGURE C1-1 OF THE AISI STANDARD "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS". STRUCTURAL FRAMING MEMBERS SHALL BE INSTALLED PER ASTM C1007 AND NON-STRUCTURAL INTERIOR FRAMING MEMBERS PER ASTM C754.

CONCRETE BEARING SURFACES AT STRUCTURAL FRAMING SHALL PROVIDE A UNIFORM BEARING SURFACE WITH A MAXIMUM 1/4" GAP BETWEEN THE TRACK AND THE CONCRETE. STEEL BEARING SHIMS OR NON-SHRINK GROUT CAN BE USED TO ACHIEVE THIS REQUIREMENT. THE BOTTOM TRACK OF LOAD BEARING WALLS SHALL NOT EXTEND OVER THE EDGE OF FORMED CONCRETE BEARING SURFACES BELOW.

COLD-FORMED STEEL SHALL NOT BE IN DIRECT CONTACT WITH THE GROUND UNLESS NOTED OTHERWISE.

WOOD STRUCTURAL PANELS

WOOD STRUCTURAL PANELS SHALL BE APA RATED SHEATHING. PLYWOOD SHALL BE GRADE C-D OR STRUCTURAL II, EXTERIOR GLUE, EXPOSURE I DURABILITY CLASSIFICATION, IN CONFORMANCE WITH USDOC PS 1 OR PS 2, ASTM D 5457 AND SBC 2304.8.1 AND TABLE 2304.8(1)-(5).

ORIENTED STRAND BOARD (OSB), SHALL BE IN ACCORDANCE WITH USDOC PS 2, AND OF EQUIVALENT THICKNESS, EXPOSURE RATING AND SPAN RATING AND MAY BE USED IN LIEU OF PLYWOOD PENDING OSB SUBSTITUTION APPROVAL BY ARCHITECT.

SCREWS

FOR STEEL-TO-STEEL CONNECTIONS AND FOR STRUCTURAL SHEATHING-TO-STEEL CONNECTIONS SHALL BE SELF TAPPING, SELF DRILLING FASTENERS IN COMPLIANCE WITH ASTM C1513 AND SHALL HAVE A TYPE II COATING IN ACCORDANCE WITH ASTM B633 "ELECTRO-DEPOSITED COATING OF ZINC ON IRON AND STEEL". SELF-PIERCING SCREWS PER ASTM C1002 ARE PERMITTED FOR CONNECTION OF 33 MILS STEEL OR THINNER. THE SCREW MANUFACTURER SHALL PROVIDE VERIFICATION OF THE FASTENERS' RESISTANCE TO HYDROGEN EMBRITTLEMENT. SCREWS SHALL CONFORM TO SAE J78 "STANDARD SPECIFICATION FOR SELF-DRILLING TAPPING SCREWS". SCREW ACCEPTANCE SHALL BE BASED ON ICC-ES AC118 "ACCEPTANCE CRITERIA FOR TAPPING SCREW FASTENERS".

SCREW CONNECTIONS SHALL BE IN COMPLIANCE WITH THE AISI S240 STANDARD "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL STRUCTURAL FRAMING". SCREW CONNECTIONS SHALL BE MADE FROM THE LIGHTER MATERIAL INTO THE THICKER MATERIAL, UNO. SCREWS SHALL EXTEND THROUGH THE STEEL CONNECTION A MINIMUM OF THREE EXPOSED THREADS AND SHALL HAVE MINIMUM CENTER-TO-CENTER SPACING AND EDGE DISTANCES OF THREE TIMES THE NOMINAL SCREW DIAMETER. SCREWS SHALL BE INSTALLED AND TIGHTENED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SHALL NOT CAUSE PERMANENT SEPARATION BETWEEN COMPONENTS. SHEATHING FASTENERS SHALL MAINTAIN A MINIMUM 3/8" EDGE DISTANCE IN SHEATHING AND SHALL HAVE THEIR HEADS FLUSH WITH THE SHEATHING (BUT NO MORE THAN 1/16" BELOW THE SURFACE OF THE SHEATHING).

STRIPPED SCREWS IN DIRECT TENSION SHALL BE CONSIDERED INEFFECTIVE AND SHALL BE REPLACED. STRIPPED SCREWS IN SHEAR THAT CONSTITUTE MORE THAN 25% OF THE TOTAL SCREWS IN THE CONNECTION SHALL BE CONSIDERED INEFFECTIVE AND SHALL BE REPLACED. STRIPPED SCREWS ARE PERMITTED TO BE REMOVED AND REPLACED WITH SCREWS OF THE NEXT LARGER DIAMETER.

MINIMUM SCREW SIZES IN COLD-FORMED STEEL TABLE

CONNECTION		MINIMUM SCREW SIZE	
METAL TO METAL (68 MILS)		#10-16 (#3 POINT)	
METAL TO METAL (33 MILS - 54 MILS)		#8-18 (#2 POINT)	
METAL TO METAL (SHEAR WALLS)		#8-18 (#2 POINT) WAFFER HEAD	
APA SHEATHING (SHEAR WALLS)		#8-18 (#2 POINT FLAT HEAD W/ 0.285"Ø HEAD MINIMUM	
GWB OR GYPSUM SHEATHING		#6 x 1" (#2 POINT) DRYWALL	
METAL DECK TO FRAMING		#12-14 (#3 POINT)	
SIMPSON HARDWARE		PER SIMPSON CATALOG	

ALLOWABLE LOADS FOR SCREW CONNECTIONS (LBS/SCREW) SHALL BE AS INDICATED IN THE TABLE BELOW AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL EITHER PROVIDE SCREW TEST DATA FROM THE SCREW MANUFACTURER'S QUALITY ASSURANCE PROGRAM OR SHALL RETAIN AN INDEPENDENT CERTIFIED TESTING AGENCY TO PROVIDE THE LOAD TEST VERIFICATION. NOMINAL STRENGTH OF SCREWS SHALL BE AT LEAST 3.0 TIMES THE ALLOWABLE LOADS SHOWN IN THE TABLE.

ALLOWABLE LOADS FOR SCREW CONNECTIONS (LBS)

STEEL THICKNESS (MILS)	STEEL PROPERTIES		NO. 6 (0.136"Ø)		NO. 8 (0.164"Ø)		NO. 10 (0.190"Ø)		NO. 12 (0.216"Ø)	
	F _y	F _u	SHEAR	PULLOUT	SHEAR	PULLOUT	SHEAR	PULLOUT	SHEAR	PULLOUT
33	33	45	151	61	164	72	177	84	188	95
43	33	45	214	79	244	94	263	109	280	124
54	50	65	214	140	426	171	534	198	569	225
68	50	65	214	140	426	195	548	249	777	284

COLD FORMED STEEL CONNECTORS

COLD-FORMED STEEL CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY THE SIMPSON STRONG-TIE COMPANY. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE SUBSTITUTED, PROVIDED THEY HAVE ICC APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. PROVIDE NUMBER, LENGTH, TYPE, AND SIZE OF FASTENERS AS SPECIFIED BY THE MANUFACTURER. FILL ALL HOLES WITH FASTENERS AS SPECIED BY THE MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

ANCHOR RODS

ANCHOR BOLTS SHALL BE EMBEDDED 7" MINIMUM. ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 36 WITH THREADED ENDS AND WELDED HEX NUT AT END. HOLDOWN ANCHOR RODS SHALL BE ASTM F1554 GRADE 55 WITH THREADED ENDS AND WELDED HEAVY HEX NUT AT END.

WELDING

WELDING OF COLD-FORMED METAL FRAMING SHALL CONFORM TO LATEST EDITION OF THE "STRUCTURAL WELDING CODE - SHEET STEEL" AWS D1.3 AND SHALL BE PERFORMED BY WELDERS CERTIFIED BY WABO TO PRODUCE THE SPECIFIED CLASSES OF WELD. ONLY PRE-QUALIFIED WELDS (AS DEFINED BY AWS) SHALL BE USED. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES OR PROCESSES. LOW HYDROGEN ELECTRODES SHALL BE PROVIDED IN HERMETICALLY SEALED CONTAINERS; ELECTRODES THAT HAVE BEEN WET SHALL NOT BE USED. REFER TO AWS REQUIREMENTS REGARDING ALLOWABLE EXPOSURE OF LOW HYDROGEN ELECTRODES TO THE ATMOSPHERE AND FOR RE-DRYING RECOMMENDATIONS AND RESTRICTIONS. MATCHING FILLER METALS PER AWS D1.3 SHALL BE USED AND SHALL BE ADJUSTED TO ELIMINATE BURN-THROUGH IN LIGHT-GAUGE STEEL MATERIALS. WELDED AREAS SHALL BE TREATED WITH ZINC PAINT CONFORMING TO ASTM A780. FOR MATERIAL LESS THAN 0.15" THICK, DRAWINGS SHOW NOMINAL WELD SIZES. FOR SUCH MATERIAL, THE EFFECTIVE THROAT OF WELDS SHALL NOT BE LESS THAN THE THICKNESS OF THE THINNEST CONNECTED PART. PUDDLE WELDS ARE PROHIBITED. WELDING OF COLD-FORMED STEEL SHALL ONLY BE APPLIED TO MATERIAL WITH A BASE STEEL THICKNESS OF 43 MILS OR GREATER, UNO.

