

**GEOTECHNICAL DATA REPORT
TAYLOR CREEK RESTORATION PROJECT
DEAD HORSE CANYON
SEATTLE, WASHINGTON**

**Work Authorization No.: Project C399315
October 2022**



**Seattle
Public
Utilities**

Geotechnical Engineering

**707 South Plummer Street
Seattle, Washington 98134**

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GEOTECHNICAL DATA REPORT TAYLOR CREEK RESTORATION PROJECT DEAD HORSE CANYON SEATTLE, WASHINGTON

1.0 INTRODUCTION

This geotechnical data report (GDR) presents the results of our geotechnical investigation for a temporary access road that Seattle Public Utilities (SPU) is considering constructing in Dead Horse Canyon as part of the Taylor Creek Restoration Project. We will provide geotechnical engineering recommendations in a separate report. The project limits addressed in this report are shown on Figures 1 through 3.

We have organized this report into several sections. The first three sections describe the purpose and scope of our work and our understanding of the project. The fourth section presents the site subsurface conditions. Figures illustrating site and project features are presented at the end of the text. Field data are presented in Appendix A, geotechnical laboratory test results are presented in Appendix B, and logs of relevant historical explorations are provided in Appendix C.

2.0 PURPOSE AND SCOPE OF WORK

The purpose of our work is to provide the SPU Project Team with subsurface information to support design of the proposed project features.

Our scope of work for this project included:

- Reviewing readily available geotechnical/geologic information for the project site and vicinity;
- Completing a geotechnical exploration program including 16 soil borings;
- Completing geotechnical laboratory testing; and,
- Preparing this geotechnical data report summarizing our investigation.

3.0 SITE AND PROJECT UNDERSTANDING

3.1 SURFACE CONDITIONS

The proposed work is in Dead Horse Canyon, an approximately 35-acre natural area in southeast Seattle which is generally bounded by Holyoke Way S to the north, Rustic Road S to the east, 69th Place S and 71st Place S to the south and 68th Place S and Waters Avenue S to the west. Dead Horse Canyon, which is part of Lakeridge Park and is under the jurisdiction of Seattle Parks and Recreation (Parks), consists of an approximately 2,600-foot-long, 500-foot-wide, and 50- to 130-foot-deep north-south oriented ravine that is densely vegetated with mature conifers and deciduous trees and low growing brush. Privately owned parcels, which are generally occupied by single family residences, are

located along the top of the ravine, and abut the east, south and west boundaries of the Lakeridge Park parcel.

The ground surface along the top of the ravine varies from elevation 270 feet at the south to elevation 215 feet at the north. Taylor Creek flows from south to north along the bottom of the ravine dropping from approximately elevation 230 feet at the southern extent of the east and west forks of the creek to 185 feet at the confluence of the east and west forks and elevation 75 feet at Holyoke Way S. After passing under Holyoke Way S, Taylor Creek continues to flow north for approximately 0.2 miles to Lake Washington. In general, the east slope of the ravine has a constant inclination from top to bottom, while the west slope of the ravine tends to be steep near the top of the slope, flatter near mid slope and becomes steeper again near the bottom of the slope. All elevations in this report are referenced to the NAVD88 datum, unless noted otherwise.

An approximately 4- to 6-foot-wide aggregate surfaced trail begins at Holyoke Way S and follows the west slope of the ravine south for approximately 1,700 feet where it splits with one trail continuing along the west slope of the ravine and the other trail crossing Taylor Creek to the east slope of the ravine. The two trails continue south and reconnect near the confluence of the east and west forks of Taylor Creek to form an approximately 1,100-foot-long loop. In general, the trail is approximately 20 to 40 feet above Taylor Creek. A 10-inch diameter sanitary sewer line that is owned by SPU is buried beneath the trail on the west slope of the ravine between Holyoke Way S and the trail junction at 1,700 feet. The sewer then crosses Taylor Creek and continues south beneath the trail on the east slope of the ravine.

3.2 PROJECT UNDERSTANDING

We understand that the proposed project scope within Dead Horse Canyon includes building wood structures in Taylor Creek to stabilize the channel and capture sediment and installing surface mounted stormwater drainpipes (tightlines) along the east slope of the ravine to reduce erosion. SPU has developed two potential options for providing access into Dead Horse Canyon to install the wood structures in Taylor Creek. One option includes an approximately 2,460-foot-long temporary access road that generally follows the existing trail alignment along the west slope of the ravine between Holyoke Way S and the confluence of the east and west forks of Taylor Creek then follows the trail alignment along the east slope of the ravine before terminating on the south side of the east fork. This option also includes a 240-foot-long temporary spur road that follows a portion of the trail alignment along the east slope of the ravine. The other option includes a temporary access road that follows the same alignment as the 2,460-foot-long temporary access road, but terminates approximately 1,250 feet south of Holyoke Way S. The proposed temporary access roads would have a 12-foot-wide aggregate surface and the proposed temporary spur road would have a 10-foot-wide aggregate surface. The

proposed alignments of the temporary access and spur roads are shown on Figures 1 through 3.

3.3 HISTORICAL LANDSLIDES

Based on our review of the Known Slide Environmental Critical Area (ECA) layer in the City of Seattle GIS online geodatabase, which includes information for landslides that have been reported in Seattle between 1890 and 2019, one historical landslide is mapped along the top of the west slope of the ravine upslope of the proposed alignment of both temporary access road options. Three historical landslides are mapped near the top of the ravine at locations that are not upslope of the proposed alignment of the temporary access and spur roads. Based on our review of other landslide related ECA layers in the City of Seattle GIS online geodatabase, most of the east and west slopes of the ravine are mapped as steep slope and/or potential slide ECAs. Steep slope ECAs are defined as slopes with an incline of 40 percent or more and a vertical elevation change of at least ten feet. Potential slide ECAs are defined as areas with indications of past landslide activity, such as landslide headscarps and sidescarps, hummocky terrain, areas with geologic conditions that can promote earth movement, and areas with signs of potential landsliding, such as springs, groundwater seepage, and bowed or backtilted trees. The location of landslide related ECAs that are mapped in the vicinity of Dead Horse Canyon are shown on Figure 1.

In 2005, the United States Geological Survey (USGS) (Schulz, 2005) used LIDAR imagery to create a landslide inventory map for Seattle. Based on landforms visible in the LIDAR imagery, possible headscarps, landslide deposits, and denuded slopes were identified. The historical landslides identified using LIDAR were compared to data from a landslide inventory based on historical records (Shannon and Wilson, Inc., 2003) to develop a relationship between landslide potential and the identified landslide landforms plus an additional landform representing the areas within Seattle where no landslide-related landforms were observed.

Schulz proposes that because future landslide activity in Seattle is expected to be similar in type and location to recent activity, projections can be made about the relative likelihood of landslides within a given area. The results of the study indicate that compared to areas where no landslide-related landforms have been observed, the likelihood of future landslides is 244, 86, and 47 times greater within areas mapped as headscarps, landslide deposits, and denuded slopes, respectively. As shown on Figure 2, the east and west slopes of the ravine are generally mapped within the three landslide-related landforms identified by Schulz, indicating that landslides are 47 to 244 times more likely to occur along these slopes when compared to slopes in Seattle where no landslide related landforms have been observed.

4.0 SUBSURFACE CONDITIONS

We based our interpretation of subsurface conditions at the project site on published geologic maps, information obtained from new and historical subsurface explorations, and laboratory tests on select soil samples. Figure 3 shows the mapped geology and the location of the new and historical explorations. We prepared a generalized subsurface profile along the proposed centerline of the 2,460-foot-long temporary access road based on our interpretation of the subsurface stratigraphy. The profile is shown on Figure 4. The proposed alignment of the temporary access road is shown on Figures 1 through 3.

The information provided in this report is based on subsurface soil conditions interpreted from these explorations. The nature and extent of variations between the explorations and current conditions may not become evident until additional explorations are completed, or construction begins. If variations are encountered, it will be necessary to reevaluate the information provided in this report.

4.1 GENERAL GEOLOGY

The general geologic condition of the Puget Sound region is a result of glacial and non-glacial activity that occurred over the course of millions of years. The most recent glacial activity in the Puget Sound area was the Vashon stade of the Fraser glaciation that ended about 10,000 years ago. Between each glaciation, non-glacial erosion and deposition shaped the region.

Review of the geologic map of Seattle (Troost, et al, 2005) indicates that the ravine is underlain by a sequence of glacial and non-glacial deposits with the youngest deposits along the top of the ravine and the oldest deposits near the bottom of the ravine.

The geologic map indicates the ravine is underlain by the following geologic units generally listed in order of highest to lowest elevation: Vashon subglacial till, Vashon advance outwash, Lawton Clay, pre-Olympia glacial diamict, pre-Olympia glacial fine-grained, pre-Olympia non-glacial, pre-Olympia glacial coarse grained, and pre-Olympia glacial till. These geologic units have been overridden by glacial ice and as a result, are over consolidated and generally dense to very dense or very stiff to hard.

Vashon subglacial till, pre-Olympia glacial diamict and pre-Olympia glacial till generally consist of a poorly sorted mix of silt, sand, and gravel that was deposited beneath a glacier. Lawton Clay and pre-Olympia glacial fine-grained deposits generally consist of laminated to massive silt and clayey silt that was deposited in lowland proglacial lakes. Vashon advance outwash and pre-Olympia glacial coarse-grained deposits generally consist of sand and gravel that was deposited in front of an advancing glacier or behind retreating glacier. pre-Olympia non-glacial deposits can consist of silt, clay, sand, or gravel of non-glacial origin that has been overridden by glacial ice.

The geologic map also indicates that a layer of landslide or mass wastage deposits could be present at the ground surface along all slopes within the ravine. Landslide and mass

wastage deposits consist of soil located on steep slopes that has been transported downslope by gravity. These deposits have not been overridden by glacial ice. The density and composition of landslide and mass wastage deposits are highly variable.

The subsurface conditions encountered in our explorations and indicated by the historical exploration logs generally agree with the mapped geology and the geologic history at the site.

4.2 SOIL CONDITIONS

We grouped the soil encountered in the new and historical explorations into the following soil units based on physical and behavioral characteristics:

4.2.1 Soil Unit 1

Soil unit 1 consists of a variable mixture of very soft to stiff silt and clay with variable amounts of sand and gravel. Soil Unit 1 includes fine-grained soil that was deposited fluvially by Taylor Creek (alluvium) and fine-grained native source material that has been reworked by human activity (fill) or natural processes (mass wastage and landslide deposits).

4.2.2 Soil Unit 2

Soil Unit 2 consists of a variable mixture of loose to medium dense silty sand with variable amounts of gravel. Soil Unit 2 includes coarse-grained soil that was deposited fluvially by Taylor Creek (alluvium) and coarse-grained native source material that has been reworked by human activity (fill) or natural processes (mass wastage and landslide deposits).

4.2.3 Soil Unit 3

Soil Unit 3 consists of dense to very dense silty sand with variable amounts of gravel and cobbles. It includes glacially overridden Vashon glacial till, pre-Olympia glacial till and pre-Olympia glacial diamict deposits.

4.2.4 Soil Unit 4

Soil Unit 4 consists of dense to very dense sand with silt and gravel to silty sand with gravel. It includes glacially overridden Vashon advance outwash and pre-Olympia glacial coarse-grained deposits.

4.2.5 Soil Unit 5

Soil Unit 5 consists of stiff to hard silt and clay. It includes glacially overridden Lawton Clay and pre-Olympia glacial fine-grained deposits.

4.2.6 Soil Unit 6

Soil Unit 6 consists of medium stiff to stiff silt and clay or medium dense to dense sand to silty sand. It includes glacially overridden pre-Olympia non-glacial deposits.

4.3 GROUNDWATER CONDITIONS

During drilling, we encountered groundwater between approximately 7 and 20 feet below ground surface in borings B-302, B-304, B-305, B-311, B-312, B-313 and B-314. In general, we interpret the groundwater we encountered as being perched on top of low permeability soil including Soil Units 3, 5 and 6. We anticipate that groundwater level fluctuates throughout the year and is generally highest during the late winter and spring seasons and lowest during the late summer and early fall seasons.

5.0 LIMITATIONS AND ADDITIONAL SERVICES

This report was prepared in accordance with generally accepted professional principles and practices in the field of geotechnical engineering at the time the report was prepared. This report does not include geotechnical engineering recommendations or conclusions based on the data obtained.

This geotechnical data report is intended to provide information to support preliminary engineering activities for this project. The data presented are from observed conditions during subsurface explorations and through laboratory testing of subsurface materials at the specific locations and depths indicated, using the means and methods described in this report. Subsurface conditions, as derived from exploration logs and test results, presented in this report should not be construed as a warranty of the subsurface conditions.



We appreciate the opportunity to be of service.

Sincerely,

SPU GEOTECHNICAL ENGINEERING

Handwritten signature of Aaron Clark in blue ink.

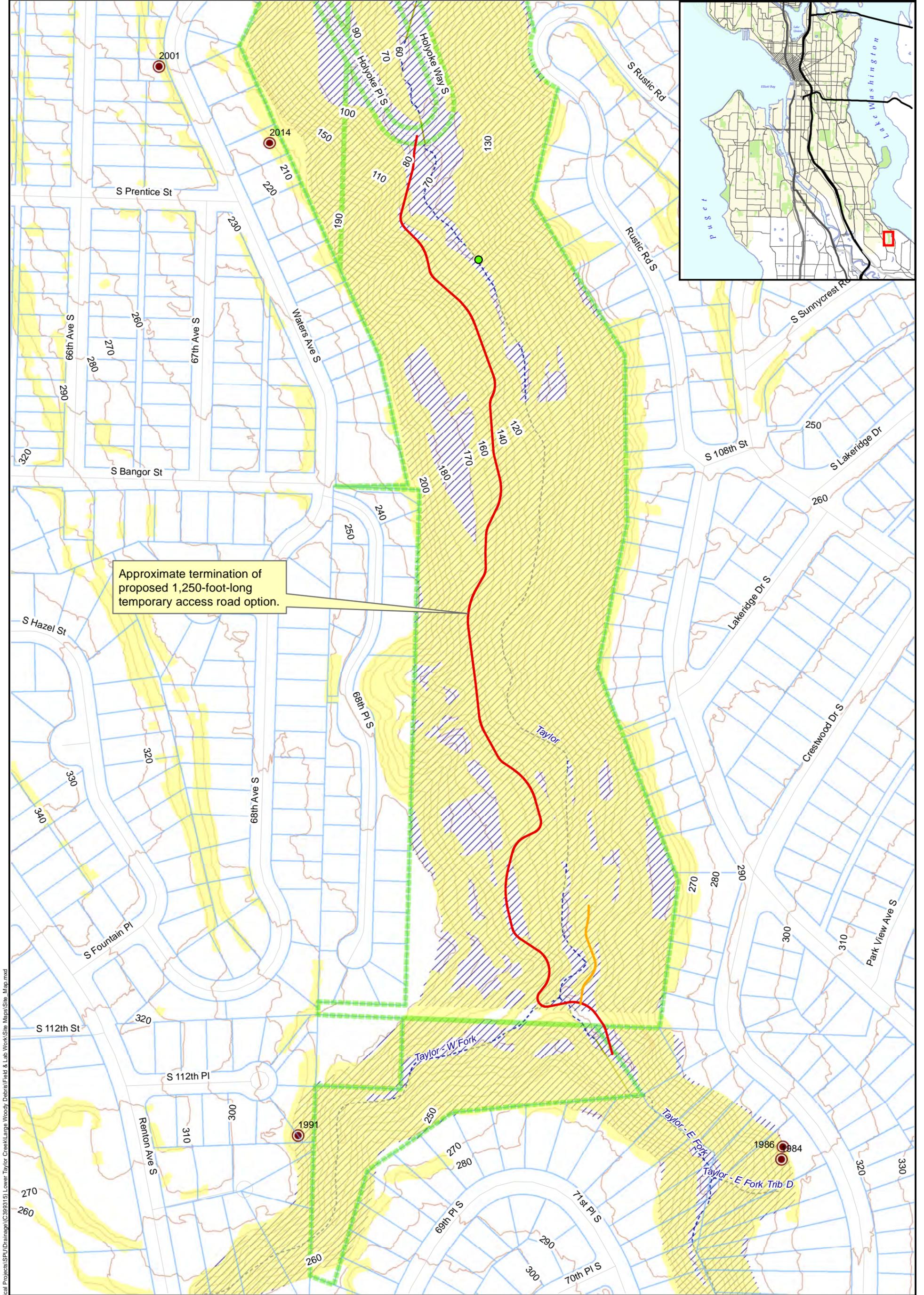
Aaron Clark, L.G.
Senior Geologist

Handwritten signature of Megan Higgins in blue ink.

Megan Higgins, P.E.
Senior Geotechnical Engineer

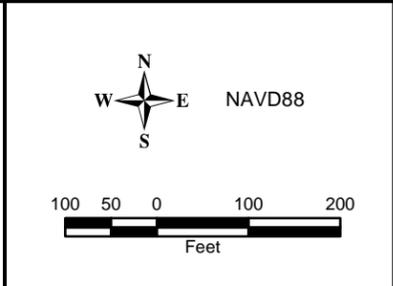
6.0 REFERENCES

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- Gallaster, R.W., and Laprade, W.T., 1991. Geology of Seattle, Washington, United States of America, in: Bulletin of the Association of Engineering Geologists, Vol. 28, No. 3, 1991, pp. 235-302.
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- Washington Geologic Information Portal. Washington Department of Natural Resources, Online Geodatabase: <https://geologyportal.dnr.wa.gov/>. Accessed April 2021.



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- LEGEND**
- Proposed Temporary Access Road Alignment
 - Proposed Temporary Spur Road Alignment
 - ▨ Lakeridge Park
 - Parcels
 - Known Slide Location
 - Steep Slope (40%)
 - Potential Slide Area
 - Topographic Contour - 10 Ft

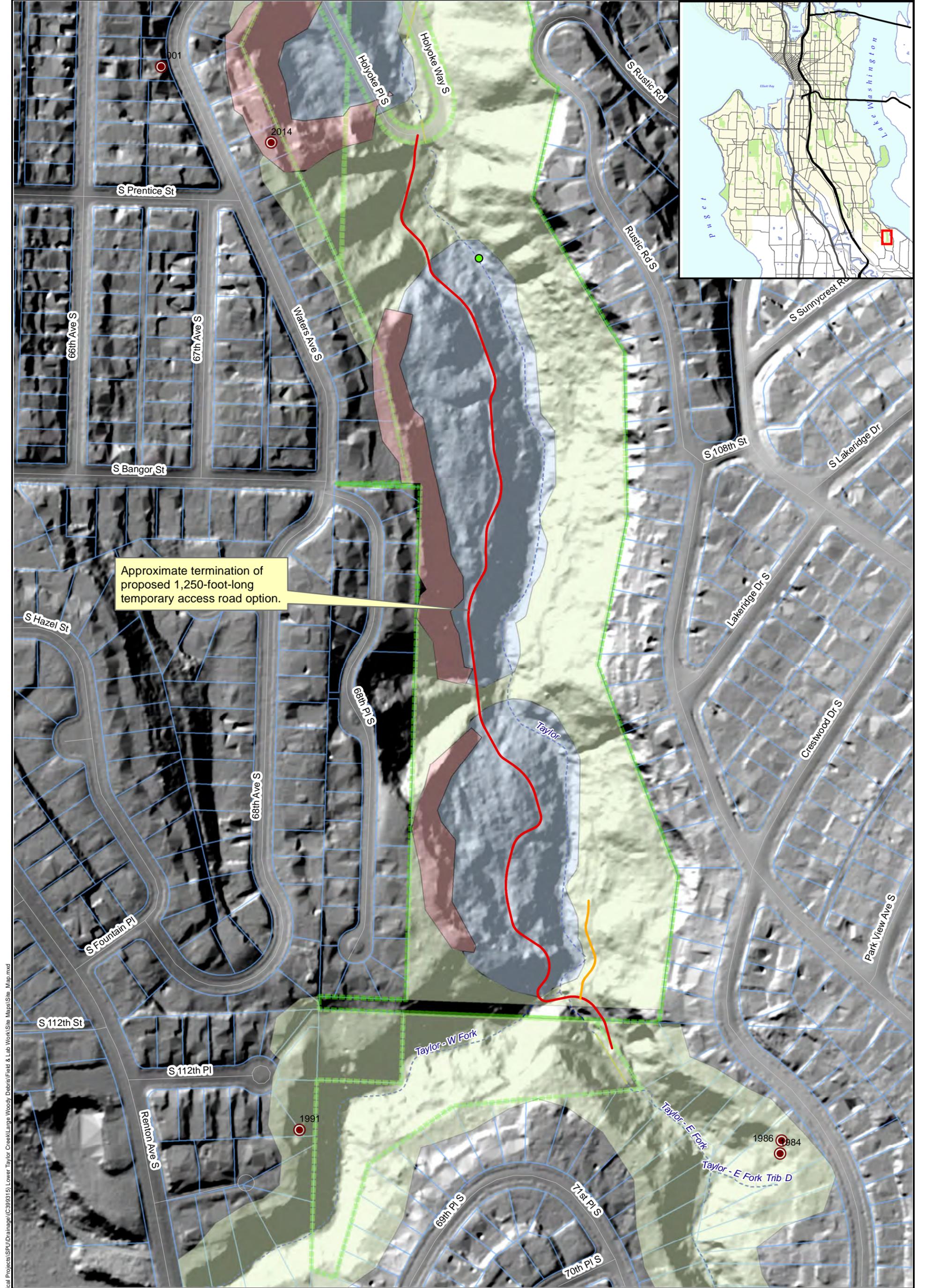


Taylor Creek Restoration Project
Dead Horse Canyon
Seattle, Washington

Site and ECA Map



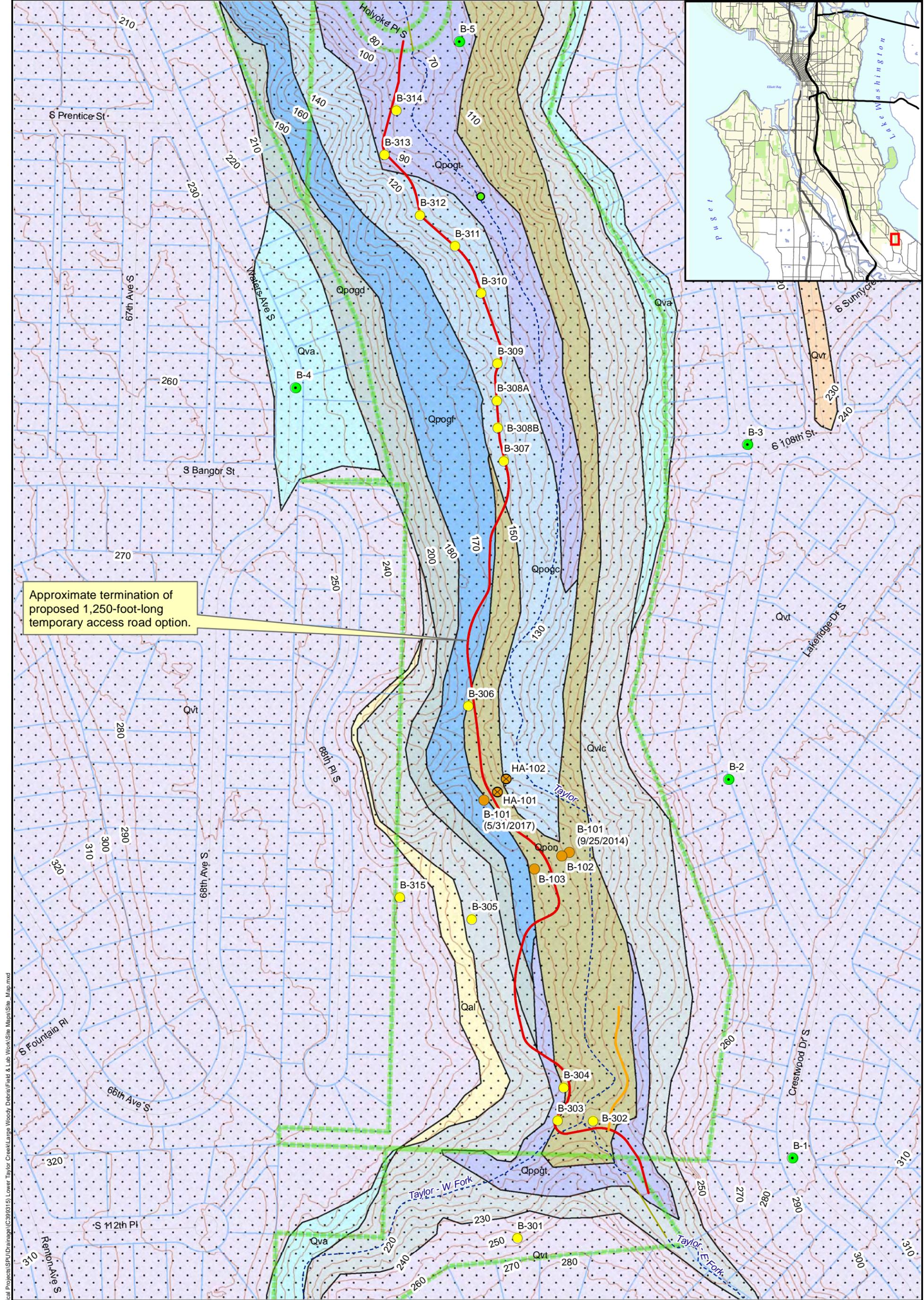
Figure 1
October 2022
Project No. C39315



Approximate termination of proposed 1,250-foot-long temporary access road option.

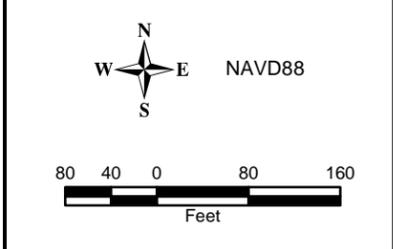
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<p>LEGEND</p> <ul style="list-style-type: none"> — Proposed Temporary Access Road Alignment — Proposed Temporary Spur Road Alignment ● Known Slide Location Denuded Slopes Headscarp Landslide Deposits Lakeridge Park Parcels 	<p>NAV88</p> <p>100 50 0 100 200</p> <p>Feet</p>	<p>Taylor Creek Restoration Project Dead Horse Canyon Seattle, Washington</p> <p>Site and Landslide Map</p> <p> Seattle Public Utilities Geotechnical Engineering</p>
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Approximate termination of proposed 1,250-foot-long temporary access road option.

LEGEND	
	Boring Completed for Current Study
	Historical Boring by SPU
	Historical Hand Auger by SPU
	Historical Boring by Others
	Proposed Temporary Access Road Alignment
	Proposed Temporary Spur Road Alignment
	Topographic Contour - 10 Ft
	Lakeridge Park
	Parcels
	Landslide Deposits
	Qal Alluvium
	Qpogc Pre-Olympia coarse-grained glacial deposits
	Qpogd Pre-Olympia glacial diamict
	Qpogf Pre-Olympia fine-grained glacial deposits
	Qpogt Pre-Olympia glacial till
	Qpon Pre-Olympia nonglacial deposits
	Qva Vashon advance outwash deposits
	Qvlc Lawton Clay member of the Vashon Drift
	Qvr Vashon recessional outwash deposits
	Qvt Vashon subglacial till

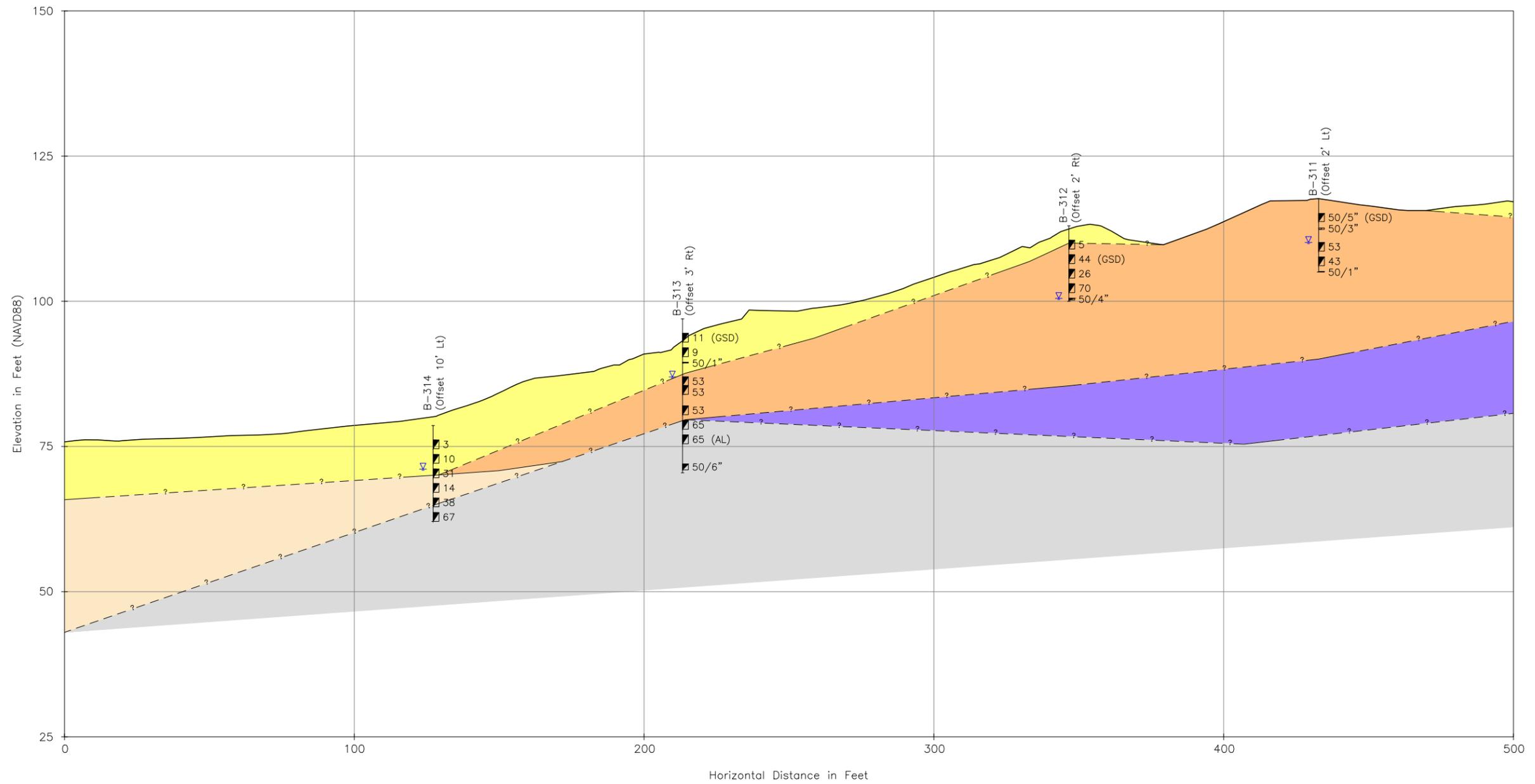


Taylor Creek Restoration Project
Dead Horse Canyon
Seattle, Washington

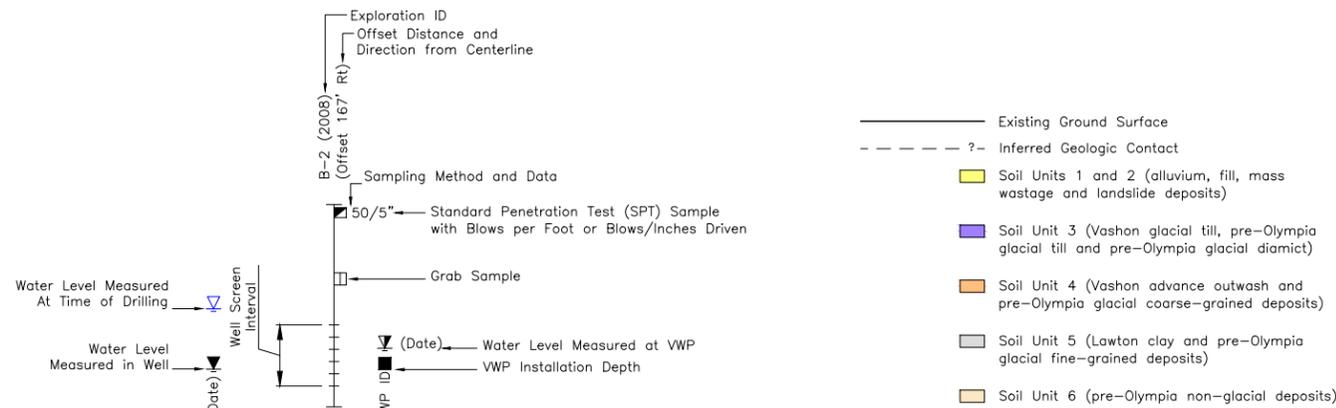
Site, Geology, and Exploration Map

Figure 3
October 2022
Project No. C399315

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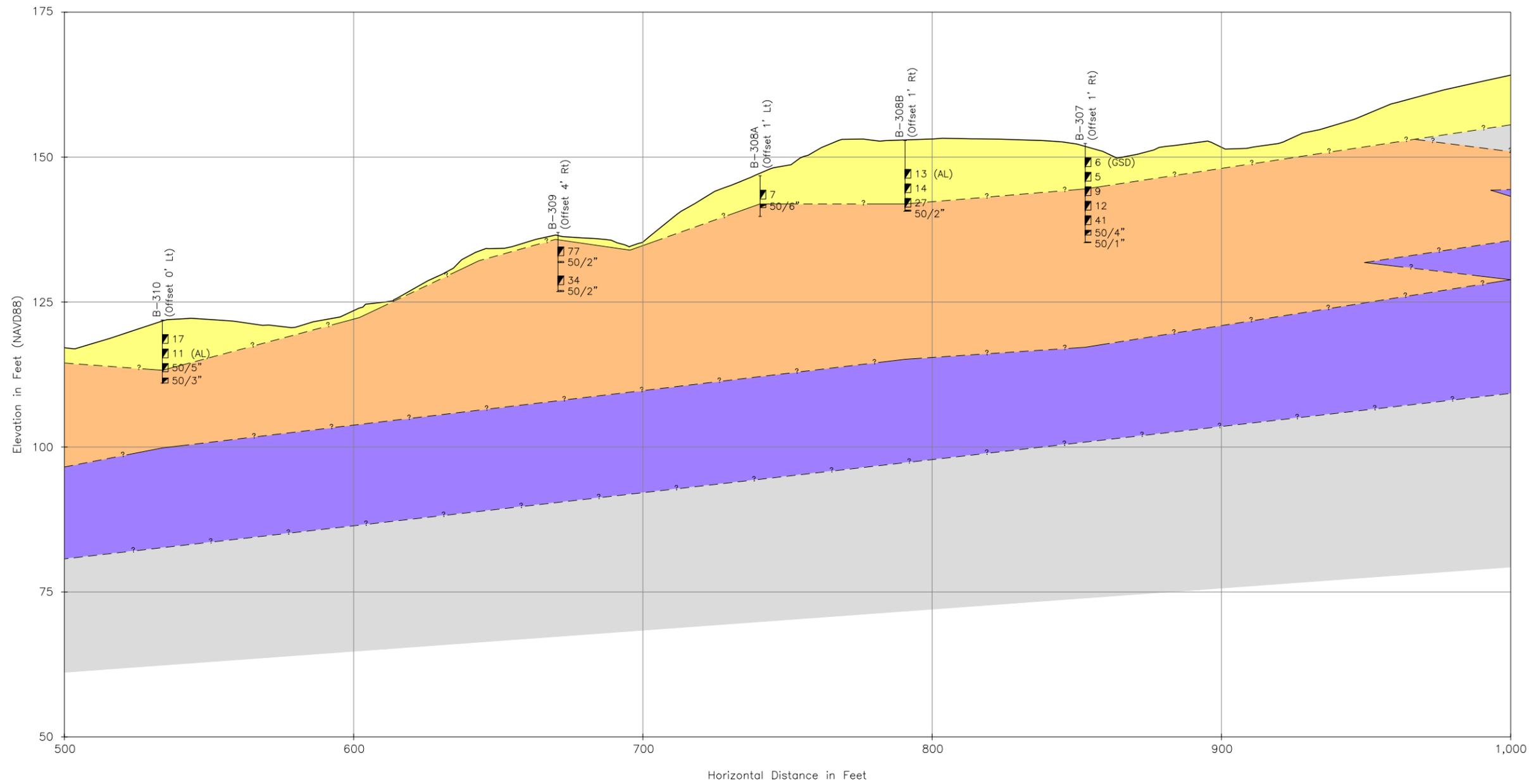


Taylor Creek Restoration
Dead Horse Canyon
Seattle, Washington

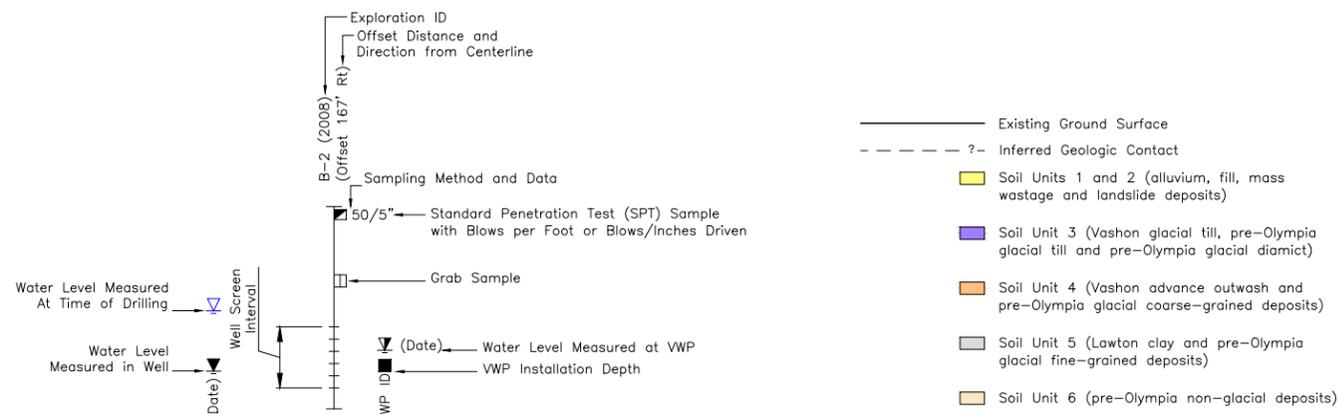
**Generalized Subsurface Profile
Along Access Road Centerline**