COLD-FORMED STEEL (CONT'D)

BOLTED CONNECTIONS

BOLTED CONNECTIONS SHALL BE IN ACCORDANCE WITH SECTION E3 OF THE AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURES". PRE-DRILLED HOLES FOR BOLTS SHALL NOT BE OVERSIZED MORE THAN 1/16" FOR BOLTS UP TO 1/2" IN DIAMETER AND 1/8" FOR LARGER BOLTS. BURNED HOLES ARE PROHIBITED.

POWER-DRIVEN OR PNEUMATIC FASTENERS

POWER-DRIVEN OR PNEUMATIC FASTENERS USED TO FASTEN COLD-FORMED STEEL MEMBERS TO STRUCTURAL STEEL OR CONCRETE SHALL BE MANUFACTURED BY THE HILTI CORPORATION, AS INDICATED IN THE TABLE BELOW. ALL FASTENERS SHALL CONFORM STRICTLY TO ICC REPORT ESR-2269 INCLUDING MINIMUM EMBEDMENT REQUIREMENTS. FASTENERS THROUGH STRUCTURAL STEEL SHALL FULLY PENETRATE THE STRUCTURAL STEEL WITH A MINIMUM PENETRATION OF 1/4" THROUGH THE LAST MATERIAL JOINED. UNDERDRIVEN PINS SHALL NOT BE RESET BUT SHALL BE REPLACED BY ANOTHER PIN INSTALLED IN ANOTHER LOCATION. FASTENERS IN CONCRETE SHALL NOT BE INSTALLED BEFORE THE SPECIFIED 28 DAY COMPRESSIVE STRENGTH OF THE CONCRETE HAS BEEN ACHIEVED. FASTENERS SHALL NOT BE INSTALLED IN MORTAR JOINTS OF HOLLOW CMU. SUBSTITUTIONS PROPOSED BY THE CONTRACTOR SHALL BE SUBMITTED WITH ICC REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. THE ESR-2269 LOAD TABLES INDICATE REQUIRED SHEAR AND PULLOUT VALUES. CONNECTED MEMBERS MAY LIMIT ACTUAL DESIGN VALUES.

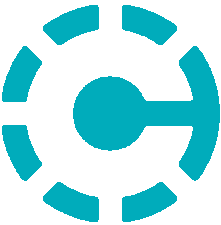
HILTI POWER ACTUATED FASTENERS AND APPLICATIONS

FASTENER	FASTENER DESCRIPTION	SHANK TYPE	SHANK Ø (IN)	HEAD Ø (IN)	MAX POINT LENGTH (IN)	MIN EFFECTIVE SHANK LENGTH (IN)	FASTENER COATING	APPLICABLE BASE MATERIAL	ESR-2269 APPLICABLE LOAD TABLES
X-U	UNIVERSAL POWER ACTUATED FASTENER	KNURLED, STRAIGHT	0.157	0.323	0.433	1.04	ASTM B633, SC1, TYPE III	STEEL	2, 7
								CONCRETE	3, 4
								CMU	6
X-U	POWER ACTUATED FASTENER	KNURLED, STEPPED	0.145	0.323	0.413	0.61	ASTM B633, SC1, TYPE III	STEEL	2
X-P	POWER ACTUATED FASTENER	SMOOTH, STRAIGHT	0.157	0.323	0.524	0.82	ASTM B633, SC1, TYPE III	CONCRETE	3
								CMU	6



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subcontract no.: 25090012-31541

FOUNDATION PLAN NOTES

1. GENERAL

- 1.1. REFERENCE FLOOR ELEVATION SHALL BE 0'-0", UNO.
- 1.2. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- 1.3. REFER TO MECHANICAL DRAWINGS FOR LOCATIONS OF ALL MECHANICAL EQUIPMENT.
- 1.4. REFER TO STRUCTURAL GENERAL NOTES FOR ADDITIONAL REQUIREMENTS.

2. FOUNDATIONS

- 2.1. EXCAVATE, BACKFILL, AND PREPARE SOILS AS REQUIRED PER STRUCTURAL GENERAL NOTES.

3. SLAB ON GRADE

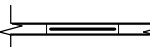
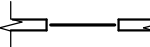
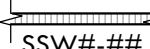
- 3.1. ELEVATION AT TOP OF SLAB SHALL BE AT REFERENCE FLOOR ELEVATION, UNO.
- 3.2. TYPICAL SLAB ON RIGID INSULATION SHALL BE 3" THICK WITH WELDED WIRE MESH AT CENTER OF SLAB.

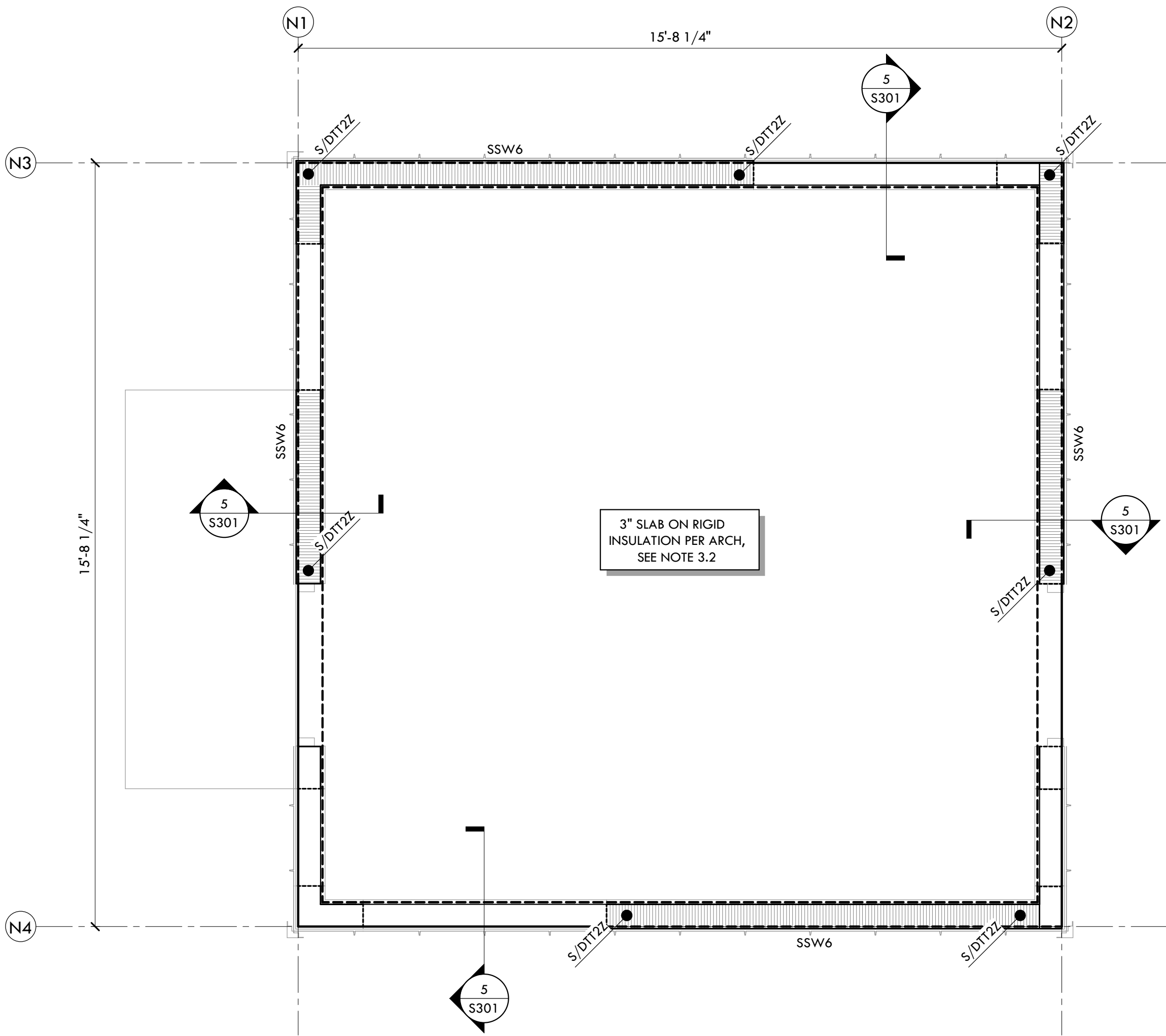
4. WALLS AND COLUMNS ABOVE

- 4.1. DEPTH OF STUD WALLS SHALL BE PER ARCHITECTURE. ALL STUD WALLS ABOVE SHALL CONSIST OF MINIMUM xxxS162-43 STUDS @ 16" OC WITH xxxT150-43. REFER TO PLAN ABOVE FOR SIZES OF HEADERS IN WALLS ABOVE. REFER TO TYPICAL DETAILS FOR MORE INFORMATION.
- 4.2. ALL EXTERIOR WALLS ABOVE SHALL BE CONSTRUCTED AS SSW6 PER COLD-FORMED STEEL SHEAR WALL SCHEDULE, UNO. BREAKS IN DOUBLE TOP TRACKS OF ALL SHEAR WALLS SHALL CONFORM TO TYPICAL DETAIL. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