Figure 4 (Sheet 1 of 5)
October 2022
C399315



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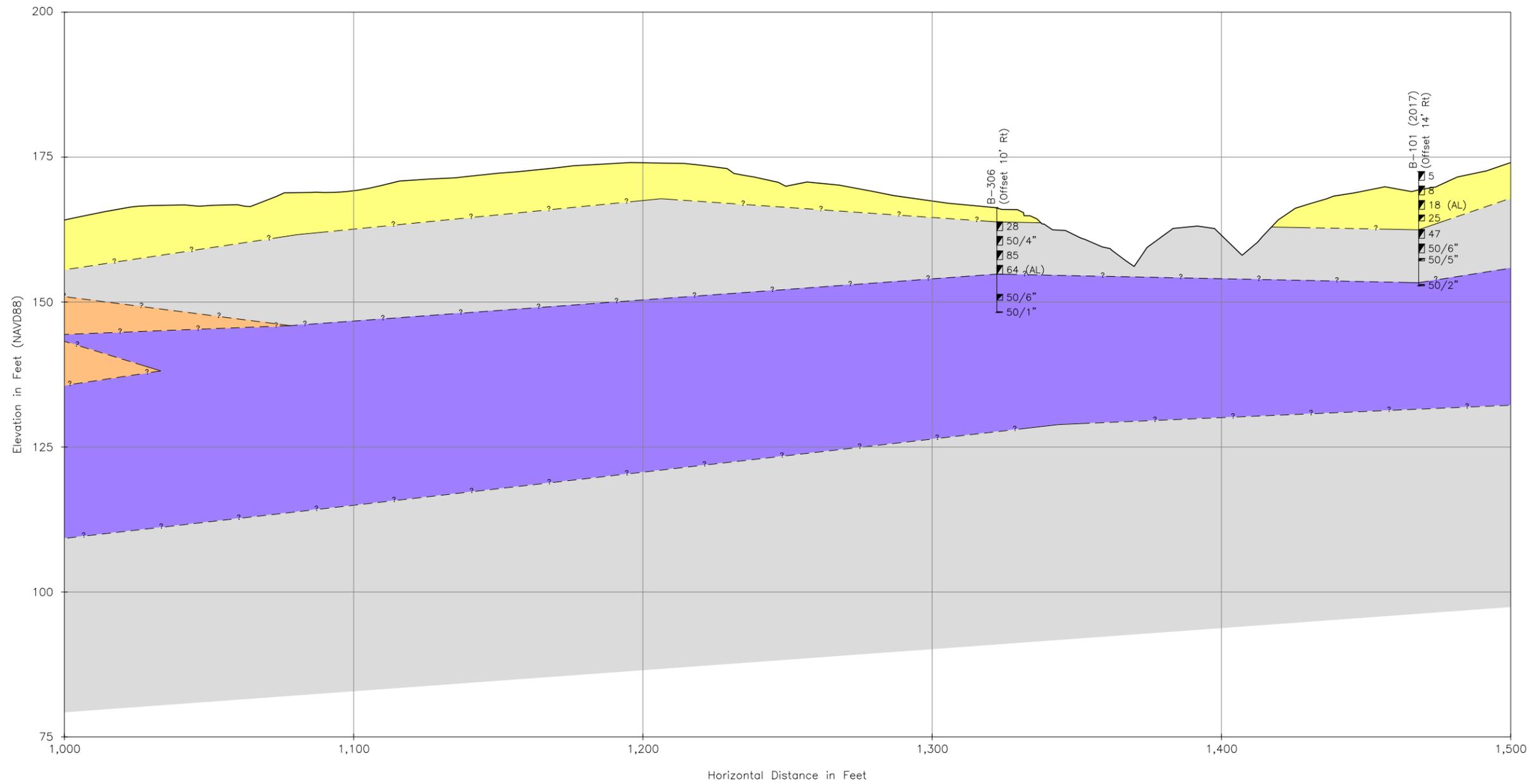


Taylor Creek Restoration
Dead Horse Canyon
Seattle, Washington

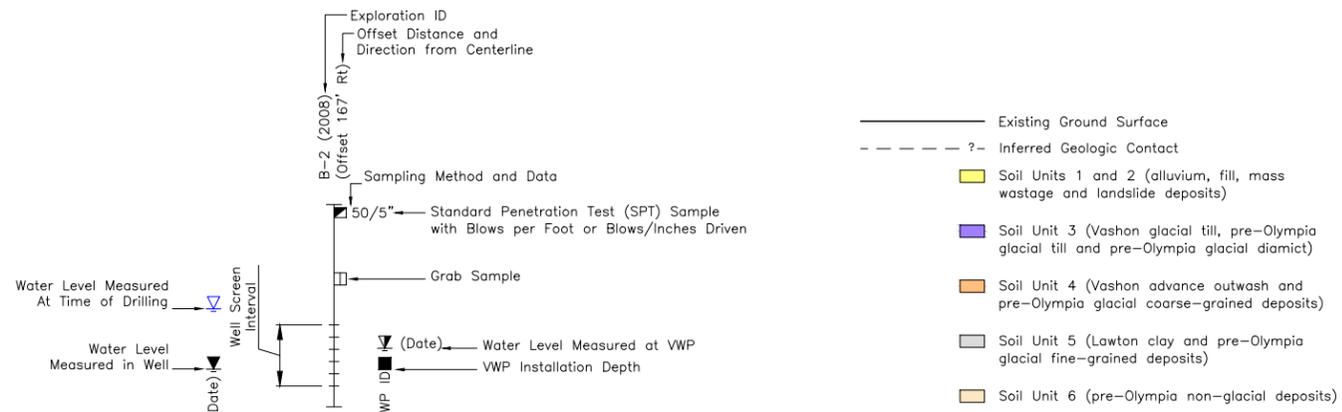
**Generalized Subsurface Profile
Along Access Road Centerline**



Figure 4 (Sheet 2 of 5)
October 2022
C399315



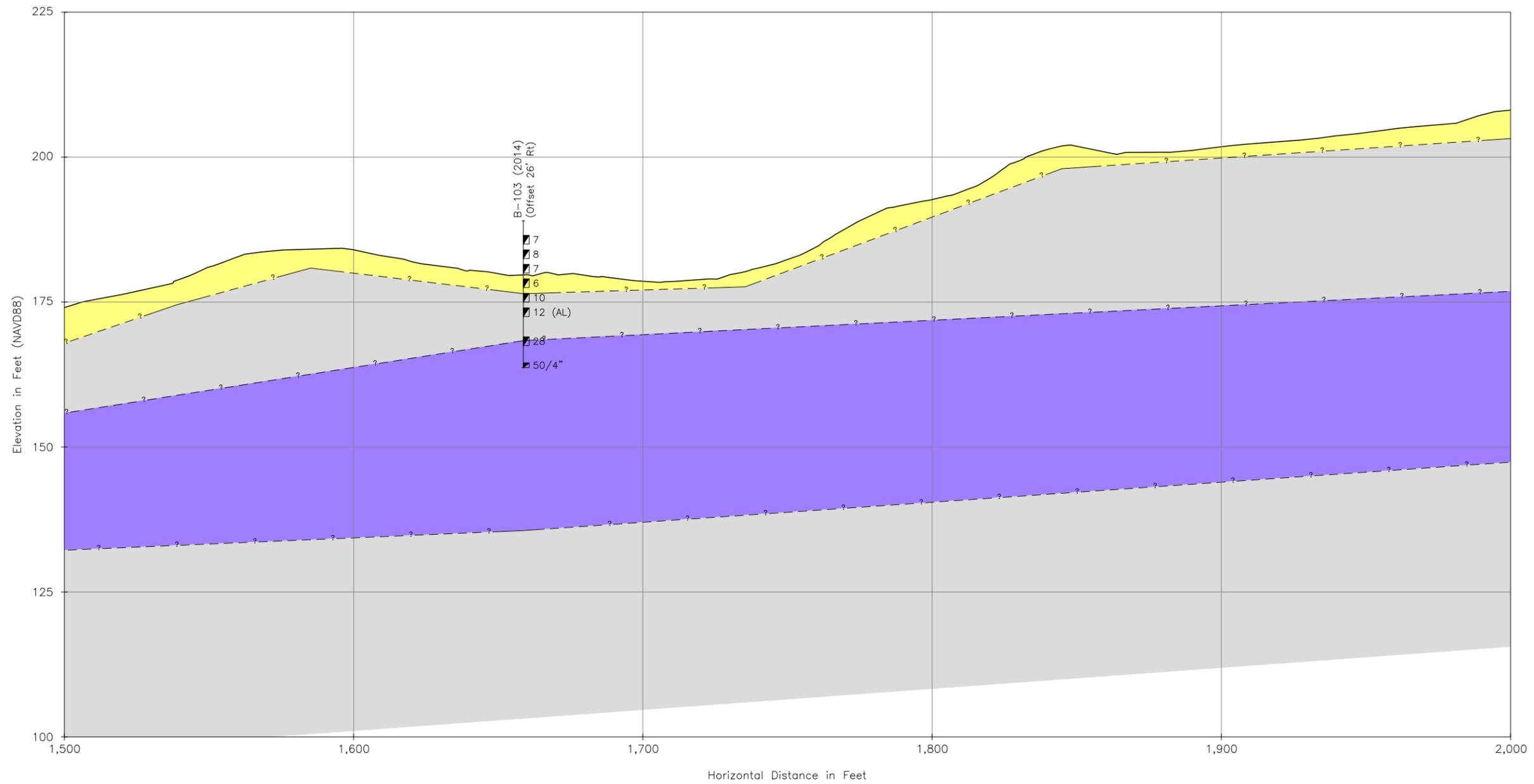
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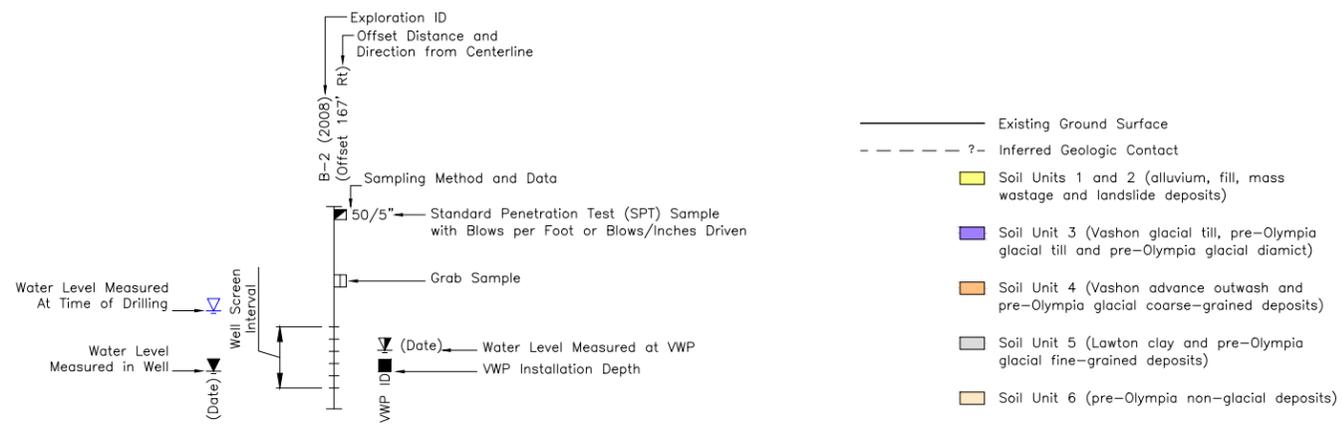
Taylor Creek Restoration
Dead Horse Canyon
Seattle, Washington
Generalized Subsurface Profile
Along Access Road Centerline



Figure 4 (Sheet 3 of 5)
 October 2022
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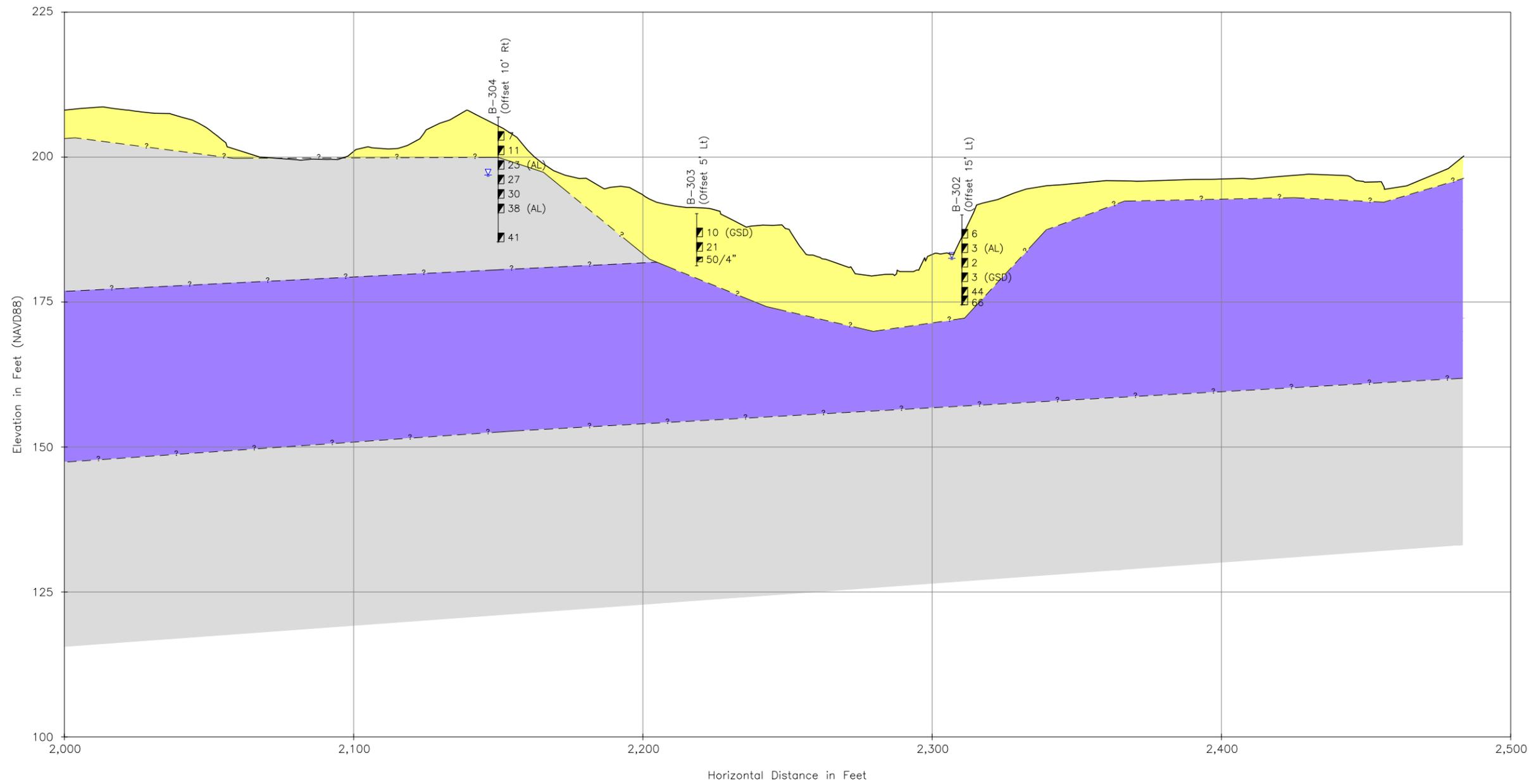


- Existing Ground Surface
- - - ? - Inferred Geologic Contact
- Soil Units 1 and 2 (alluvium, fill, mass wastage and landslide deposits)
- Soil Unit 3 (Vashon glacial till, pre-Olympia glacial till and pre-Olympia glacial diamict)
- Soil Unit 4 (Vashon advance outwash and pre-Olympia glacial coarse-grained deposits)
- Soil Unit 5 (Lawton clay and pre-Olympia glacial fine-grained deposits)
- Soil Unit 6 (pre-Olympia non-glacial deposits)

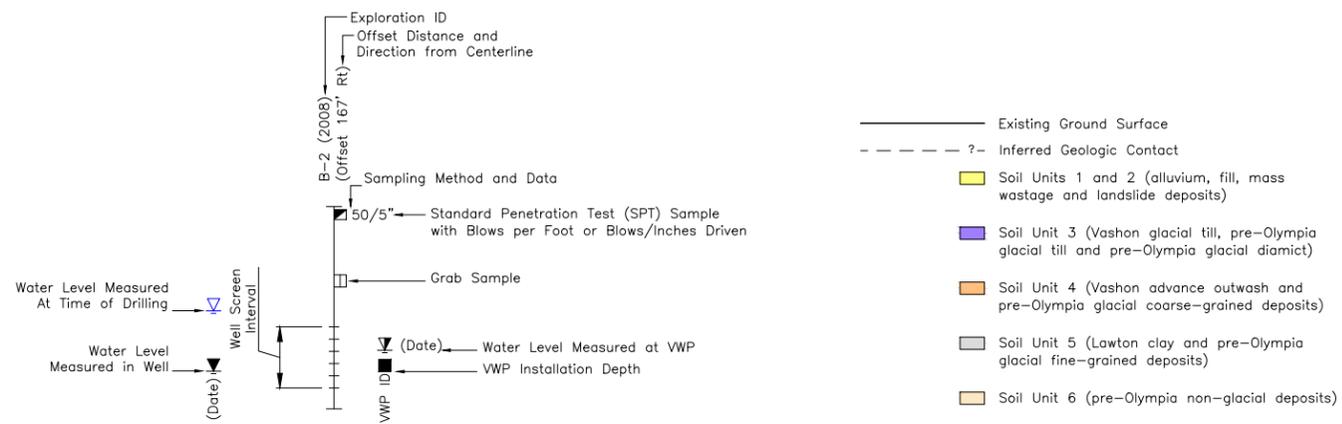
Taylor Creek Restoration
Dead Horse Canyon
Seattle, Washington
Generalized Subsurface Profile
Along Access Road Centerline



Figure 4 (Sheet 4 of 5)
 October 2022
 C399315



LEGEND



Taylor Creek Restoration
Dead Horse Canyon
Seattle, Washington

**Generalized Subsurface Profile
Along Access Road Centerline**

Seattle Public Utilities
Geotechnical Engineering

Figure 4 (Sheet 5 of 5)
October 2022
C399315

APPENDIX A

FIELD EXPLORATION PROGRAM

APPENDIX A

FIELD EXPLORATION PROGRAM

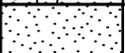
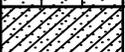
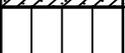
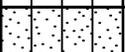
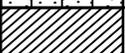
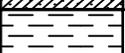
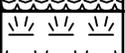
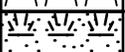
GEOTECHNICAL SOIL BORINGS

Subsurface conditions for the current study were explored using hollow stem auger (HSA) drilling techniques. 16 borings, B-301 through B-307, B-308A, B-308B, and B-309 through B-315, were completed to depths ranging from 7 to 26.5 feet between April 13 and April 21, 2021. The approximate location of the explorations is shown on Figure 1 in the main body of the text. The explorations were located relative to prominent features in the area. The approximate ground surface elevations at the exploration locations are referenced to the NAVD88 datum.

All borings were drilled by Geologic Drill Partners. Borings B-301 through B-305 were drilled using a hand carried Acker drill rig with a 2¼-inch inner diameter (ID) HSA. Borings B-306 through B-315 were drilled using an MT-55 tracked drill rig with 4¼-inch ID HSA. The results of the explorations are summarized on the individual summary boring logs, which are included here as Figures A-2 through A-17. A key to the symbols and terms used on the summary logs is presented as Figure A-1.

Soil samples were obtained from all borings at 2.5-foot and 5-foot depth intervals using the Standard Penetration Test (SPT, ASTM D-1586). The 2.0-inch outside diameter (OD) SPT sampler was driven into the soil a distance of 18 inches using a 140-pound drive automatic trip hammer or cathead falling a distance of 30 inches. Recorded blows for each 6 inches of sampler penetration (blow counts) are shown on the summary logs in this appendix. The blow counts provide a qualitative measure of the relative density of cohesionless soil, or the relative consistency of fine-grained soils. Representative portions of all recovered samples were placed in sealed containers and transported to our laboratory for further observation and testing.