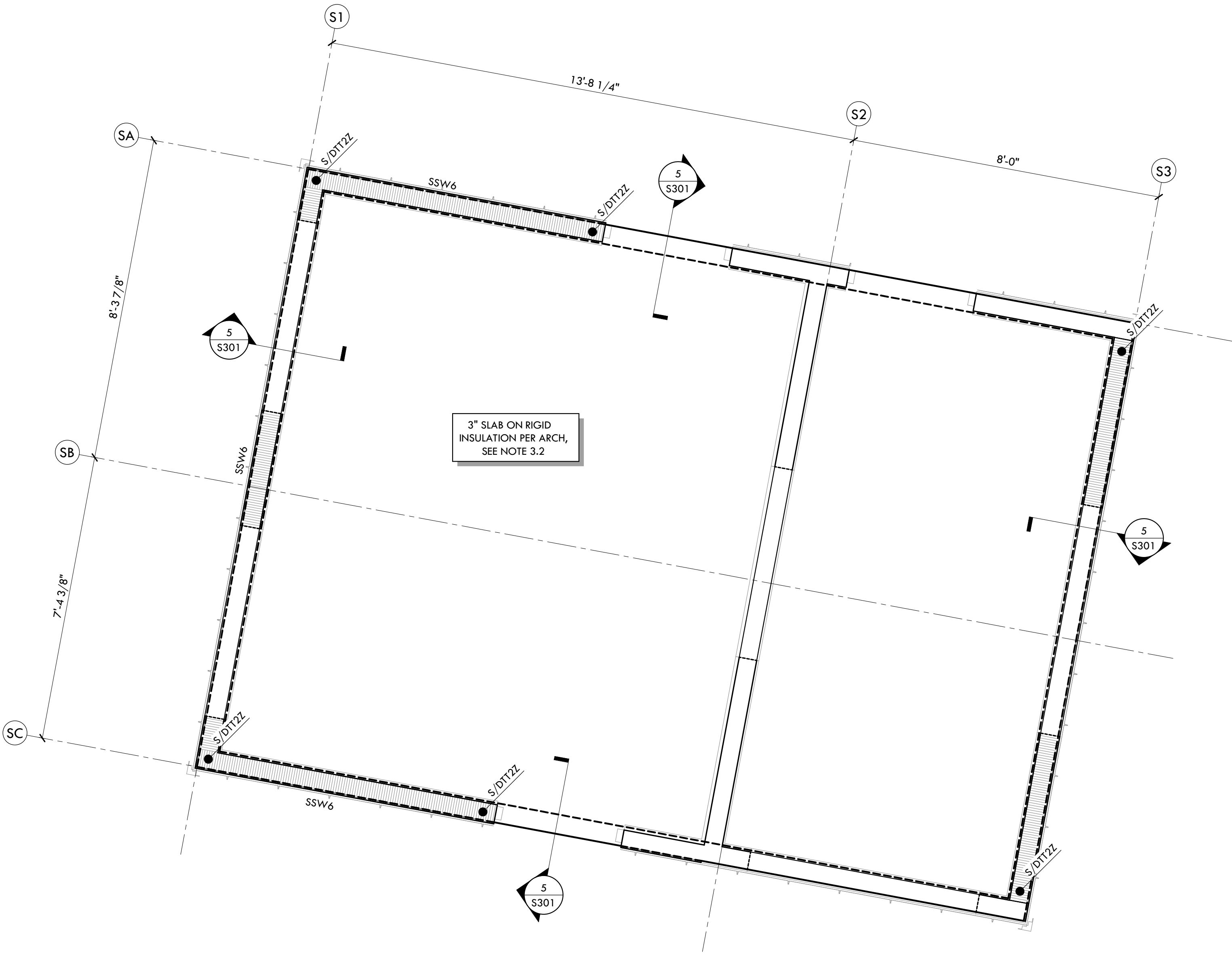
SSW# INDICATES COLD-FORMED STEEL SHEAR WALL PER SCHEDULE.

LEGEND

-  STRUCTURAL WALL THIS LEVEL WITH CONT SILL PLATE AT OPENING
-  STRUCTURAL WALL THIS LEVEL WITH BREAK IN SILL PLATE AT OPENING
-  SHEAR WALL PER S302
-  SIMPSON TENSION TIE HOLDOWN
USE (2) 600S162-43 MIN HOLDOWN STUDS. SEE S301



 **1** NE SECURITY BOOTH FOUNDATION PLAN
SCALE: 1/2" = 1'-0"



 **2** SE SECURITY BOOTH FOUNDATION PLAN
SCALE: 1/2" = 1'-0"

ROOF FRAMING PLAN NOTES

1. GENERAL

- 1.1. ELEVATION AT TOP OF SHEATHING SHALL BE 8'-6", UNO.
- 1.2. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- 1.3. REFER TO MECHANICAL DRAWINGS FOR LOCATIONS OF ALL MECHANICAL EQUIPMENT.
- 1.4. REFER TO STRUCTURAL GENERAL NOTES FOR ADDITIONAL REQUIREMENTS.

2. ROOFS

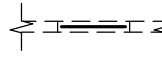
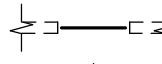
- 2.1. ROOF SHEATHING SHALL BE 15/32" APA-RATED SHEATHING (32/16), EXPOSURE 1. SHEATHING IS SUPPORTED BY COLD-FORMED METAL JOISTS NAIL SHEATHING PANELS AS FOLLOWS:

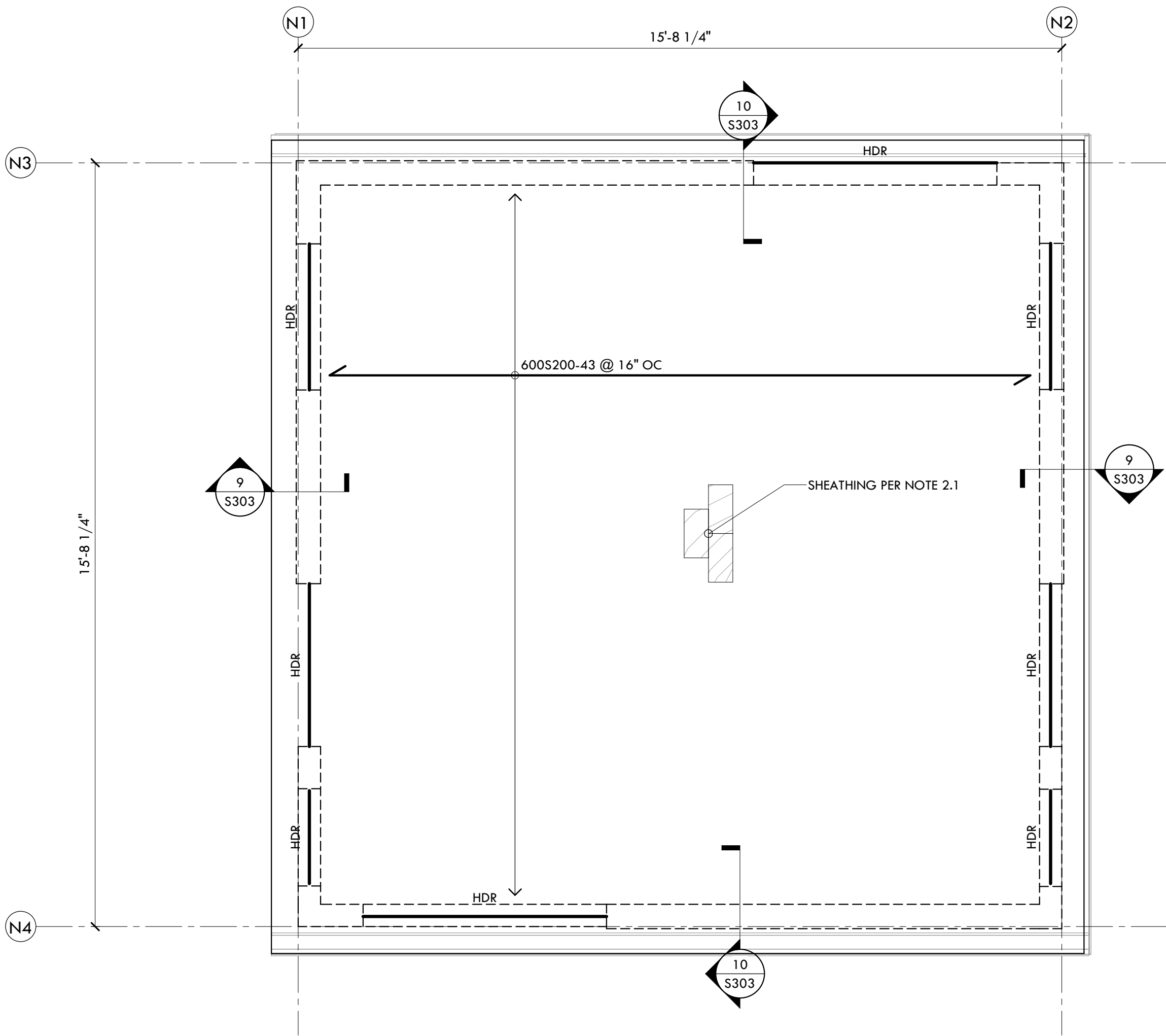
ROOF BOUNDARY (BN).....10d @ 6"
PANEL EDGES (EN).....10d @ 6"
OTHER SUPPORTS, FIELD NAILING.....10d @ 12"

NAILS SHALL BE DRIVEN FLUSH WITH THE FACE OF SHEATHING.

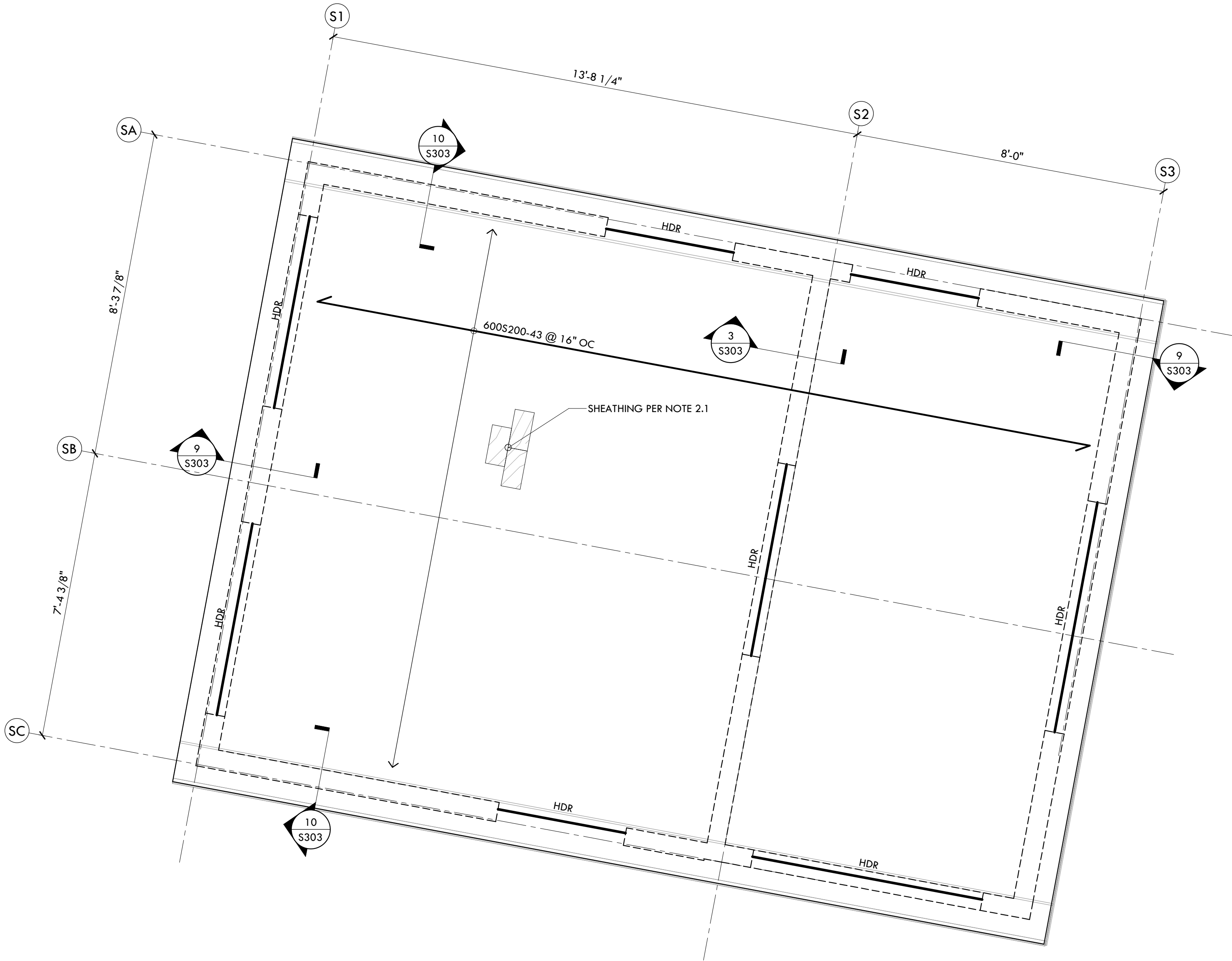
- 2.2. TYPICAL INTERIOR HEADER PER 11/S303.
- 2.3. TYPICAL RIM JOISTS SHALL BE MINIMUM 600T150-54, UNO. SEE 2/S303. FOR TYPICAL JOIST TO RIM CONNECTION. REFER TO SHEAR WALL SCHEDULE FOR ADDITIONAL REQUIREMENTS AT RIMS LOCATED ABOVE OR BELOW SHEAR WALLS.

LEGEND

-  STRUCTURAL WALL BELOW WITH HEADER (CONT WALL PLATES)
-  STRUCTURAL WALL BELOW WITH FLUSH BEAM (BREAK WALL PLATES)
-  FLOOR JOIST & EXTENT
-  BEAM PER PLAN OR HEADER PER NOTE 2.2



1 NE SECURITY BOOTH ROOF PLAN
SCALE: 1/2" = 1'-0"



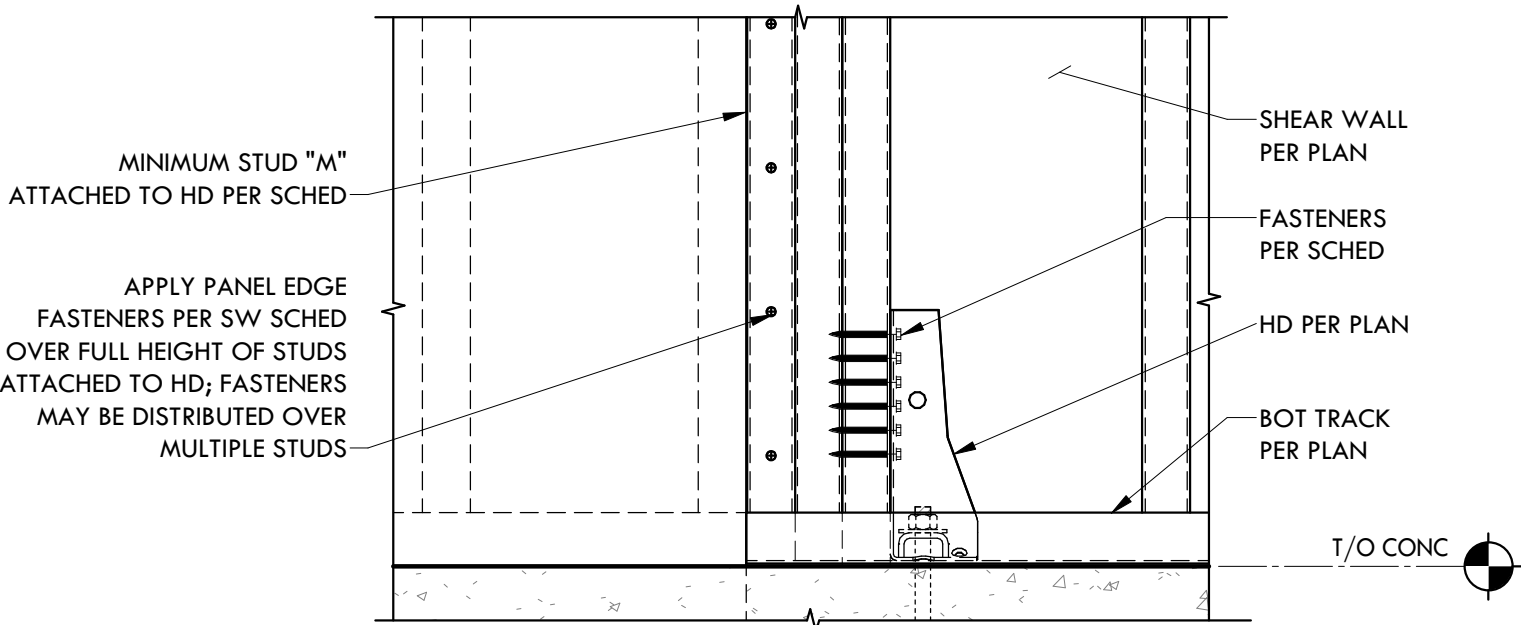
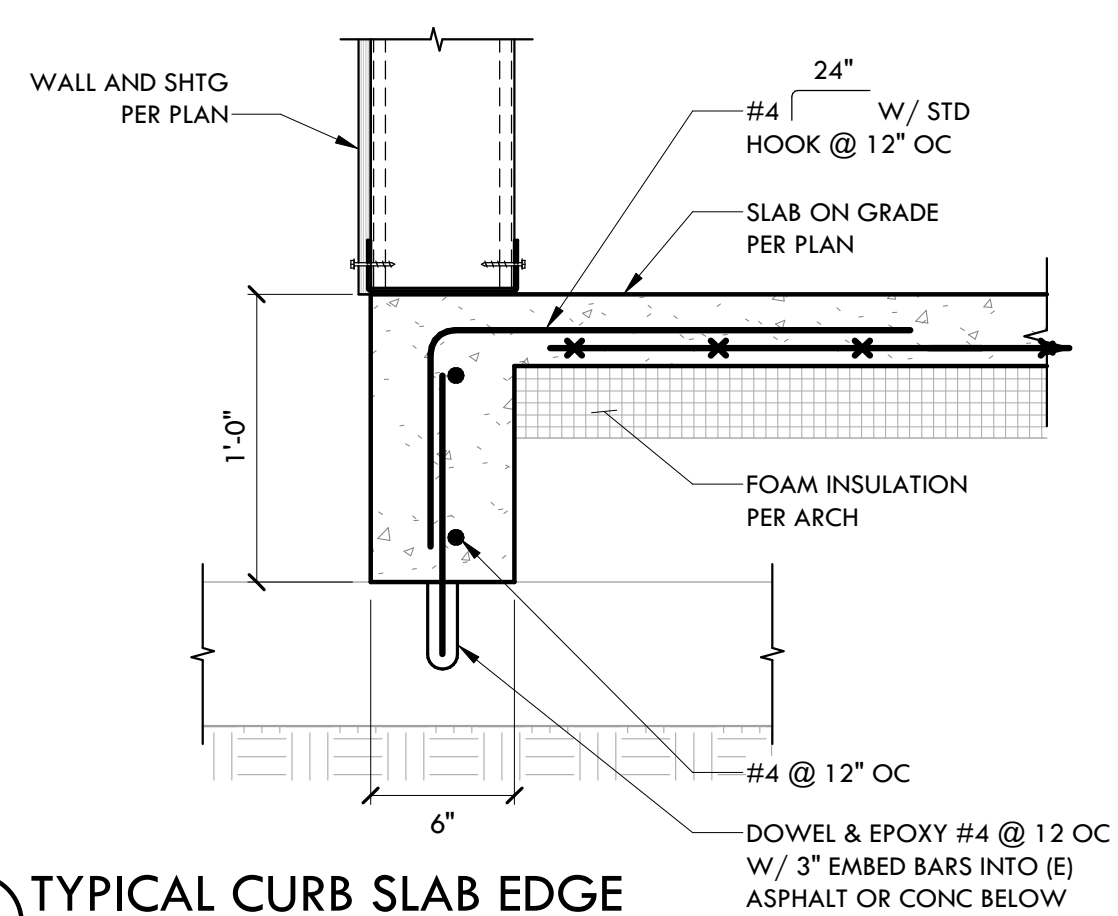
2 SE SECURITY BOOTH ROOF PLAN
SCALE: 1/2" = 1'-0"