UNIFIED SOIL CLASSIFICATION SYSTEM - ASTM D2488

MAJOR DIVISION		GROUP SYMBOL	LETTER SYMBOL	GROUP NAME		
COARSE GRAINED SOILS CONTAINS LESS THAN 50% FINES	GRAVEL AND GRAVELLY SOILS MORE THAN 50% OF COARSE FRACTION <u>RETAINED</u> ON NO. 4 SIEVE	GRAVEL WITH \leq 5% FINES		GW	Well-graded GRAVEL	
				GW	Well-graded GRAVEL WITH SAND	
				GP	Poorly graded GRAVEL	
				GP	Poorly graded GRAVEL WITH SAND	
		GRAVEL WITH BETWEEN 5% AND 15% FINES		GW-GM	Well-graded GRAVEL WITH SILT	
				GW-GC	Well-graded GRAVEL WITH CLAY	
				GP-GM	Poorly graded GRAVEL WITH SILT	
				GP-GC	Poorly graded GRAVEL WITH CLAY	
			GRAVEL WITH \geq 15% FINES		GM	SILTY GRAVEL
					GC	CLAYEY GRAVEL
	SAND AND SANDY SOILS MORE THAN 50% OF COARSE FRACTION <u>PASSING</u> ON NO. 4 SIEVE	SAND WITH \leq 5% FINES		SW	Well-graded SAND	
				SW	Well-graded SAND WITH GRAVEL	
				SP	Poorly graded SAND	
				SP	Poorly graded SAND WITH GRAVEL	
		SAND WITH BETWEEN 5% AND 15% FINES		SW-SM	Well-graded SAND WITH SILT	
				SW-SC	Well-graded SAND WITH CLAY	
				SP-SM	Poorly graded SAND WITH SILT	
				SP-SC	Poorly graded SAND WITH CLAY	
		SAND WITH \geq 15% FINES		SM	SILTY SAND	
				SC	CLAYEY SAND	
FINE GRAINED SOILS CONTAINS MORE THAN 50% FINES	SILT AND CLAY	LIQUID LIMIT <u>LESS</u> THAN 50		ML	Inorganic SILT, low plasticity	
				ML	Inorganic SILT WITH SAND, low plasticity	
				CL	Lean inorganic CLAY, low plasticity	
				CL	Lean inorganic CLAY WITH SAND, low plasticity	
				OL	ORGANIC SILT, low plasticity	
			LIQUID LIMIT <u>GREATER</u> THAN 50		MH	Elastic inorganic SILT, moderate to high plasticity
		CH		Fat inorganic CLAY, moderate to high plasticity		
		OH		ORGANIC SILT or CLAY, moderate to high plasticity		
	HIGHLY ORGANIC SOILS			PT	PEAT soils with high organic contents	
TOPSOIL			TP	TOPSOIL		

NOTES:

1. Sample descriptions are based on visual field and laboratory observations using classification methods of ASTM D2488. Where laboratory data are available, classifications are in accordance with ASTM D2487.
2. Solid lines between soil descriptions indicate change in interpreted geologic unit. Dashed lines indicate stratigraphic change within the unit.
3. Fines are material passing the U.S. std. #200 sieve.

SOIL CLASSIFICATION AND EXPLORATION LOG KEY



Seattle Public Utilities
Geotechnical Engineering

FIGURE A-1
(Sheet 1 of 2)

SAMPLING METHOD

- 2" OD SPT Split Spoon Sample with 140 lb hammer falling 30" (ASTM D1587)
- No Recovery
- Shelby Tube Sample (ASTM D1587)
- 3" OD Split Spoon Sample with 300 lb hammer falling 30"
- Grab Sample
- Non Standard (As noted on log)
- Core Run

LABORATORY TESTS

- AL Atterberg Limits
- FC Fines Content
- GSD Grain Size Distribution
- ENV Environmental Testing
- SG Specific Gravity
- MD Moisture Density Relationship
- C Consolidation
- UU Unconsolidated Undrained Triaxial
- CU Consolidated Undrained Triaxial
- CD Consolidated Drained Triaxial
- UCS Unconfined Compression Strength
- PERM Hydraulic Conductivity Test
- PP Pocket Penetrometer
- TV Torvane
- DS Direct Shear
- ORG Organic Content
- PID Photoionization Detector Reading

COMPONENT DEFINITIONS

- Boulders Larger than 12 in.
- Cobbles 3 in. to 12 in.
- Gravel 3 in. to No. 4 (4.75 mm)
- Coarse Gravel 3 in. to 3/4 in.
- Fine Gravel 3/4 in. to No. 4 (4.75 mm)
- Sand No. 4 (4.75 mm) to No. 200 (0.075 mm)
- Coarse Sand No. 4 (4.75 mm) to No. 10 (2.00 mm)
- Medium Sand No. 10 (2.00 mm) to No. 40 (0.425 mm)
- Fine Sand No. 40 (0.425 mm) to No. 200 (0.075 mm)
- Silt and Clay Smaller than No. 200 (0.075 mm)

RELATIVE DENSITY OF COARSE-GRAINED COHESIONLESS SOILS

Relative Density	N (blows/ft)	Approximate Relative Density (lb/ft ³)	1/2" Dia. Metal Probe Penetration Depth (ft)
Very Loose	0 to 4	0 - 15	> 3
Loose	4 to 10	15 - 35	1 - 3
Medium Dense	10 to 30	35 - 65	0.3 - 1
Dense	30 to 50	65 - 85	0.1 - 0.3
Very Dense	over 50	85 - 100	< 0.1

COMPONENT PROPORTIONS

Trace	Less than 5%
Few	5 - 15%
Some	15 - 30%

RELATIVE CONSISTENCY OF FINE-GRAINED COHESIVE SOILS

Relative Consistency	N (blows/ft)	Approximate Undrained Shear Strength (psf)	1/2" Dia. Metal Probe Penetration Depth (ft)
Very Soft	0 to 2	< 250	> 2
Soft	2 to 4	250 - 500	1 - 2
Medium Stiff	4 to 8	500 - 1000	0.5 - 1
Stiff	8 to 15	1000 - 2000	0.25 - 0.5
Very Stiff	15 to 30	2000 - 4000	0.1 - 0.25
Hard	over 30	> 4000	< 0.1

MOISTURE CONTENT

- Dry Dusty, or dry to the touch.
- Moist No visible water. Near optimum moisture content.
- Wet Visible free water.
- Saturated Water content prevents soil from retaining structure.

ORGANIC CONTENT

TERM	PERCENT BY VOLUME
Occasional	0 to 1
Scattered	1 to 10
Numerous	10 to 30
Organic	30 to 50
PEAT	50 to 100

SEEPAGE

- Slow** A small amount of water is observed flowing from the sides of the excavation.
- Moderate** Water collects in the bottom of the excavation during digging. Some bailing is needed to observe the excavation bottom.
- Rapid** Water collects in the bottom of the excavation during digging. Bailing may be ineffective to observe the excavation bottom.

CAVING

- Slight** Soil sloughed from wall of excavation is < 6" thick.
- Moderate** Soil sloughed from wall of excavation is between 6" - 12" thick.
- Significant** Soil sloughed from wall of excavation is >12" thick.

DILATANCY

- Slow** Water appears slowly on the surface of the specimen during shaking and does not disappear or disappears slowly upon squeezing.
- Rapid** Water appears quickly on the surface of the specimen during shaking and disappears quickly upon squeezing.

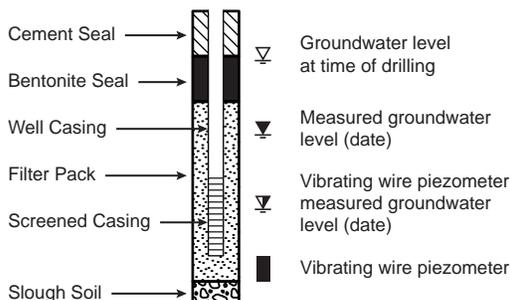
STRUCTURE

TERM	THICKNESS OR SPACING	TERM	CRITERIA / DESCRIPTION	TERM	CRITERIA
Parting	0 - 1/16" thick	Laminated	Alternating layers (< 1/2") of varying material or color	Near Horizontal	0 - 10 degrees
Seam	1/16 - 1/2" thick	Interbedded	Alternating layers (> 1/2") of varying material or color	Low Angle	10 - 45 degrees
Layer	1/2 - 12" thick	Fractured	Breaks easily along definite fracture planes	High Angle	45 - 80 degrees
Pocket	Inclusions < 1" thick	Slickensided	Polished, glossy, striated fracture planes	Near Vertical	80 - 90 degrees
Occasional	< 1 occurrence per foot	Blocky	Readily breaks into small angular lumps		
Scattered	1 > 10 occurrence per foot	Lensed	Inclusions of small pockets of different soil		
Numerous	> 10 occurrence per foot	Homogenous	Same color and appearance throughout		

ORDER OF CLASSIFICATION TERMS

Density/consistency, color, USCS group name, minor constituent; moisture; additional comments.

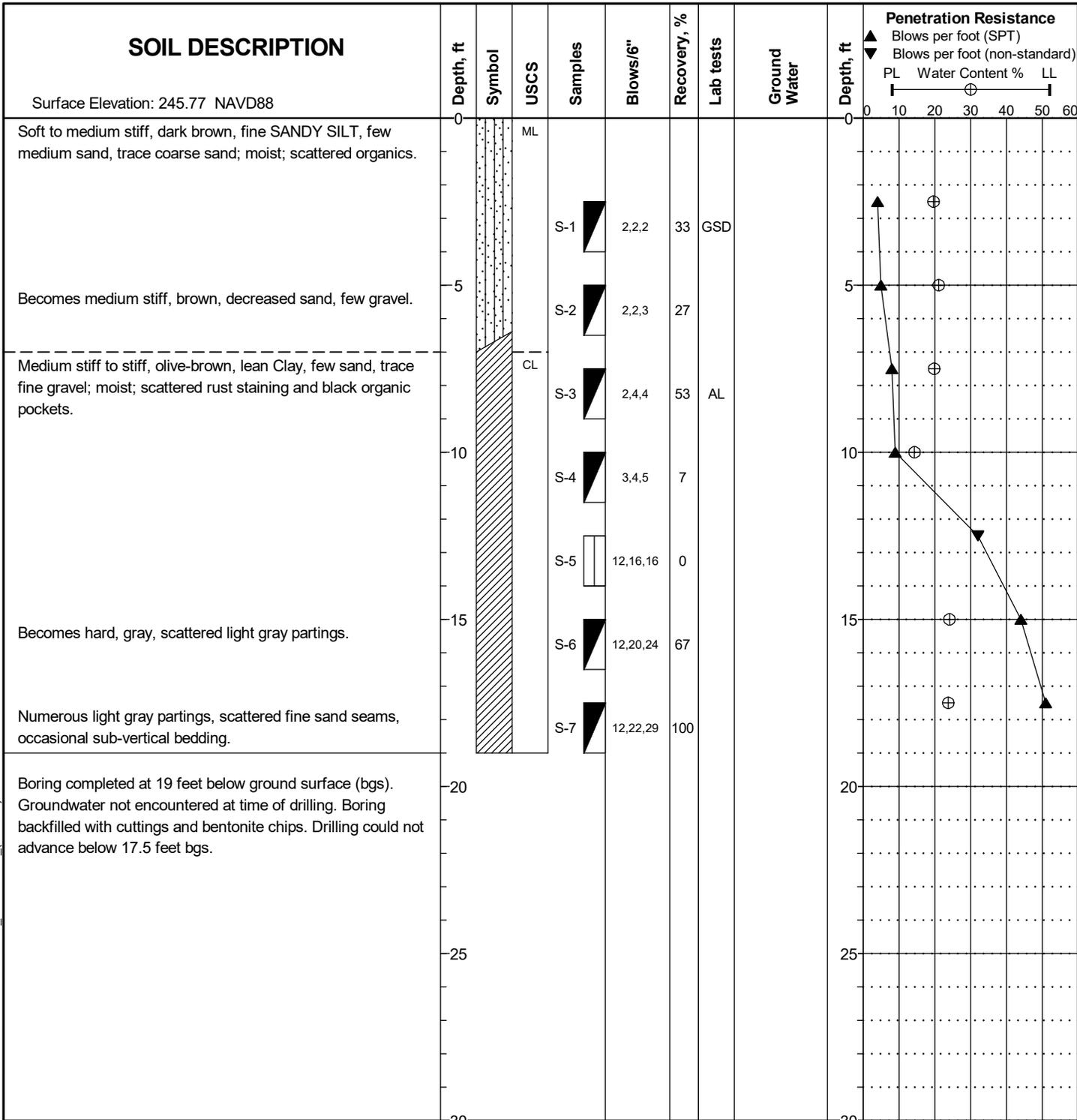
WELL CONSTRUCTION



SOIL CLASSIFICATION AND EXPLORATION LOG KEY



FIGURE A-1 (Sheet 2 of 2)



LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE (7-21-11).GDT 10/19/22

Date Completed: 4/13/2021
 Driller: Geologic Drill Partners
 Equipment: Acker
 Drilling Method: 2-1/4 inch ID HSA
 Hammer System: Cathead

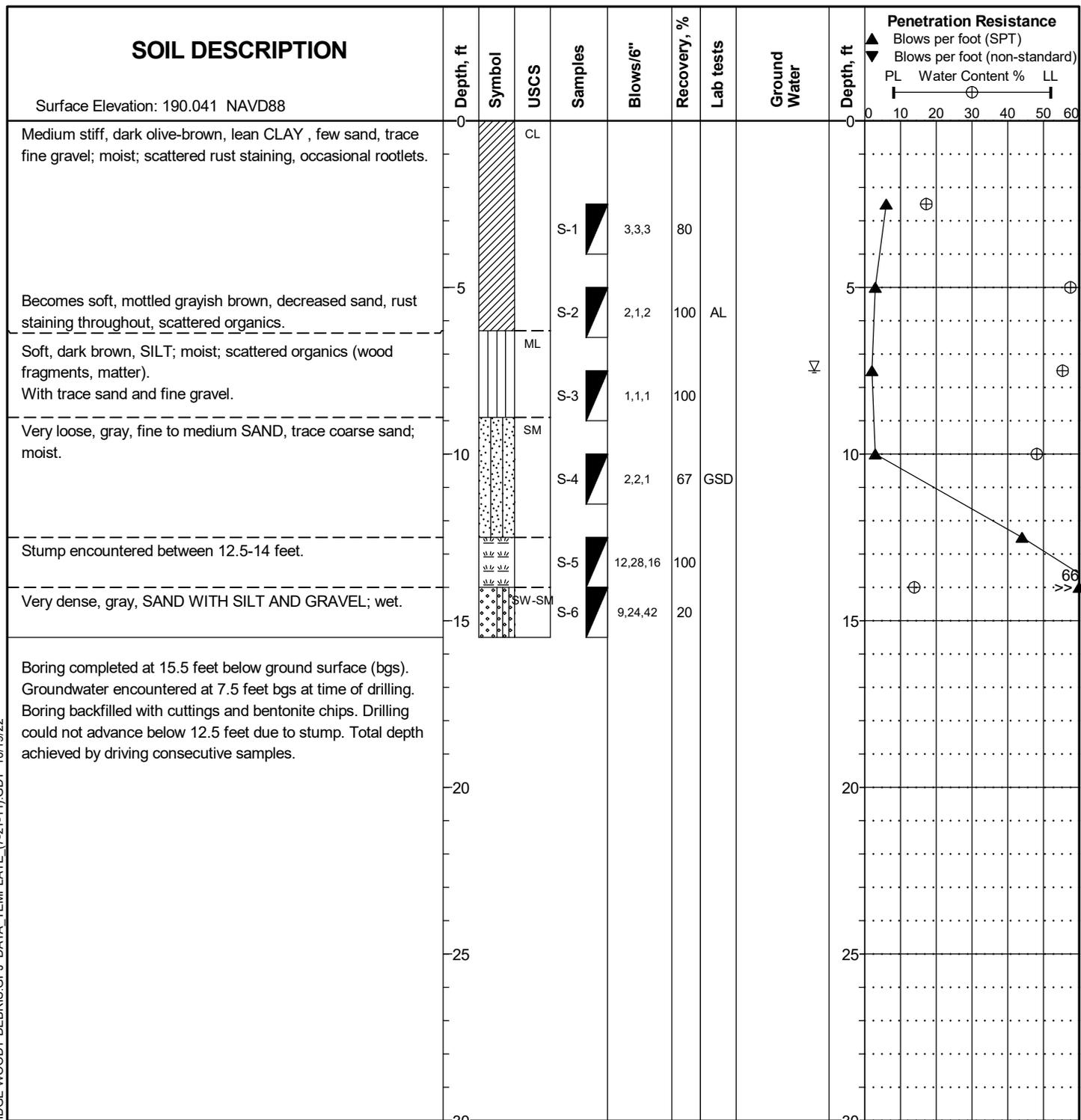
Approximate Location: 3.5 feet west and 4 feet north of survey marker 61012. (N: 186522.1871 E: 1290999.469)

**Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA**



LOG OF BORING B-301
C399315 FIGURE A-2

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/13/2021
 Driller: Geologic Drill Partners
 Equipment: Acker
 Drilling Method: 2-1/4 inch ID HSA
 Hammer System: Cathead

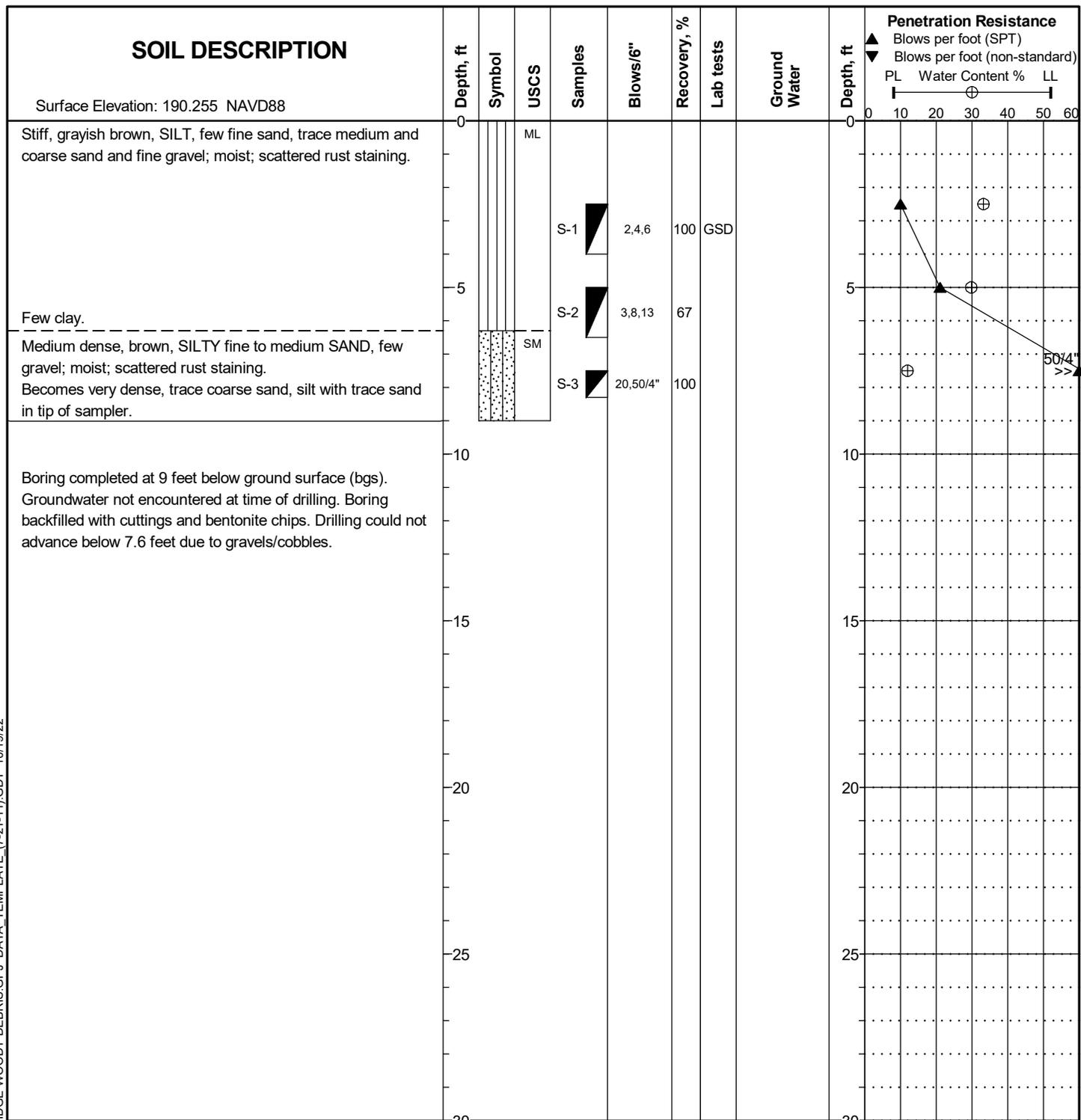
Approximate Location: 5 feet east of east bridge edge. 1.5 feet south of north bridge abutment. (N: 186739.0937 E: 1291138.994)

**Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA**



LOG OF BORING B-302
C399315 FIGURE A-3

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/14/2021
 Driller: Geologic Drill Partners
 Equipment: Acker
 Drilling Method: 2-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 6.5 feet west and 4 feet north of survey marker 60843. (N: 186739.1768 E: 1291073.407)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**

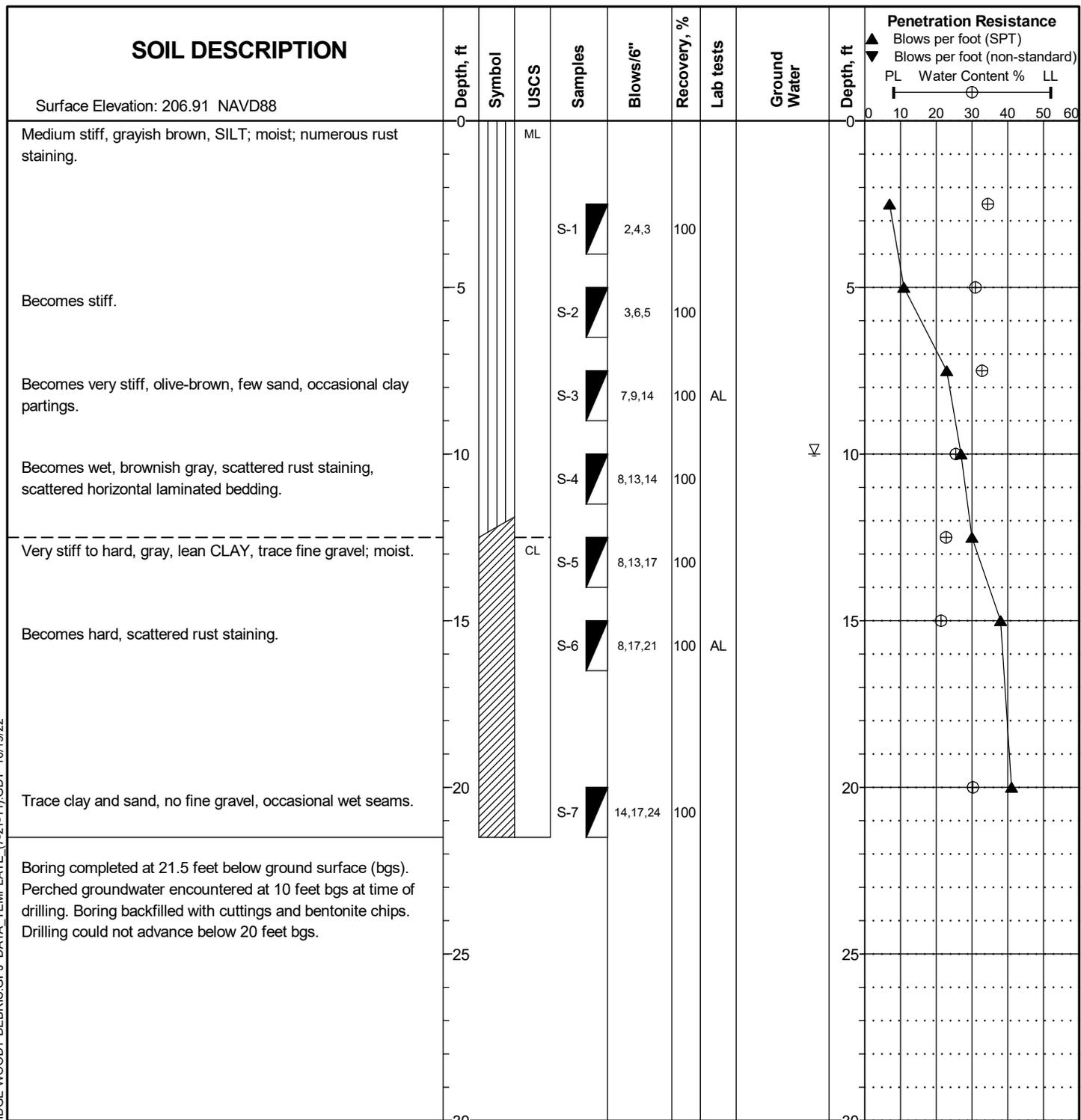


LOG OF BORING B-303

C399315

FIGURE A-4

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE (7-21-11).GDT 10/19/22



Date Completed: 4/14/2021
 Driller: Geologic Drill Partners
 Equipment: Acker
 Drilling Method: 2-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 9 feet south of survey marker 60827. 11.5 feet north of survey marker 60827. In center of trail. (N: 186800.2225 E: 1291084.593)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**

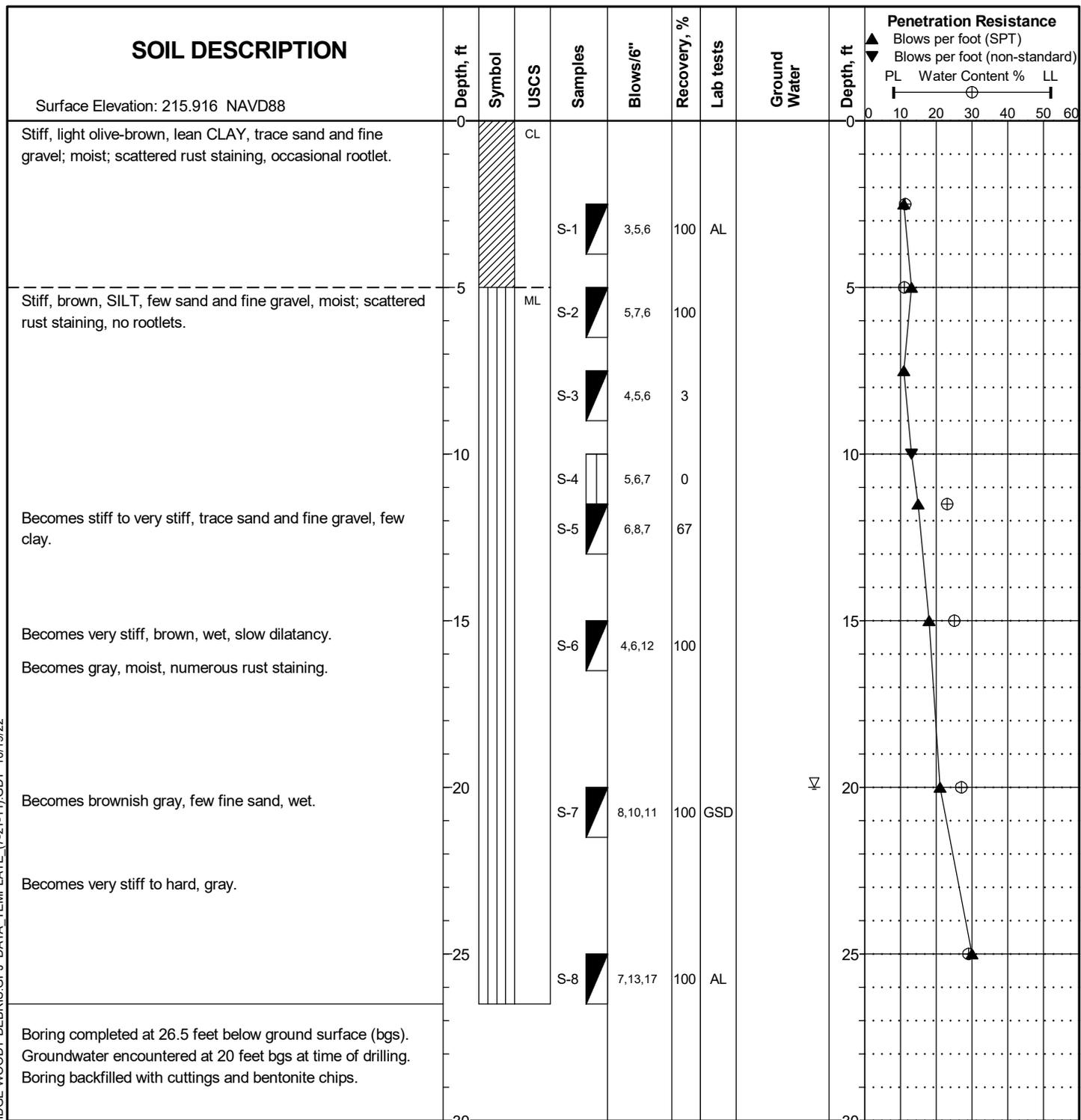


LOG OF BORING B-304

C399315

FIGURE A-5

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



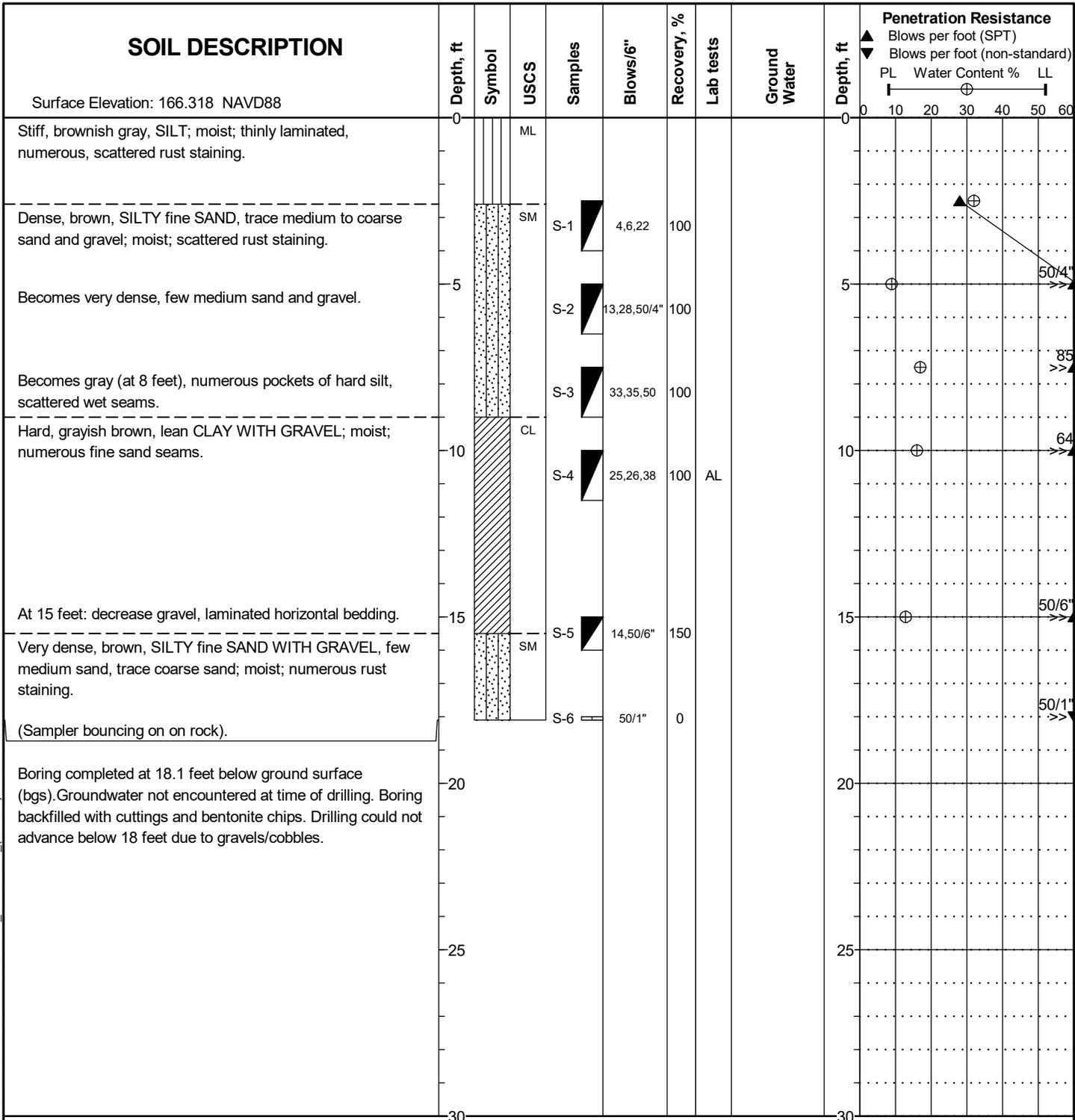
Date Completed: 4/14/2021
 Driller: Geologic Drill Partners
 Equipment: Acker
 Drilling Method: 2-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 16.5 feet east of tree 3678. 7 feet west of tree 3677. 26 feet north of tree 3676. (N: 187112.3277 E: 1290914.909)

**Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA**



LOG OF BORING B-305
C399315 FIGURE A-6



LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE (7-21-11).GDT 10/19/22

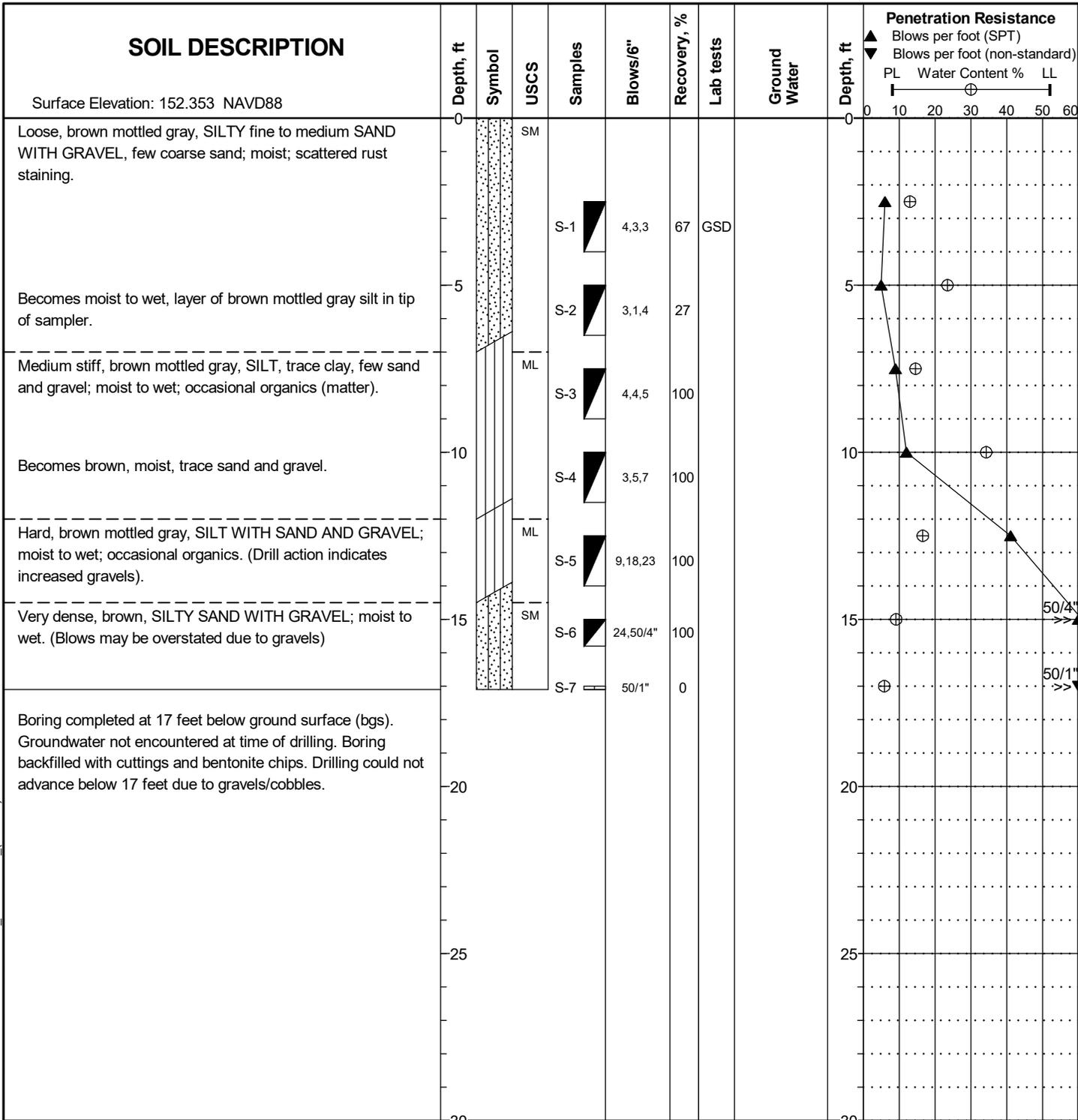
Date Completed: 4/19/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 6.0 feet south and 4 feet west of survey marker 60489. (N: 187507.1509 E: 1290908.698)

**Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA**



LOG OF BORING B-306
C399315 FIGURE A-7



LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22

Date Completed: 4/19/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

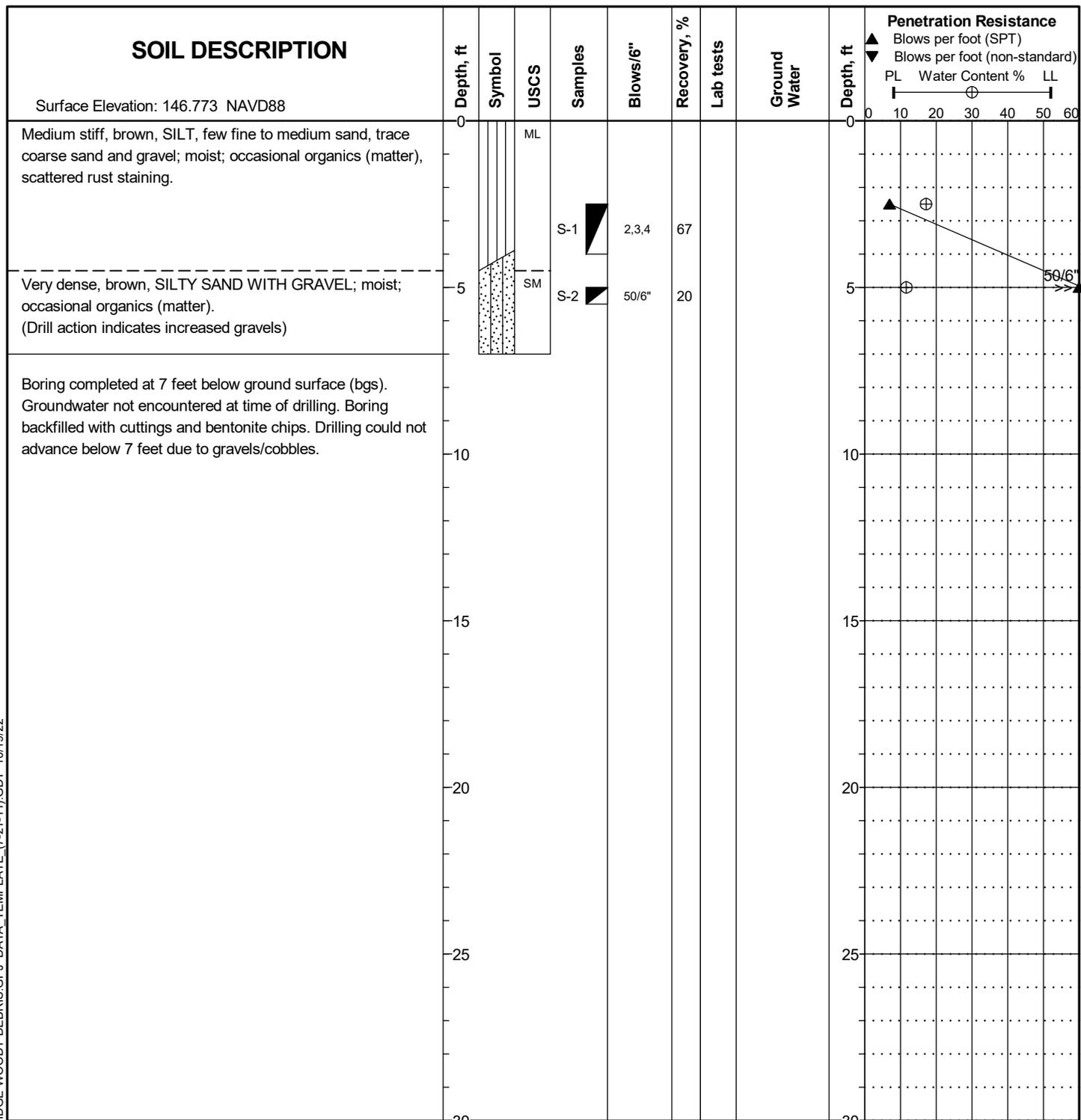
Approximate Location: 11 feet north and 5 feet east of survey marker 60373. (N: 187960.9151 E: 1290974.128)

**Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA**



LOG OF BORING B-307
C399315 FIGURE A-8

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/20/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 1.5 feet south and 1 foot east of survey marker 60323. (N: 188072.3592 E: 1290961.294)

**Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA**

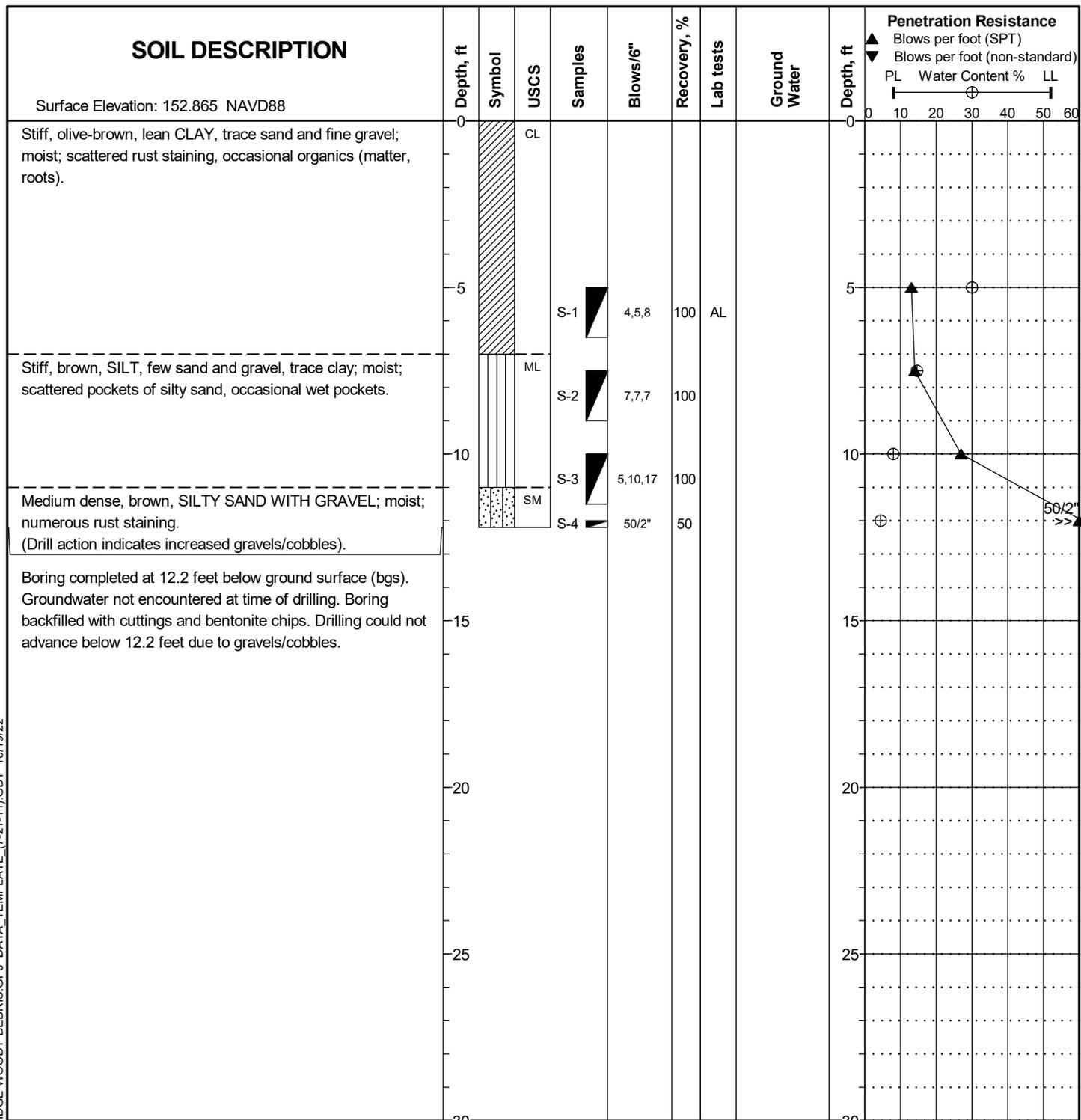


LOG OF BORING B-308A

C399315

FIGURE A-9

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/20/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 1 foot east of survey marker 201. (N: 188022.2266
 E: 1290963.087)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**

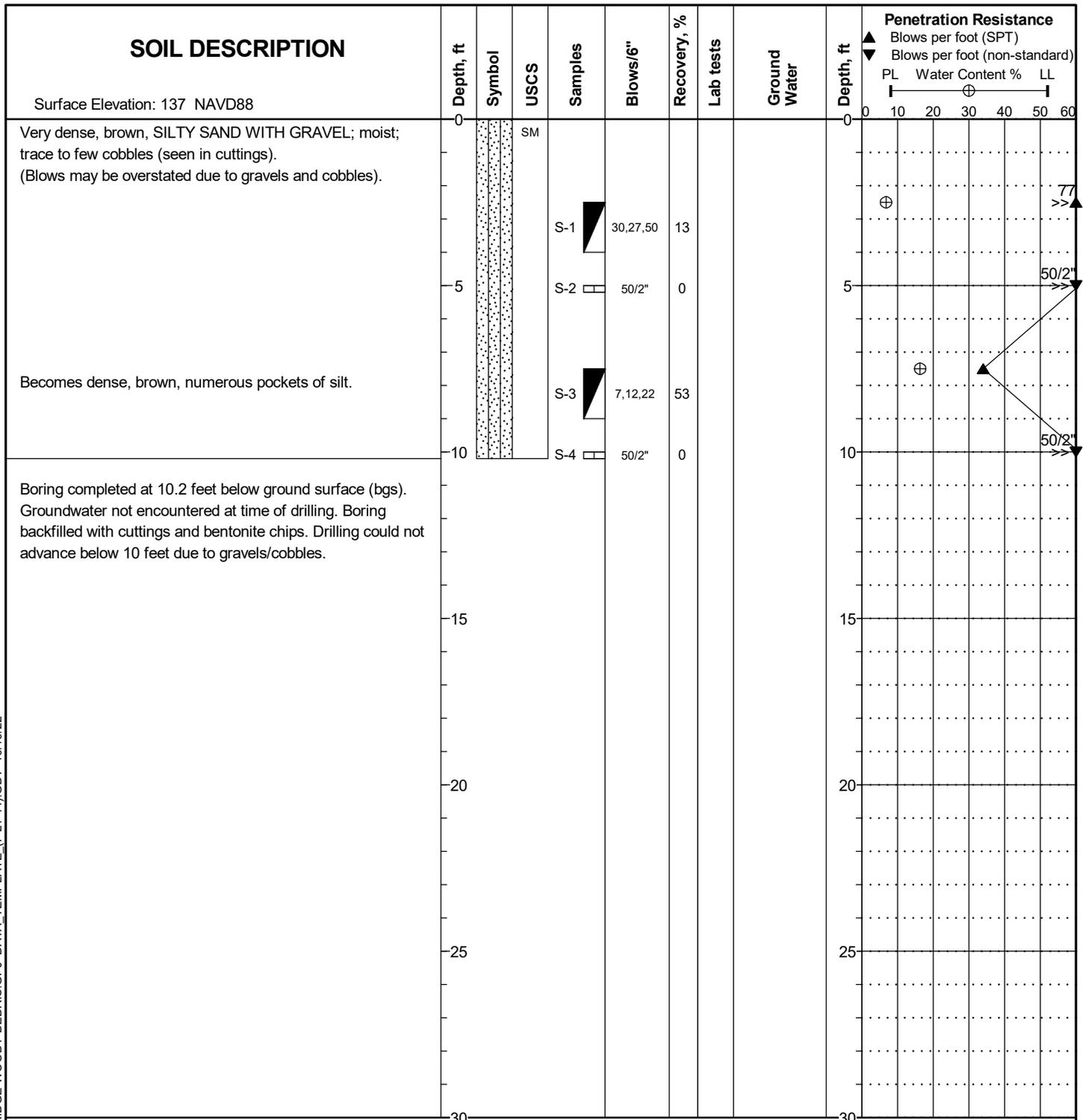


LOG OF BORING B-308B

C399315

FIGURE A-10

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/20/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 3.5 feet south and 2.5 feet east of survey marker 60320. (N: 188141.5785 E: 1290962.356)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**

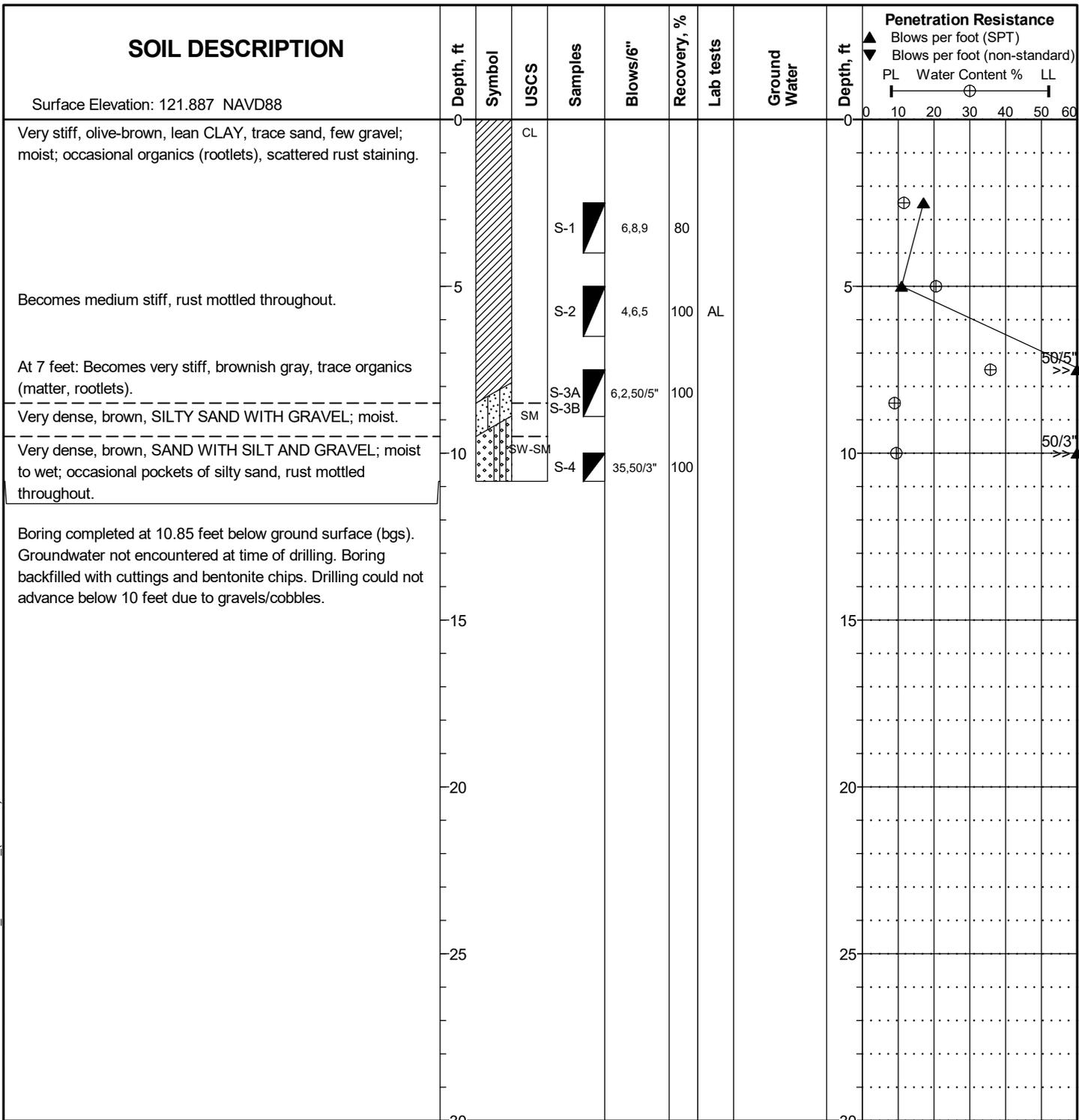


LOG OF BORING B-309

C399315

FIGURE A-11

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/20/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 16 feet south and 3.5 feet east of survey marker 60305. (N: 188271.1168 E: 1290932.917)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**

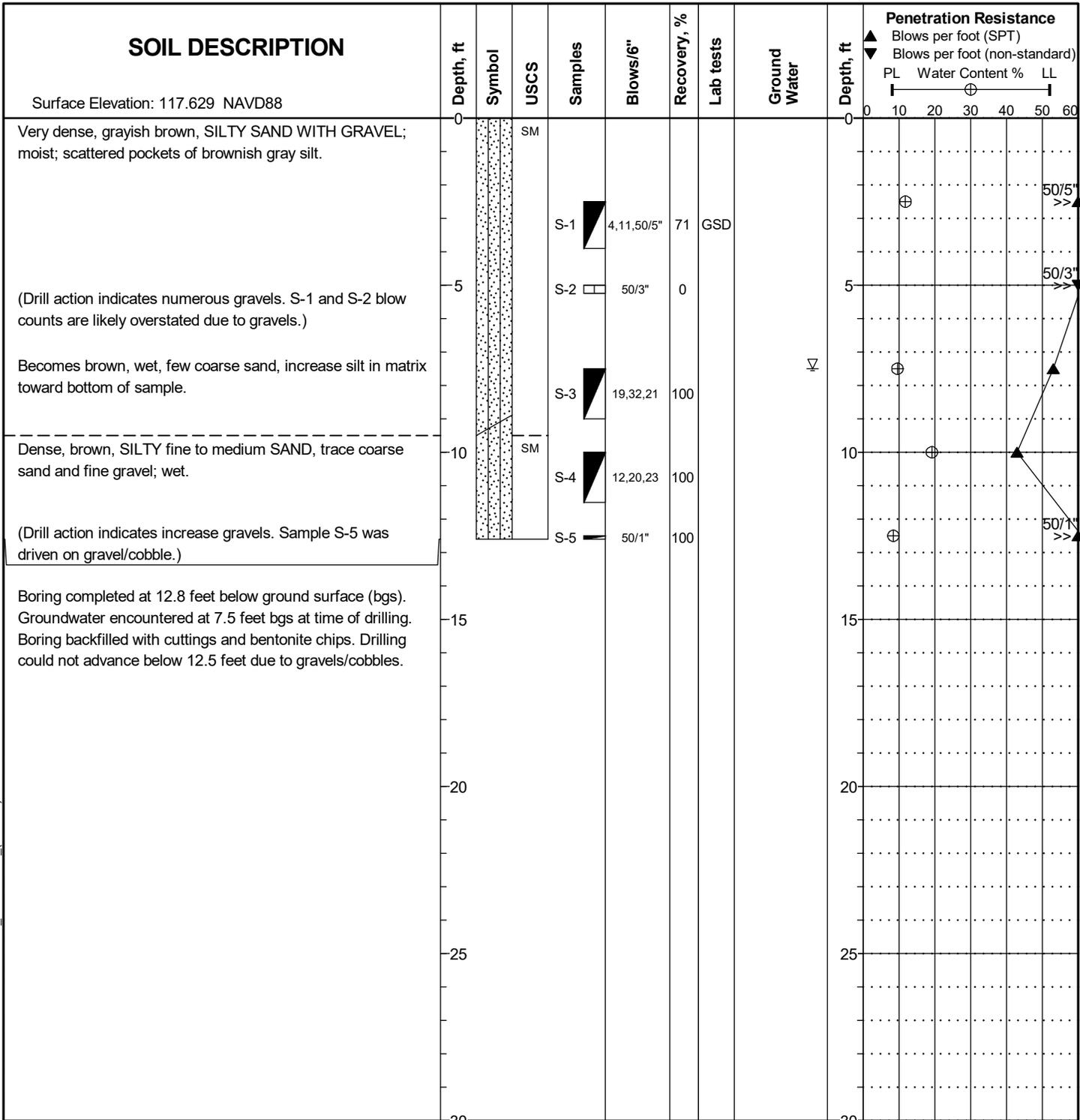


LOG OF BORING B-310

C399315

FIGURE A-12

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/21/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 3.5 feet north and 6.5 feet east of survey marker 201. 19 feet south of survey marker 60262. (N: 188358.6194 E: 1290884.272)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**