HOLDOWN SCHEDULE

MARK	FRAMING ATTACHMENT		ANCHORAGE	
	M	FASTENERS	D	ANCHOR TYPE
S/DTTZ	(2) XXXS-43	(8) #14 SCREWS	4 1/2"	1/2"Øx4 1/2" OR HILTI KWIK BOLT TZ2 1/2"Øx4-1/2"

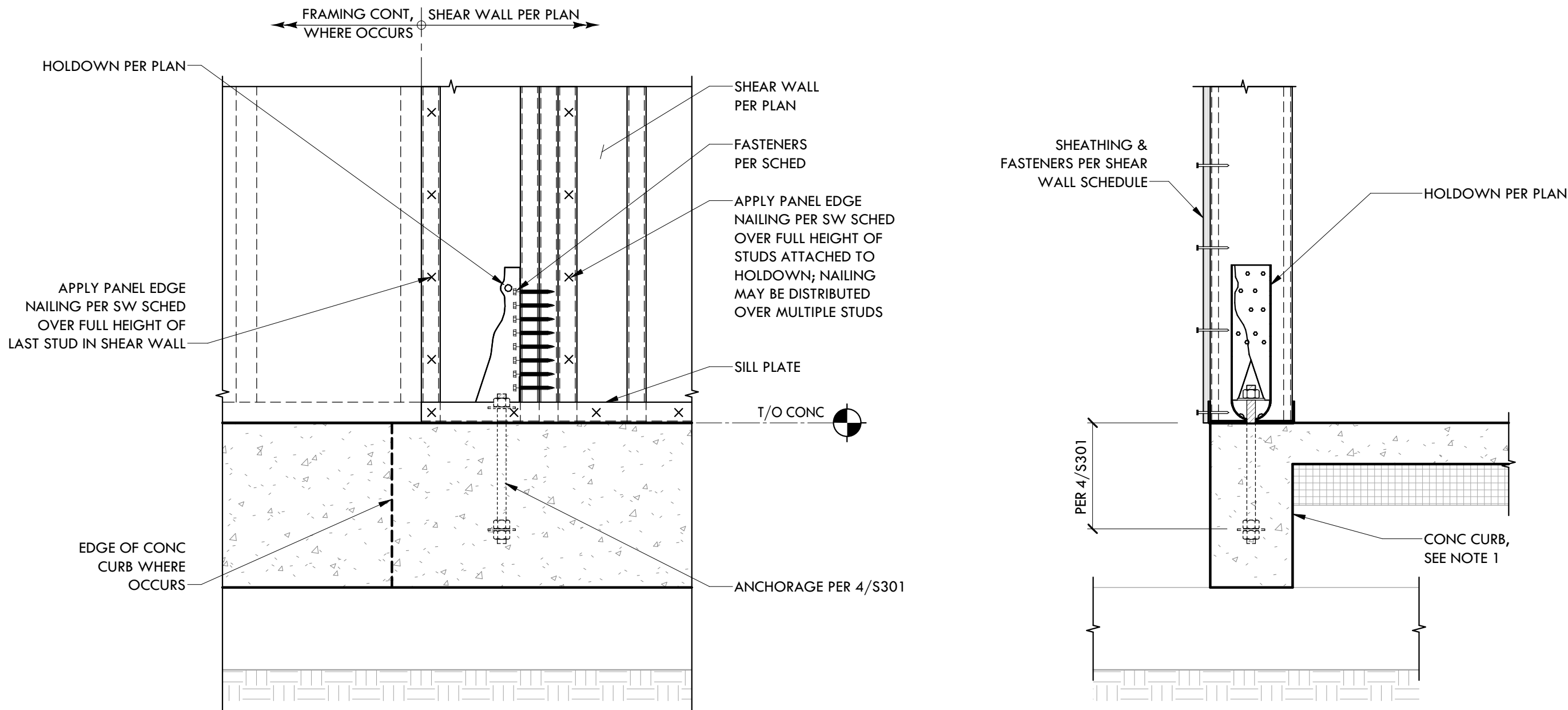
NOTES:

1. INSTALL ALL HOLDOWNS PER MANUFACTURER'S INSTRUCTIONS.
2. PLACEMENT OF ALL ANCHORS IS BASED ON CAST-IN-PLACE INSTALLATION, UNO. POST-INSTALLED ANCHORS SHALL NOT BE INSTALLED WITHOUT PRIOR APPROVAL OF ENGINEER OF RECORD.
3. "M" INDICATES MINIMUM QUANTITY AND GAUGE FRAMING MEMBER ATTACHED TO HOLDDOWN.
4. "D" INDICATES MINIMUM DISTANCE FROM CENTERLINE ANCHOR BOLT TO END OF CONCRETE WALL.
5. FOR ANCHORAGE THRU-BOLT CONNECTIONS, SEE DETAILS ON S6.16



4 TYPICAL HOLDOWN SCHEDULE

SCALE : 1 1/2" = 1'-0"



6 TYPICAL HOLDOWN - CURB

SCALE : 1 1/2" = 1'-0"

CONCRETE COVER FOR REINFORCING STEEL

REINFORCING BAR LOCATION	MINIMUM CONCRETE COVER
UNFORMED SURFACES CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
FORMED SURFACES EXPOSED TO EARTH OR WEATHER (#6 BARS AND LARGER)	2"
FORMED SURFACES EXPOSED TO EARTH OR WEATHER (#5 BARS AND SMALLER)	1 1/2"
COLUMNS AND BEAMS W/ BARS ENCLOSED IN STIRRUPS, TIES OR SPIRAL REINFORCEMENT	1 1/2"
SLABS, JOISTS AND INTERIOR FACES OF WALLS (#11 BARS AND SMALLER)	3/4" (NOTE 3)
2-HOUR AND 3-HOUR SLABS	(REFER TO PLAN NOTES)
CLEAR SPACING BETWEEN LONGITUDINAL BARS IN COLUMNS AND BOUNDARY ELEMENTS	1 1/2" OR 1.5db
CLEAR SPACING BETWEEN PARALLEL BARS IN A LAYER	1" OR db
CLEAR SPACING BETWEEN (2) OR MORE PARALLEL LAYERS	1"

NOTES:

1. WHERE A THICKNESS OF COVER REQUIRED FOR FIRE PROTECTION IS GREATER THAN THAT SPECIFIED IN THIS TABLE, THE GREATER THICKNESS SHALL BE USED.
2. WHERE TWO VALUES ARE SHOWN, THE GREATER VALUE SHALL BE USED.
3. PROVIDE 1" CLEAR TO VERTICAL REINFORCEMENT AT INTERIOR FACE OF SHOTCRETE WALLS.

8 CONCRETE COVER FOR REINFORCING STEEL

REINFORCING BAR LAP SPlice & DEVELOPMENT LENGTH TABLE

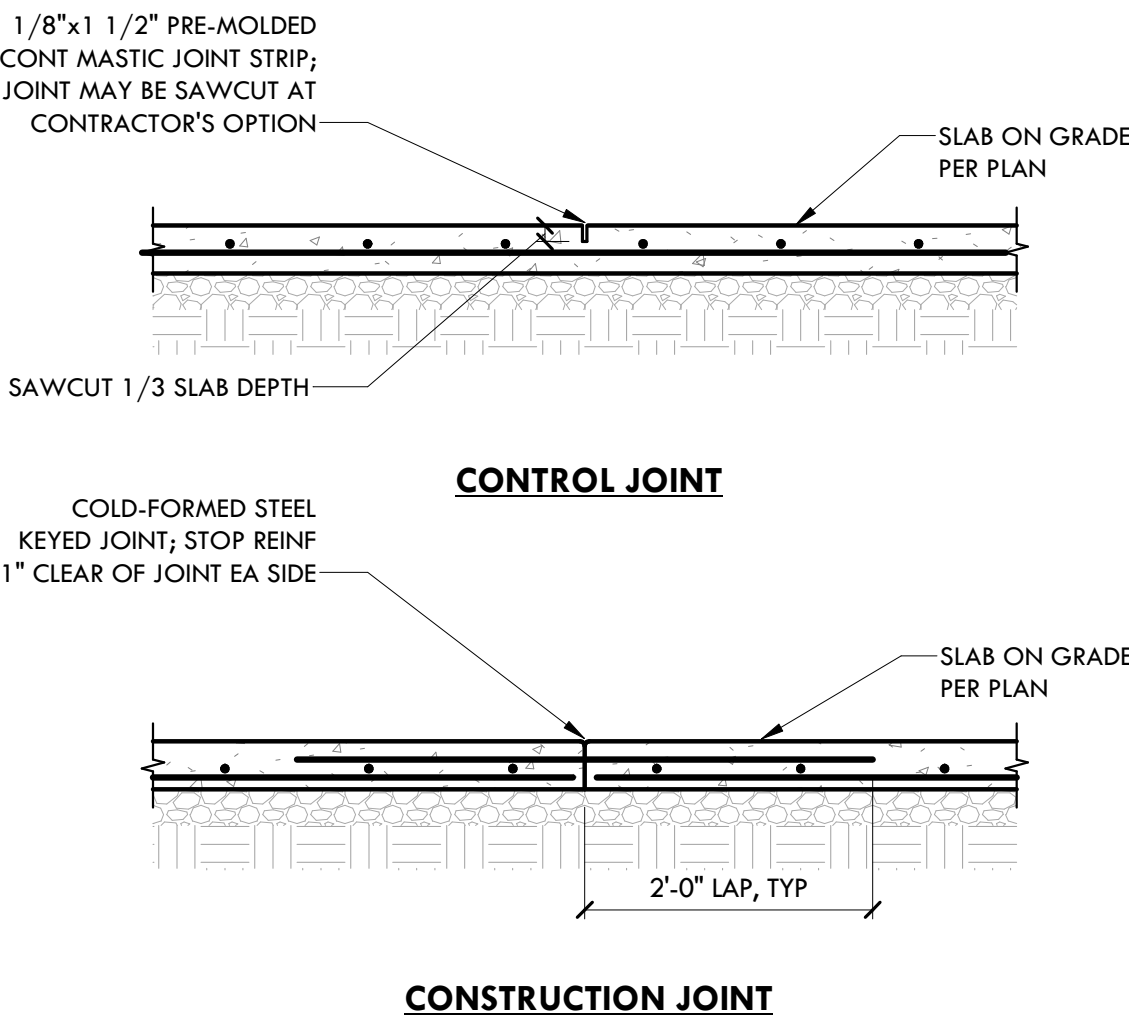
f'c = 4,000 psi

GRADE 60 REINFORCING

BAR SIZE	MIN LAP SPlice LENGTHS (Ls)		MIN STRAIGHT BAR DEVELOPMENT LENGTHS (Ld)		MIN HOOKED BAR EMBEDMENT LENGTHS (Ldh)
	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	
#3	25"	19"	19"	15"	6"
#4	32"	25"	25"	19"	7"
#5	41"	31"	31"	24"	10"
#6	49"	37"	37"	29"	13"
#7	71"	54"	54"	42"	16"
#8	81"	62"	62"	48"	19"
#9	91"	70"	70"	54"	23"
#10	102"	79"	79"	61"	27"
#11	114"	87"	87"	67"	32"

REINFORCING BAR LAP SPlice & DEVELOPMENT LENGTH TABLES

SCALE : 1 1/2" = 1'-0"

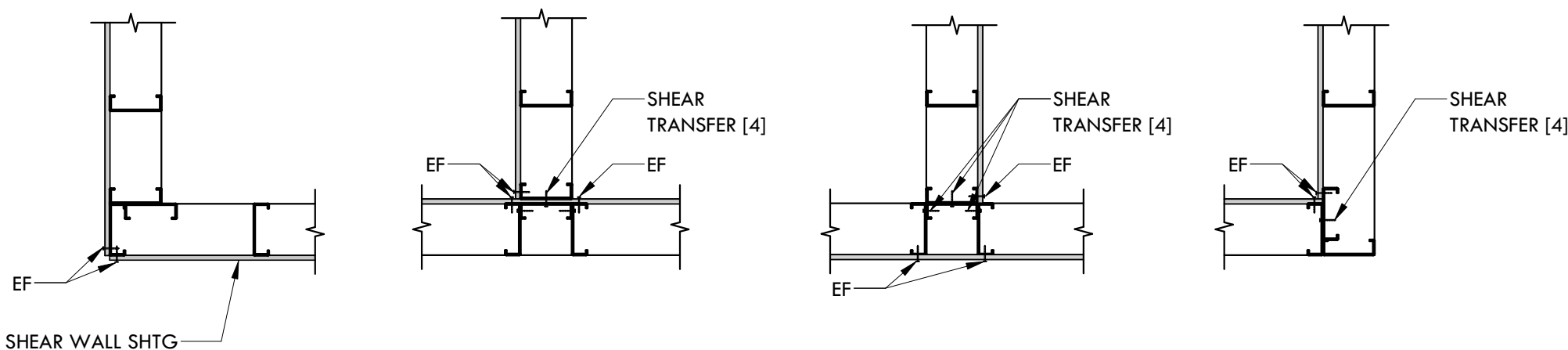


NOTES:

1. SAWED JOINTS SHALL BE MADE AS SOON AS THE JOINT CAN BE CUT WITHOUT EDGES RAVELING AND WITHIN 24 HOURS OF SLAB PLACEMENT.
2. SAWED JOINTS SHALL BE FITTED WITH SEALANT AS COORDINATED WITH THE ARCHITECT.

11 TYPICAL JOINTS IN SLAB ON GRADE

SCALE : 3/4" = 1'-0"



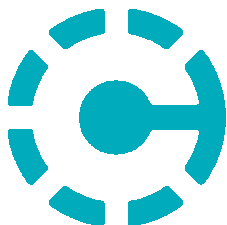
PLAN VIEW: TYPICAL SHEAR WALL INTERSECTIONS

9 TYPICAL SHEAR WALL INTERSECTIONS

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SHEAR WALL SCHEDULE (ALL VALUES BASED ON 2021 IBC)

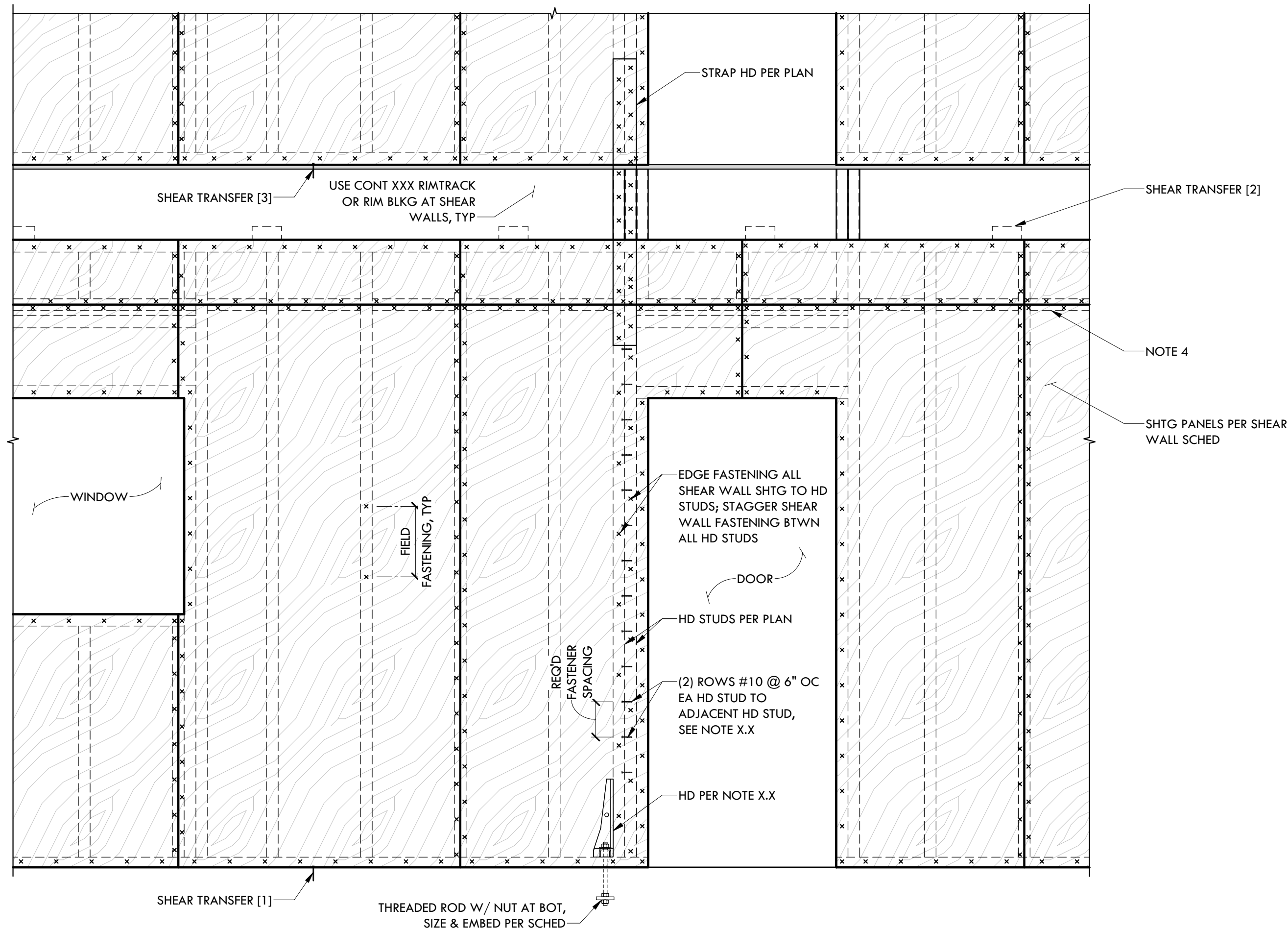
MARK	MIN STUD & TRACK GAUGE	SHEATHING (15/32" PLYWOOD)	FASTENING (5)		SHEAR TRANSFER				[E] TOP TRACK OR SILL TRACK TO STEEL >1/4" THICK (STAGGERED)	LATERAL CAPACITY (ASD) (8)
			EDGE (EF)	FIELD	[A] SILL TRACK TO CONCRETE	[B] RIM TO TOP TRACK	[C] SILL TRACK TO RIM/JOIST/BLKG	[D] SHEAR WALL INTERSECTIONS		
SSW6	43 MIL	(1) SIDE	#8 @ 6" OC	#8 @ 12" OC	5/8"Ø TITEN HD @ 32" OC	#8 @ 6" OC	#8 @ 6" OC	#8 @ 8" OC	0.157"Ø PAF(S) @ 16" OC (STAGGERED)	356 PLF
SSW4	43 MIL	(1) SIDE	#8 @ 4" OC	#8 @ 12" OC	5/8"Ø TITEN HD @ 32" OC	#8 @ 5" OC	#8 @ 4" OC	#8 @ 4" OC	0.157"Ø PAF(S) @ 11" OC (STAGGERED)	532 PLF