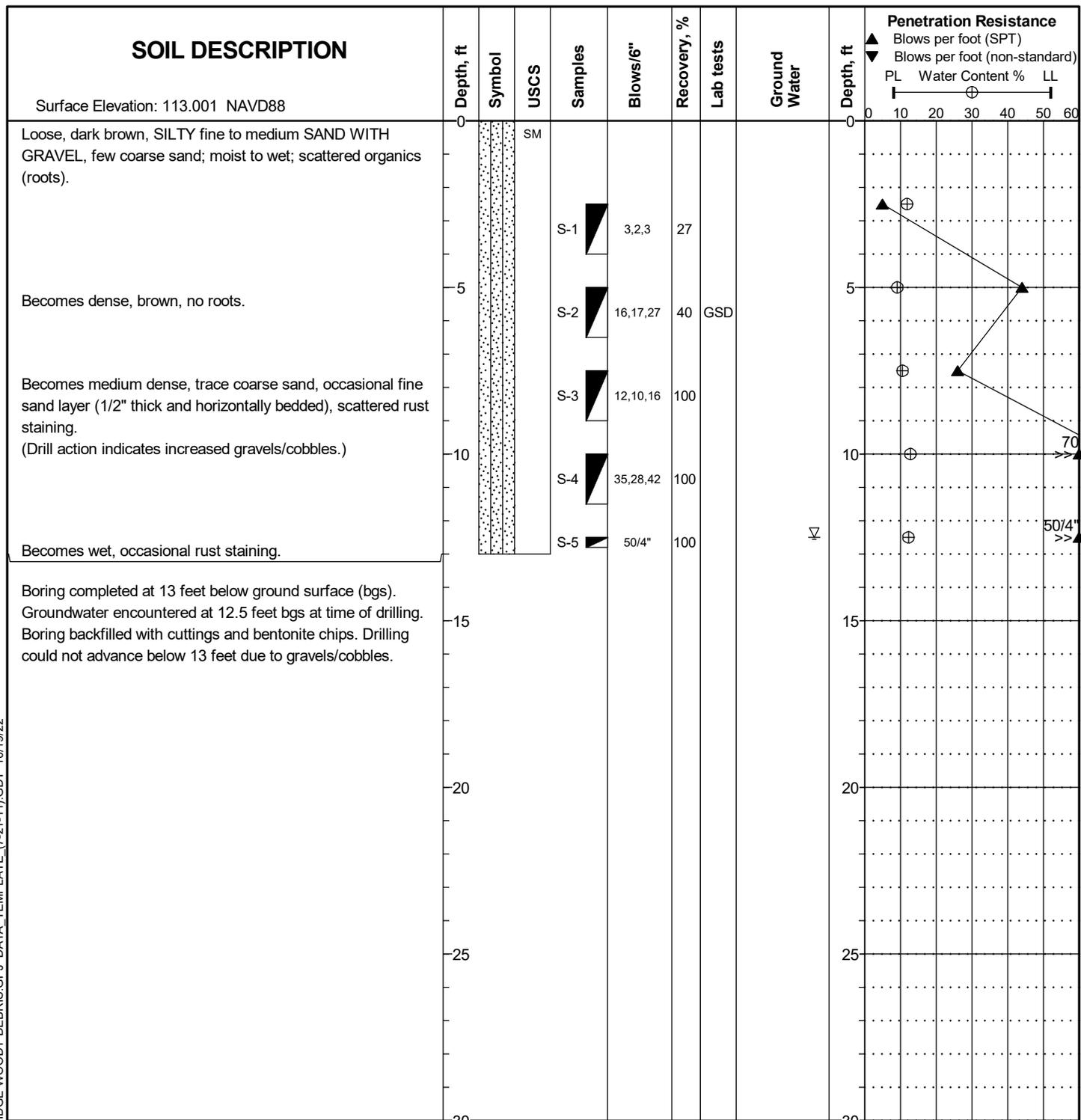


LOG OF BORING B-311

C399315

FIGURE A-13

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/21/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 8 feet south and 3 feet east of survey marker 60248. (N: 188415.95 E: 1290819.558)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**

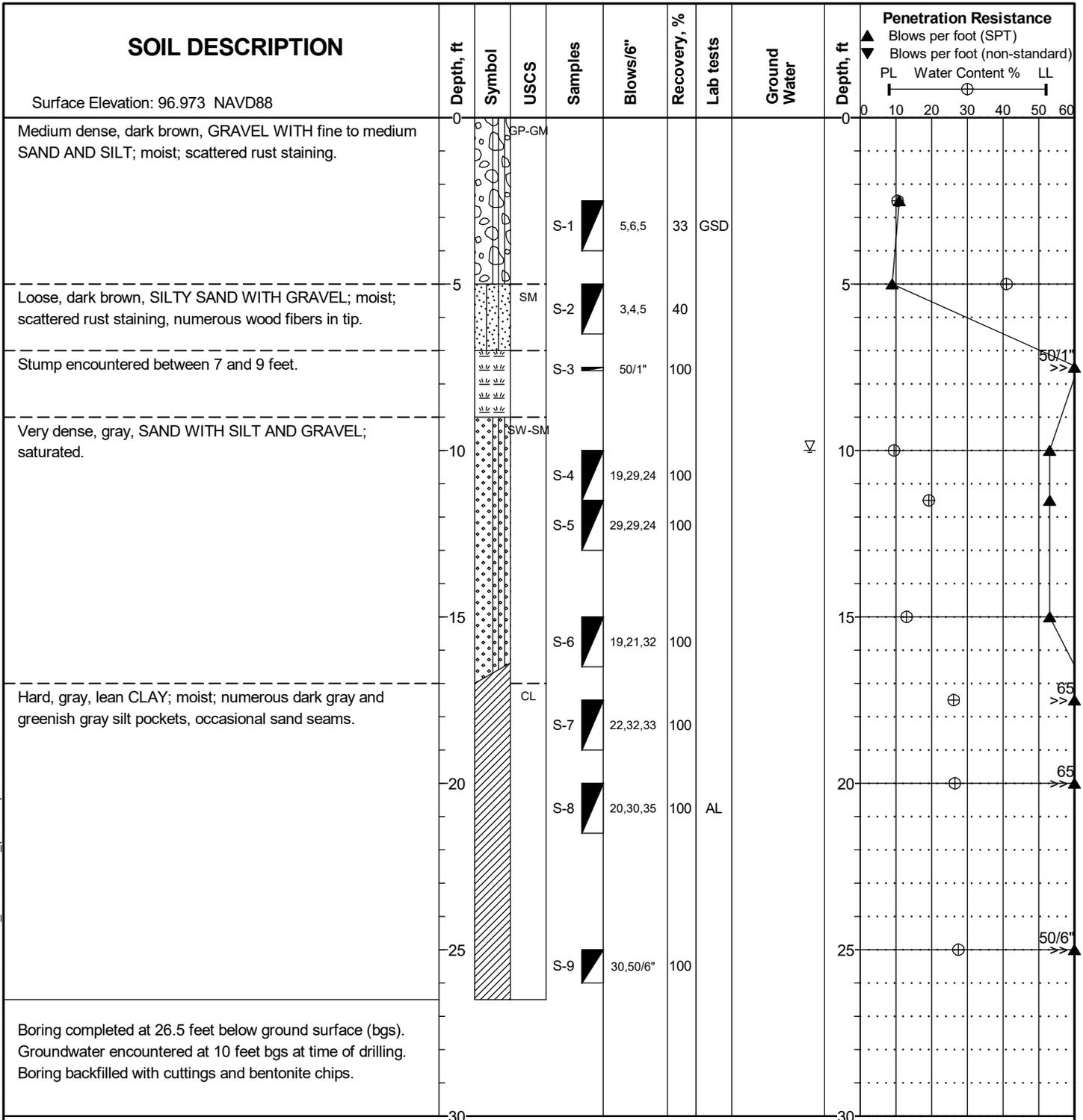


LOG OF BORING B-312

C399315

FIGURE A-14

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/21/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 4 feet north and 5.5 feet east of survey marker
 60122. (N: 188527.89 E: 1290753.798)

**Taylor Creek Restoration: Dead Horse
 Canyon
 Seattle, WA**

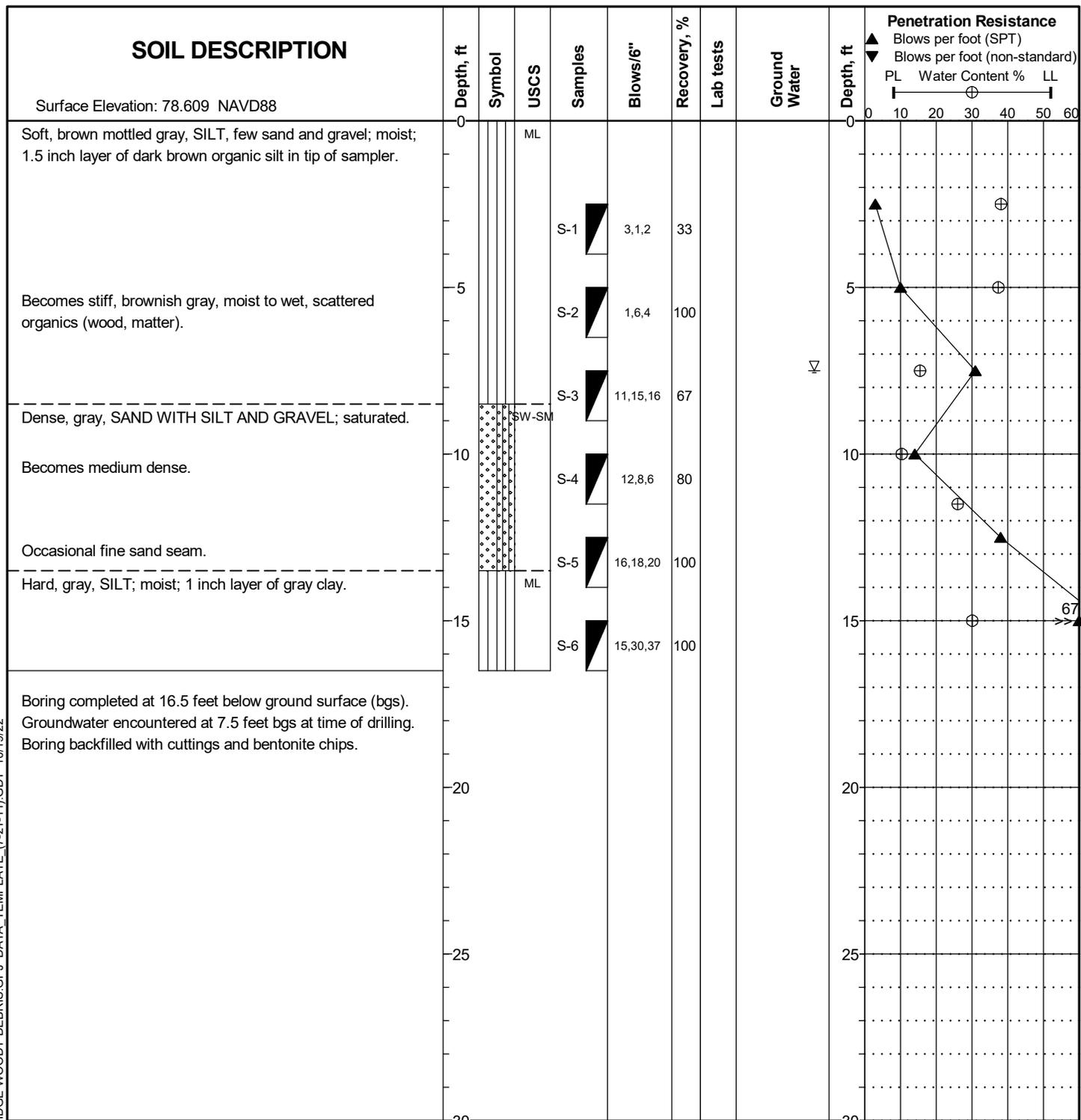


LOG OF BORING B-313

C399315

FIGURE A-15

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/16/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

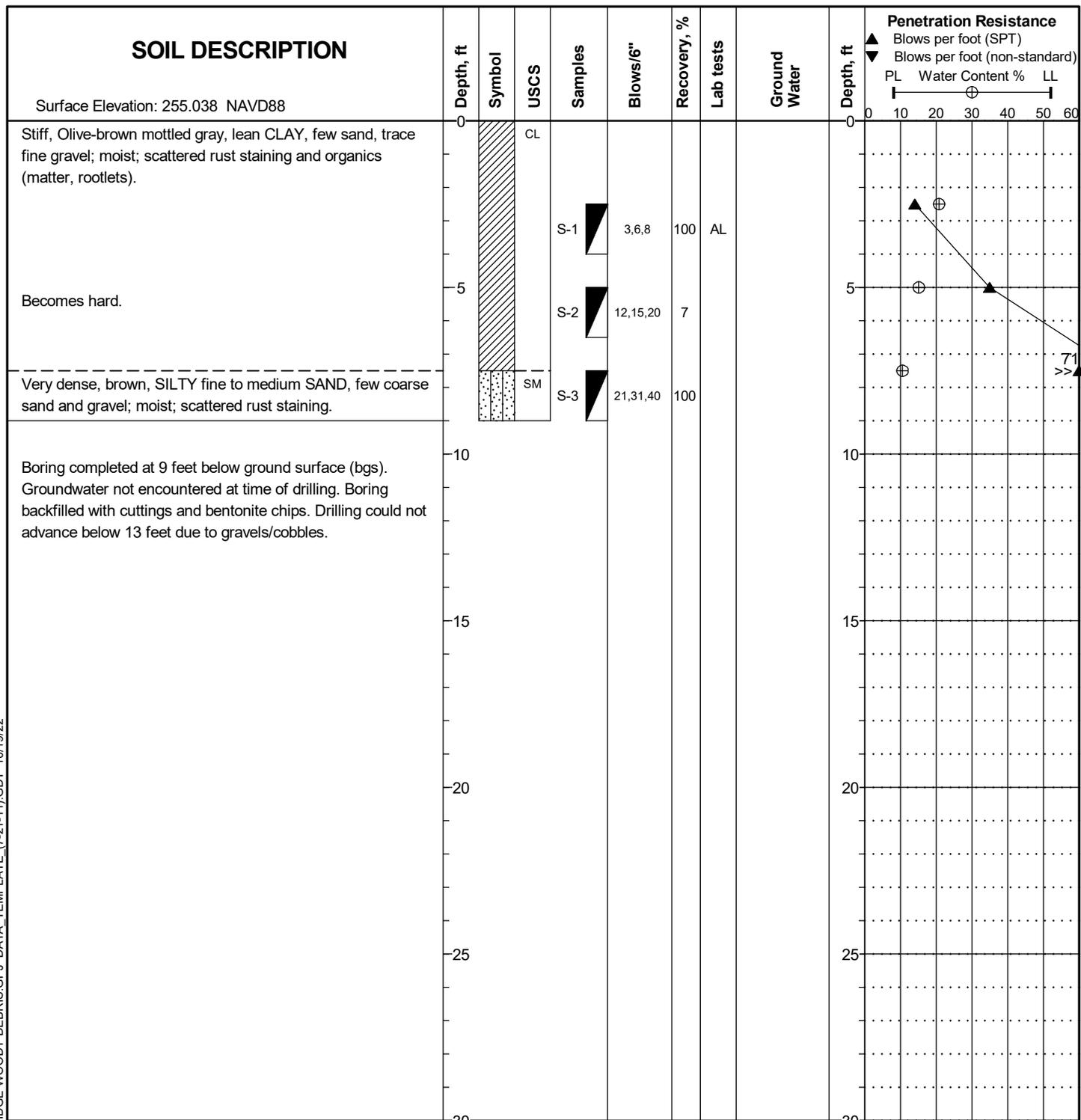
Approximate Location: 3 feet north and 5.5 feet east of survey marker 60102. (N: 188609.6978 E: 1290775.74)

**Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA**



LOG OF BORING B-314
C399315 FIGURE A-16

LOG OF BORING (2/1/11) LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA_TEMPLATE_(7-21-11).GDT 10/19/22



Date Completed: 4/15/2021
 Driller: Geologic Drill Partners
 Equipment: MT55 Track Rig
 Drilling Method: 4-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: 14 feet north and 12.5 west of survey stake 61228.
 (N: 187152.6913 E: 1290782.49)

**Taylor Creek Restoration: Dead Horse Canyon
 Seattle, WA**



LOG OF BORING B-315
 C399315 FIGURE A-17

APPENDIX B

GEOTECHNICAL LABORATORY TESTING PROGRAM

APPENDIX B

GEOTECHNICAL LABORATORY TESTING PROGRAM

SPU Geotechnical Engineering representatives completed laboratory tests on select soil samples collected during our field investigation. The laboratory tests were completed in general accordance with appropriate ASTM test methods. The test procedures and test results are discussed below. Additional laboratory tests were completed by our consultant, HWA GeoSciences Inc. (HWA). HWA's test procedures and test results are provided in this Appendix after the results of testing completed by SPU Geotechnical Engineering.

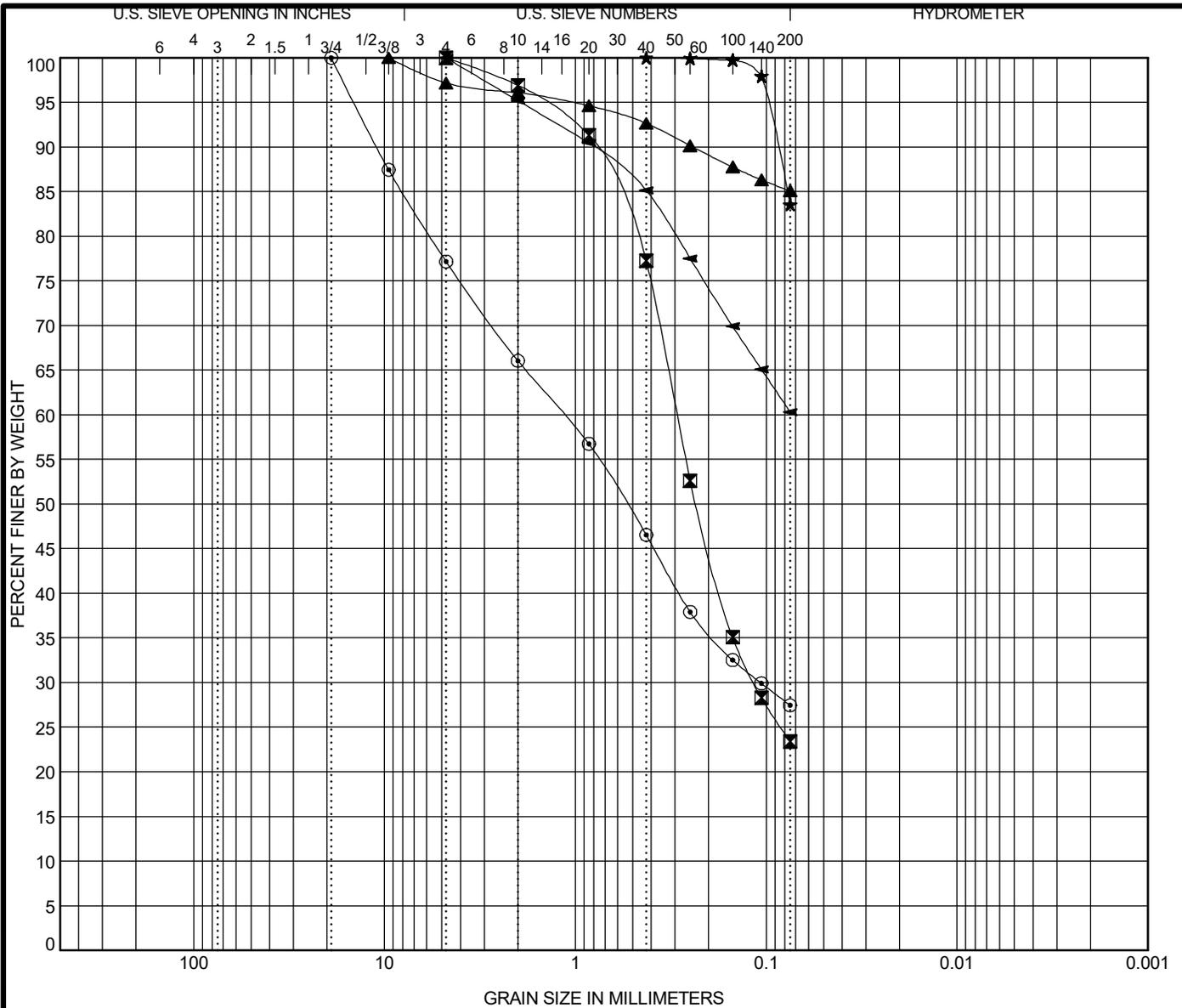
Natural Water Content

Natural water content determinations were made on select soil samples in general accordance with ASTM D-2216, Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass. Test results are graphically indicated at the appropriate sample depth on the summary logs in Appendix A.