SHEAR WALL SCHEDULE NOTES:

1. PROVIDE FRAMING AT SHEAR WALLS AS INDICATED.
2. STAGGER PANEL JOINT EACH SIDE OF WALL WHERE SHEATHING IS REQUIRED BOTH SIDES OF WALL.
3. STUDS SHALL HAVE MINIMUM 1 5/8" FLANGE, 3/8" LIP, AND 3 1/2" WEB DEPTH. TRACK SHALL HAVE MINIMUM 1 1/4" FLANGE AND 3 1/2" WEB DEPTH. 43 MIL STUDS AND TRACK SHALL BE ASTM A1003 STRUCTURAL GRADE 33 TYPE H STEEL. 54 MIL AND GREATER STUDS AND TRACK SHALL BE ASTM A1003 STRUCTURAL GRADE 50 TYPE H STEEL, UNLESS NOTED OTHERWISE.
4. BLOCK ALL PANEL EDGES.
5. SHEATHING FASTENERS SHALL BE #8 (0.165"Ø) x 1 1/4" LONG (MIN) FLAT-HEAD SELF-DRILLING TAPPING SCREWS WITH A MINIMUM HEAD DIAMETER OF 0.285". DRIVE ALL SCREWS FLUSH WITH FACE OF SHEATHING. DO NOT OVERDRIVE.
6. CONNECT SHEATHING & STUDS AT SHEAR WALL INTERSECTIONS AS INDICATED.
7. A MINIMUM OF (2) FULL-HEIGHT STUDS ARE REQUIRED AT ENDS OF WALL AND AT ALL STRAP/HOLDOWN LOCATIONS. CONNECT STUDS TOGETHER AS SHOWN PER ELEVATION.
8. ALLOWABLE LATERAL CAPACITIES ARE FOR TYPE I SHEAR WALLS WITH MAXIMUM ASPECT RATIO OF 2:1. VALUES SHALL BE ADJUSTED IN ACCORDANCE WITH THE AISI STANDARD FOR SEISMIC DESIGN OF COLD-FORMED STEEL STRUCTURAL SYSTEMS WHERE THE ASPECT RATIO EXCEEDS 2:1.

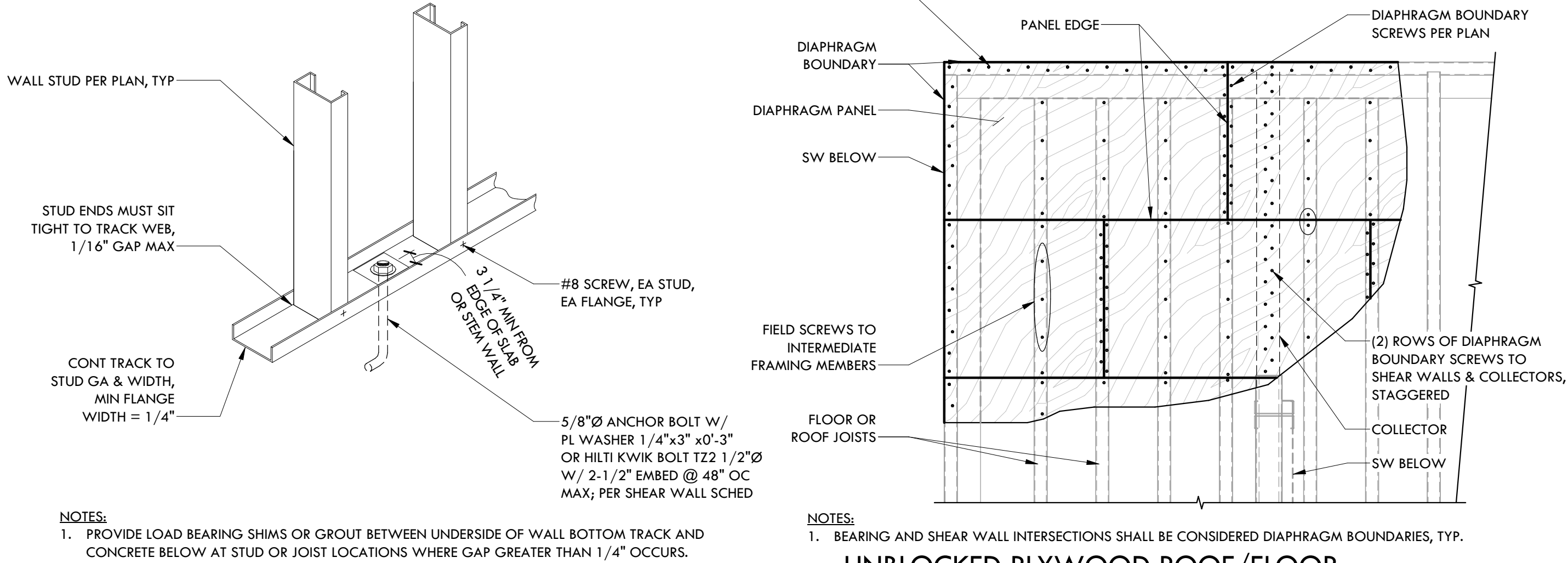
1 SHEAR WALL SCHEDULE NOTES

SCALE : 1 1/2" = 1'-0"



9 TYPICAL SHEAR WALL FRAMING

SCALE : 3/4" = 1'-0"

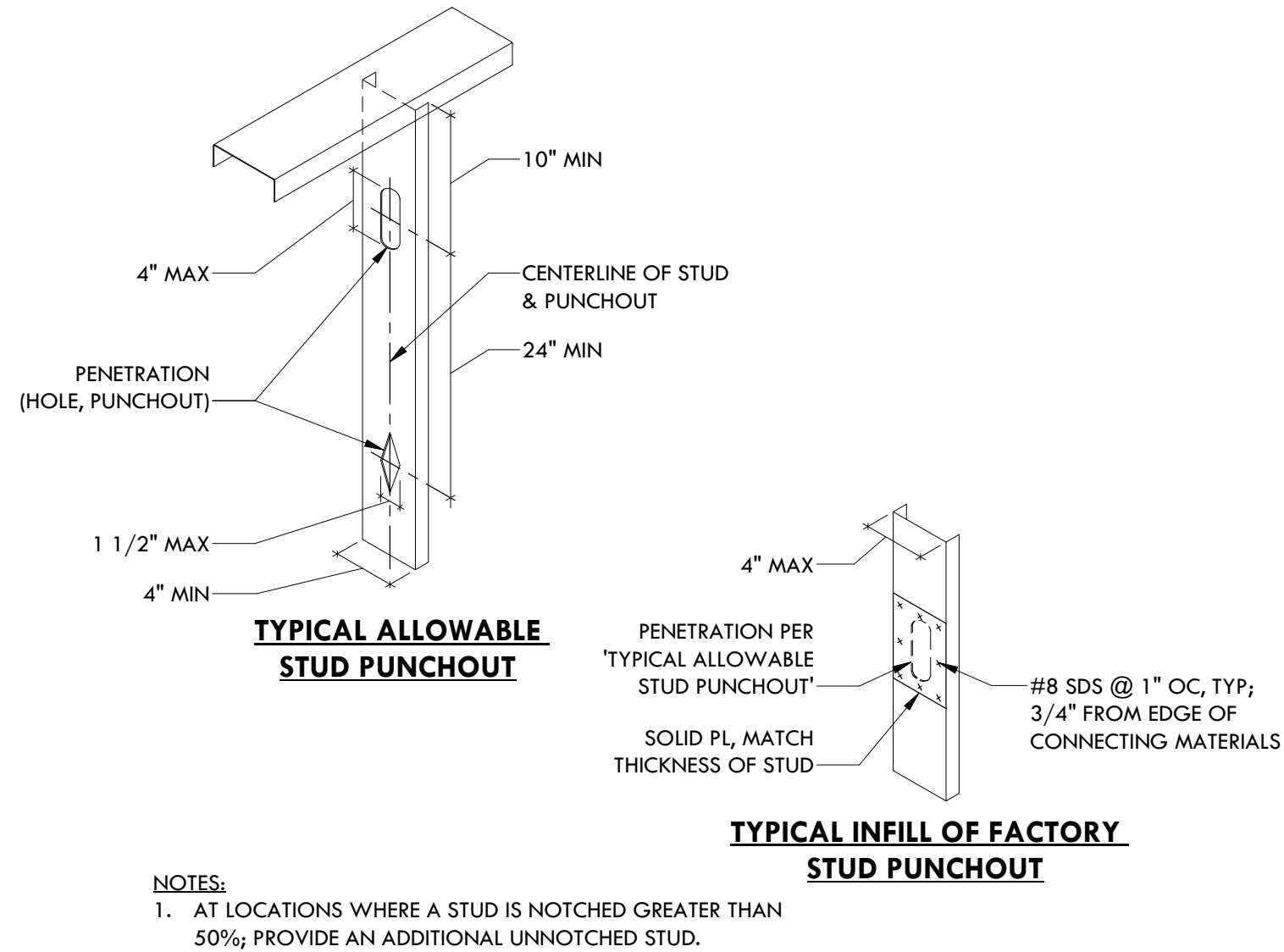


3 TYPICAL BASE TRACK CONNECTION

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4 SHEATHING LAYOUT

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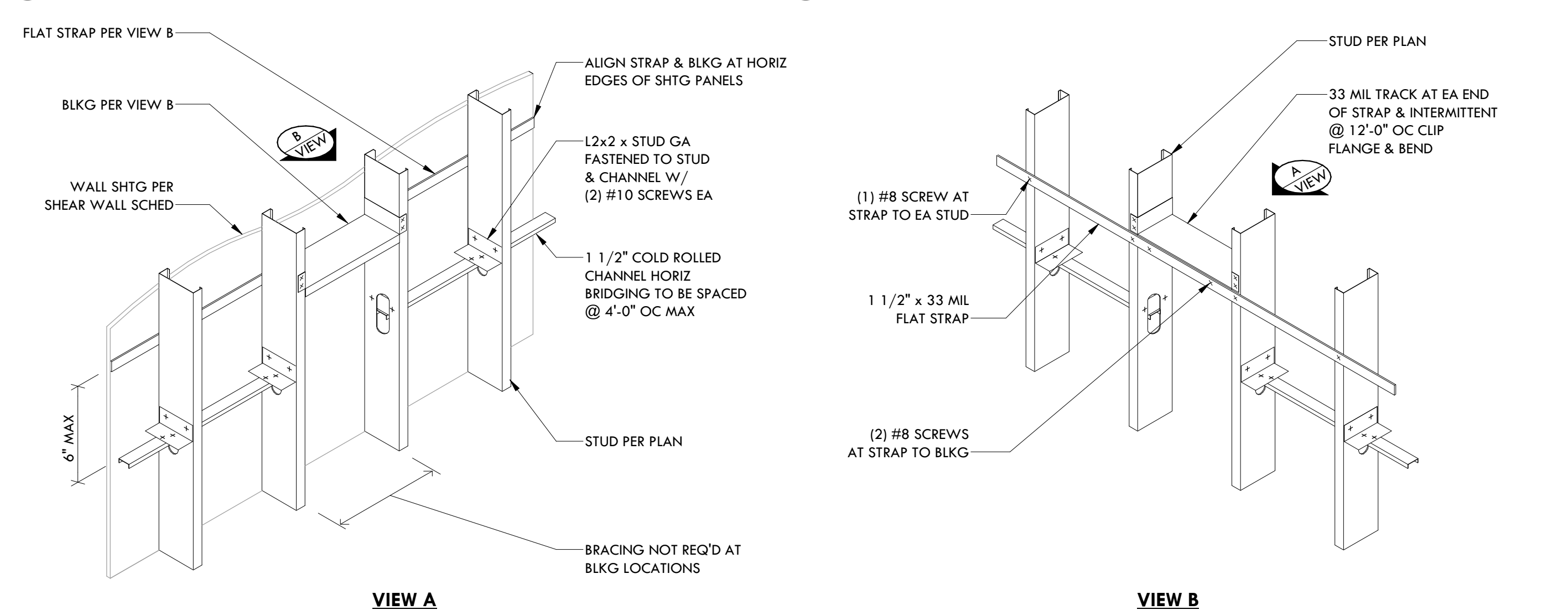


7 TYPICAL STUD PUNCHOUT

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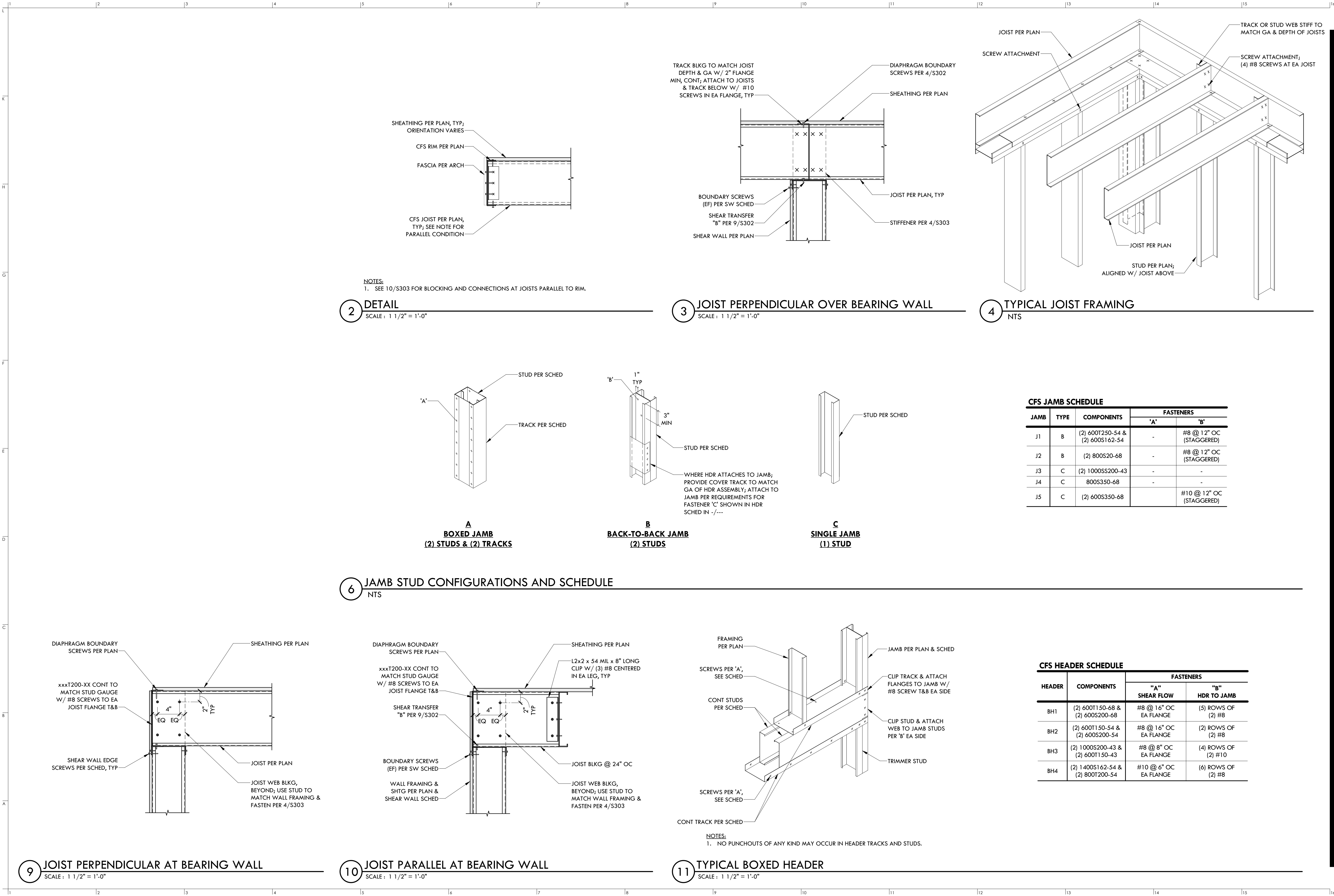
8 JOIST BRIDGING DETAIL

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11 TYPICAL BRACING AND BLOCKING

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