Grain-size Distribution

The grain-size distribution of select samples was analyzed in general accordance with ASTM D422, Standard Test Method for Particle-Size Analysis of Soils. Results of grain size analyses are plotted on Figures B-1 and B-2 of this appendix. The soil samples tested for grain size distribution are indicated on the summary logs in Appendix A.

GSD.D90 LOWER TAYLOR CREEK LAKE RIDGE WOODY DEBRIS.GPJ DATA TEMPLATE (7-21-11).GDT 10/19/22



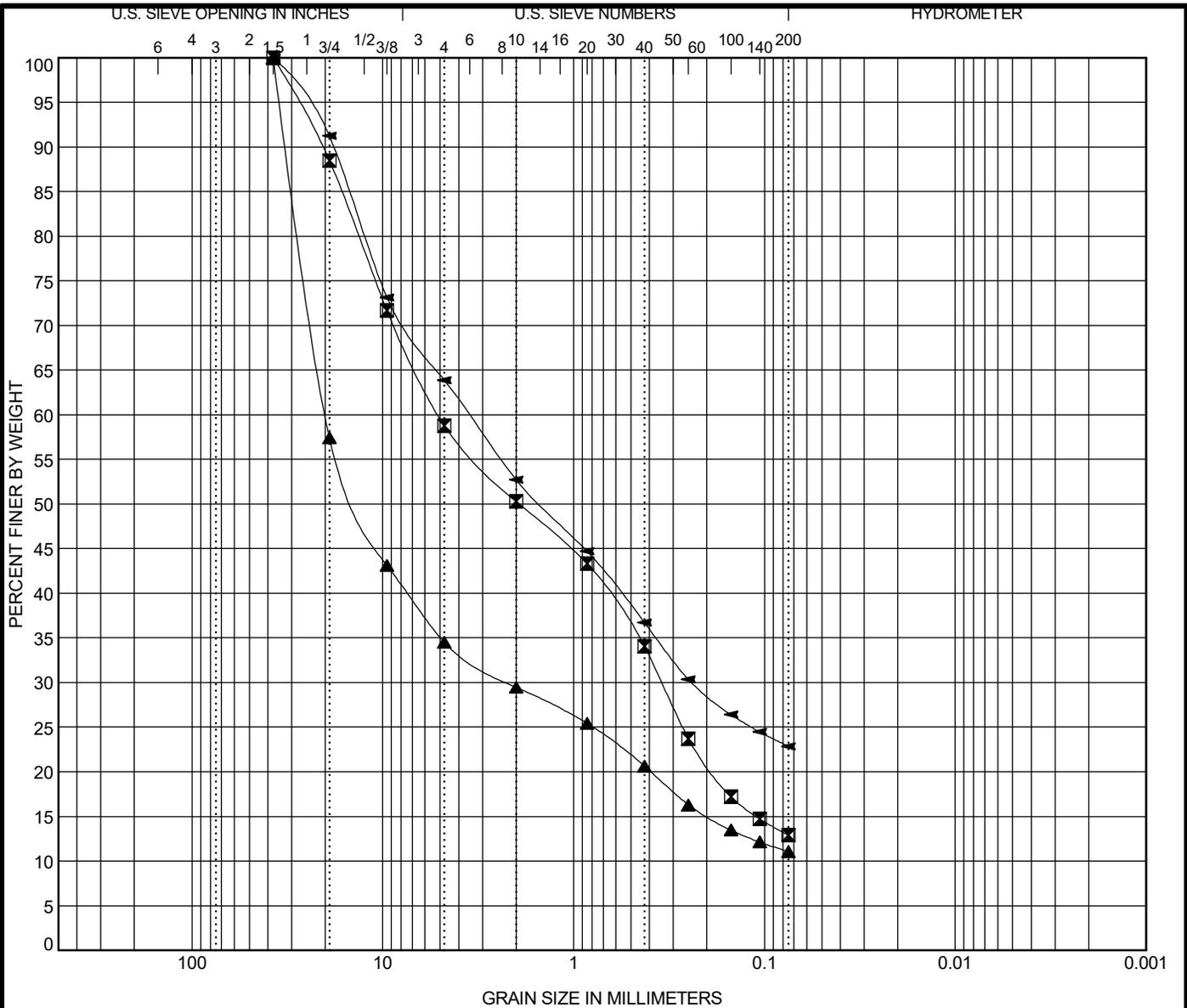
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Sample ID	Depth	Classification					LL	PL	PI	Cc	Cu
▲ B-301; S-1	2.5	Dark brown, fine SANDY SILT, few medium sand, trace coarse sand (ML)									
■ B-302; S-4	10.0	Gray, fine to medium SAND, trace coarse sand (SM)									
▲ B-303; S-1	2.5	Grayish brown, SILT, few fine sand, trace medium and coarse sand and fine gravel (ML)									
★ B-305; S-7	20.0	Brownish gray, SILT, few fine sand (ML)									
◎ B-307; S-1	2.5	Brown mottled gray, SILTY fine to medium SAND WITH GRAVEL, few coarse sand (SM)									
Sample ID	Depth	D90	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay		
▲ B-301; S-1	2.5	0.8				0.0	39.7	60.3			
■ B-302; S-4	10.0	0.8	0.29	0.12		0.0	76.6	23.4			
▲ B-303; S-1	2.5	0.24				2.8	12.1	85.1			
★ B-305; S-7	20.0	0.09				0.0	16.4	83.6			
◎ B-307; S-1	2.5	10.93	1.15	0.11		22.8	49.7	27.4			



GRAIN SIZE DISTRIBUTION (ASTM D422)
Taylor Creek Restoration: Dead Horse Canyon
Seattle, WA
 C399315 FIGURE: B-1

GSD.D90 LOWER TAYLOR CRK-LAKERIDGE WOODY DEBRIS.GPJ DATA TEMPLATE (7-21-1).GDT 10/19/22





May 27, 2021
HWA Project No. 2019-129-21 T2E

Seattle Public Utilities
700 5th Ave
P.O. Box 34018
Seattle, Washington 98124-4018

Attention: Mr. Aaron Clark, L.G.

Subject: **Materials Laboratory Report**
Atterberg Limits Testing
Lower Taylor Creek – Lakeridge Haul Road
Client Project No.: C399315

Dear Mr. Clark;

In accordance with your request, HWA GeoSciences Inc. (HWA) performed laboratory testing for the above referenced project. Herein we present the results of our laboratory analyses, which are summarized on the attached reports. The laboratory testing program was performed in general accordance with your instructions and appropriate ASTM Standards as outlined below.

SAMPLE DESCRIPTION: The subject samples were delivered to our laboratory on May 19, 2021 by the client. The samples were delivered in re-sealable plastic jars and were designated with exploration ID, sample number, and depth of sampling. The soil samples were classified using visual-manual methods. The descriptions may be found on the attached Summary of Material Properties, Figure 1.

LIQUID LIMIT, PLASTIC LIMIT, AND PLASTICITY INDEX OF SOILS (ATTERBERG LIMITS): The plasticity index of each specified sample was tested using method ASTM D4318, multi-point method. The results are reported on the attached Liquid Limit, Plastic Limit, and Plasticity Index Report, Figures 2 through 3.



CLOSURE: Experience has shown that test values on soil and other natural materials vary with each representative sample. As such, HWA has no knowledge as to the extent and quantity of material the tested samples may represent. HWA also makes no warranty as to how representative either the samples tested or the test results obtained are to actual field conditions. It is a well-established fact that sampling methods present varying degrees of disturbance that affect sample representativeness.

No copy should be made of this report except in its entirety.

We appreciate the opportunity to provide laboratory testing services on this project. Should you have any questions or comments, or if we may be of further service, please call.

HWA GEOSCIENCES INC.

A handwritten signature in blue ink, appearing to read "Kristin Nolan".

Kristin Nolan
Materials Laboratory Manager

A handwritten signature in blue ink, appearing to read "Steven E. Greene".

Steven E. Greene, L.G., L.E.G.
Principal Engineering Geologist
Vice President

Attachments:

Figure 1	Summary of Material Properties
Figures 2-3	Liquid Limit, Plastic Limit, and Plasticity Index Report

EXPLORATION DESIGNATION	TOP DEPTH (feet)	BOTTOM DEPTH (feet)	MOISTURE CONTENT (%)	ORGANIC CONTENT (%)	SPECIFIC GRAVITY	ATTERBERG LIMITS (%)			% GRAVEL	% SAND	% FINES	ASTM SOIL CLASSIFICATION	SAMPLE DESCRIPTION
						LL	PL	PI					
B-301,S-3	7.5	9.0	21.3			29	18	11				CL	Olive-brown, lean CLAY
B-302,S-2	5.0	6.5	23.8			33	22	11				CL	Dark olive-brown, lean CLAY with organics
B-304,S-3	7.5	9.0	28.4			32	25	7				ML	Olive-brown, sandy SILT
B-304,S-6	15.0	16.5	22.0			35	23	12				CL	Dark gray, lean CLAY
B-305,S-1	2.5	4.0	23.3			35	24	11				CL	Light olive-brown, lean CLAY with sand
B-305,S-8	25.0	26.5	29.2			24	21	3				ML	Gray, sandy SILT
B-306,S-4	10.0	11.5	15.6			32	17	15				CL	Grayish-brown, sandy lean CLAY
B-308B,S-1	5.0	6.5	27.9			49	26	23				CL	Olive-brown, lean CLAY
B-310,S-2	5.0	6.5	16.4			34	19	15				CL	Olive-brown, sandy lean CLAY with gravel
B-313,S-8	20.0	21.5	24.8			40	24	16				CL	Gray, lean CLAY
B-315,S-1	2.5	4.0	19.8			33	19	14				CL	Olive-brown, lean CLAY

Notes: 1. This table summarizes information presented elsewhere in the report and should be used in conjunction with the report test, other graphs and tables, and the exploration logs.
2. The soil classifications in this table are based on ASTM D2487 and D2488 as applicable.

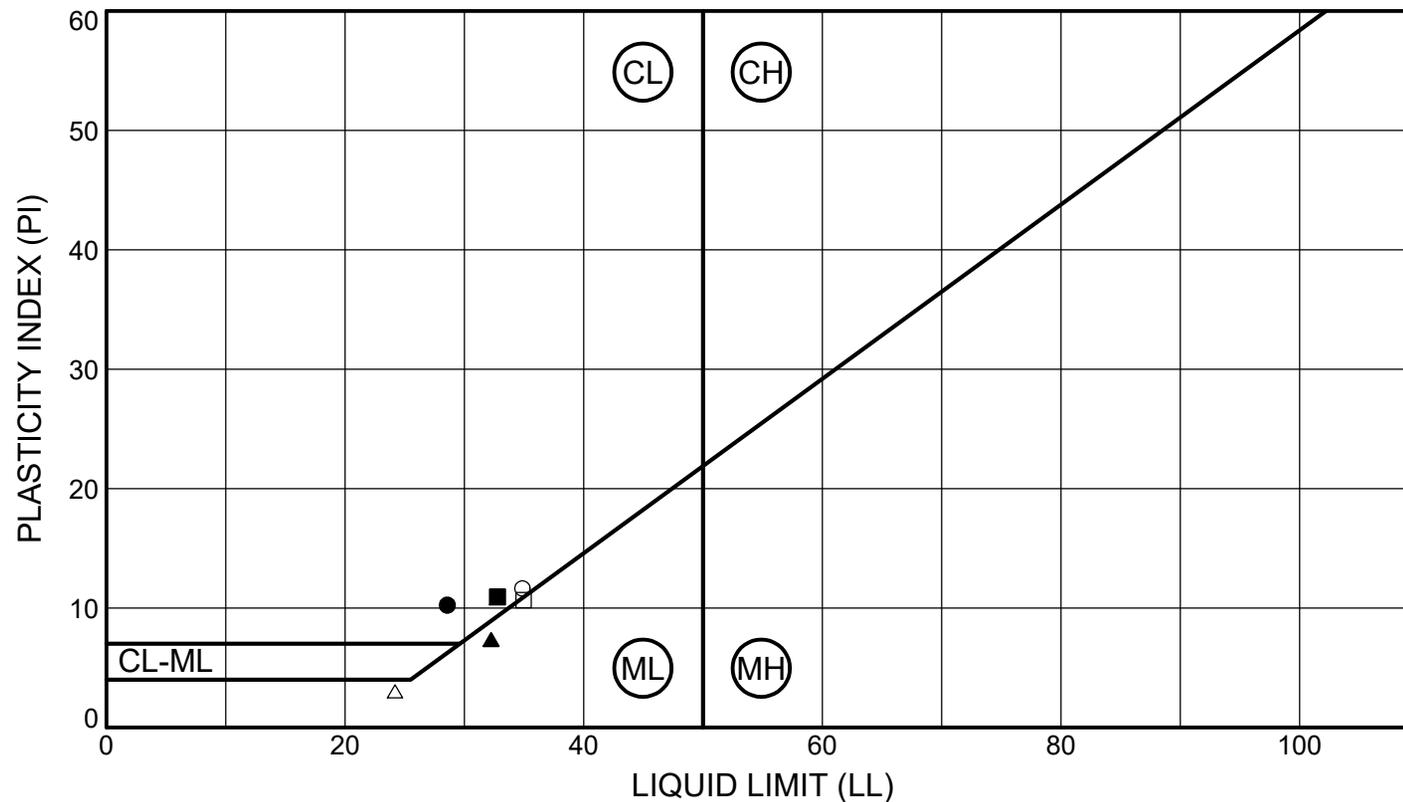


Seattle Public Utilities
Lower Taylor Creek - Lake Ridge Haul Road
Client Project No.: C399315

SUMMARY OF MATERIAL PROPERTIES

PAGE: 1 of 1

PROJECT NO.: 2019-129 T02E FIGURE: 1



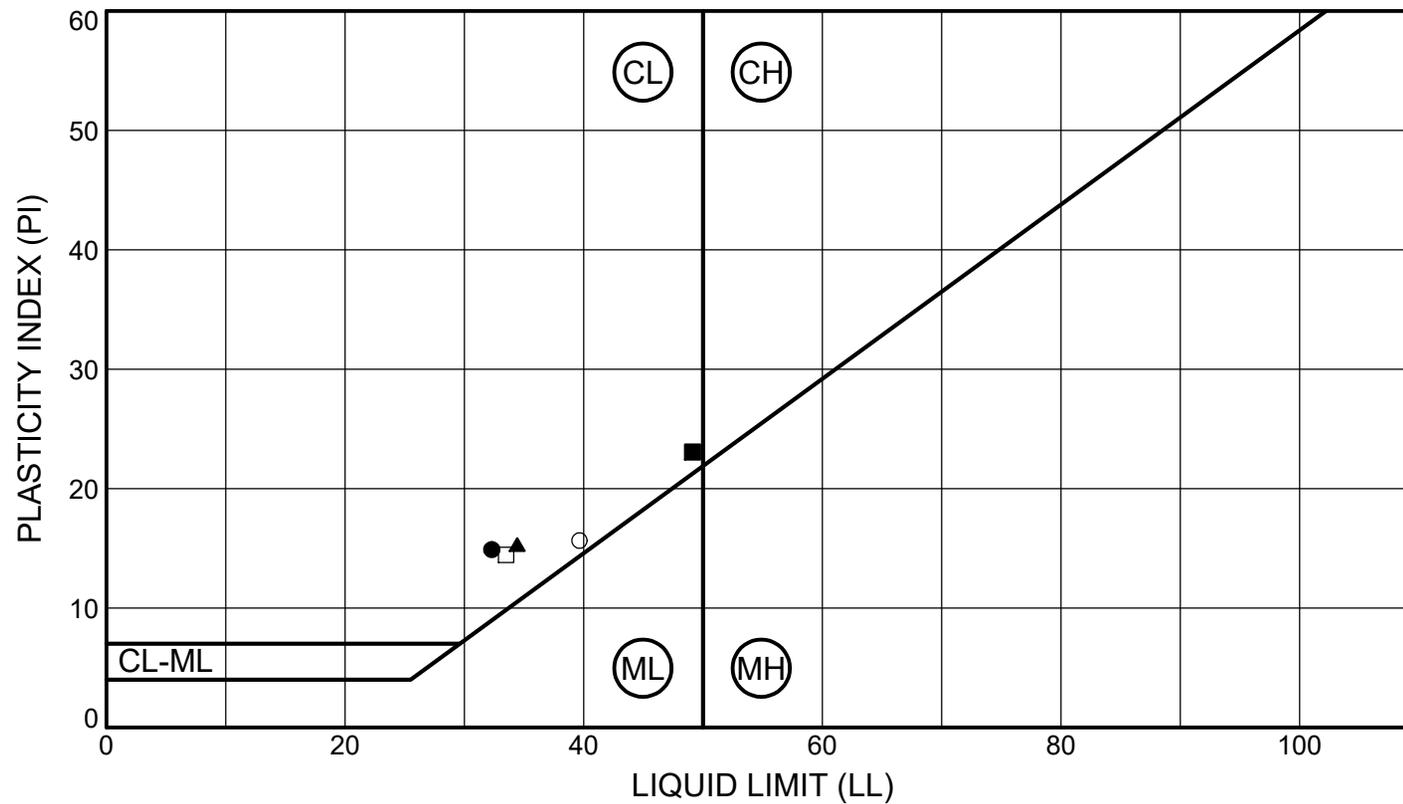
SYMBOL	SAMPLE		DEPTH (ft)	CLASSIFICATION	% MC	LL	PL	PI	% Fines
●	B-301	S-3	7.5 - 9.0	(CL) Olive-brown, lean CLAY	21	29	18	11	
■	B-302	S-2	5.0 - 6.5	(CL) Dark olive-brown, lean CLAY with organics	24	33	22	11	
▲	B-304	S-3	7.5 - 9.0	(ML) Olive-brown, sandy SILT	28	32	25	7	
○	B-304	S-6	15.0 - 16.5	(CL) Dark gray, lean CLAY	22	35	23	12	
□	B-305	S-1	2.5 - 4.0	(CL) Light olive-brown, lean CLAY with sand	23	35	24	11	
△	B-305	S-8	25.0 - 26.5	(ML) Gray, sandy SILT	29	24	21	3	



Seattle Public Utilities
 Lower Taylor Creek - Lake Ridge Haul Road
 Client Project No.: C399315

LIQUID LIMIT, PLASTIC LIMIT AND
 PLASTICITY INDEX OF SOILS
 METHOD ASTM D4318

PROJECT NO.: 2019-129 T02E FIGURE: 2



SYMBOL	SAMPLE		DEPTH (ft)	CLASSIFICATION	% MC	LL	PL	PI	% Fines
●	B-306	S-4	10.0 - 11.5	(CL) Grayish-brown, sandy lean CLAY	16	32	17	15	
■	B-308B	S-1	5.0 - 6.5	(CL) Olive-brown, lean CLAY	28	49	26	23	
▲	B-310	S-2	5.0 - 6.5	(CL) Olive-brown, sandy lean CLAY with gravel	16	34	19	15	
○	B-313	S-8	20.0 - 21.5	(CL) Gray, lean CLAY	25	40	24	16	
□	B-315	S-1	2.5 - 4.0	(CL) Olive-brown, lean CLAY	20	33	19	14	



Seattle Public Utilities
 Lower Taylor Creek - Lake Ridge Haul Road
 Client Project No.: C399315

LIQUID LIMIT, PLASTIC LIMIT AND
 PLASTICITY INDEX OF SOILS
 METHOD ASTM D4318

PROJECT NO.: 2019-129 T02E FIGURE: 3

APPENDIX C

HISTORICAL EXPLORATIONS

APPENDIX C

HISTORICAL EXPLORATIONS

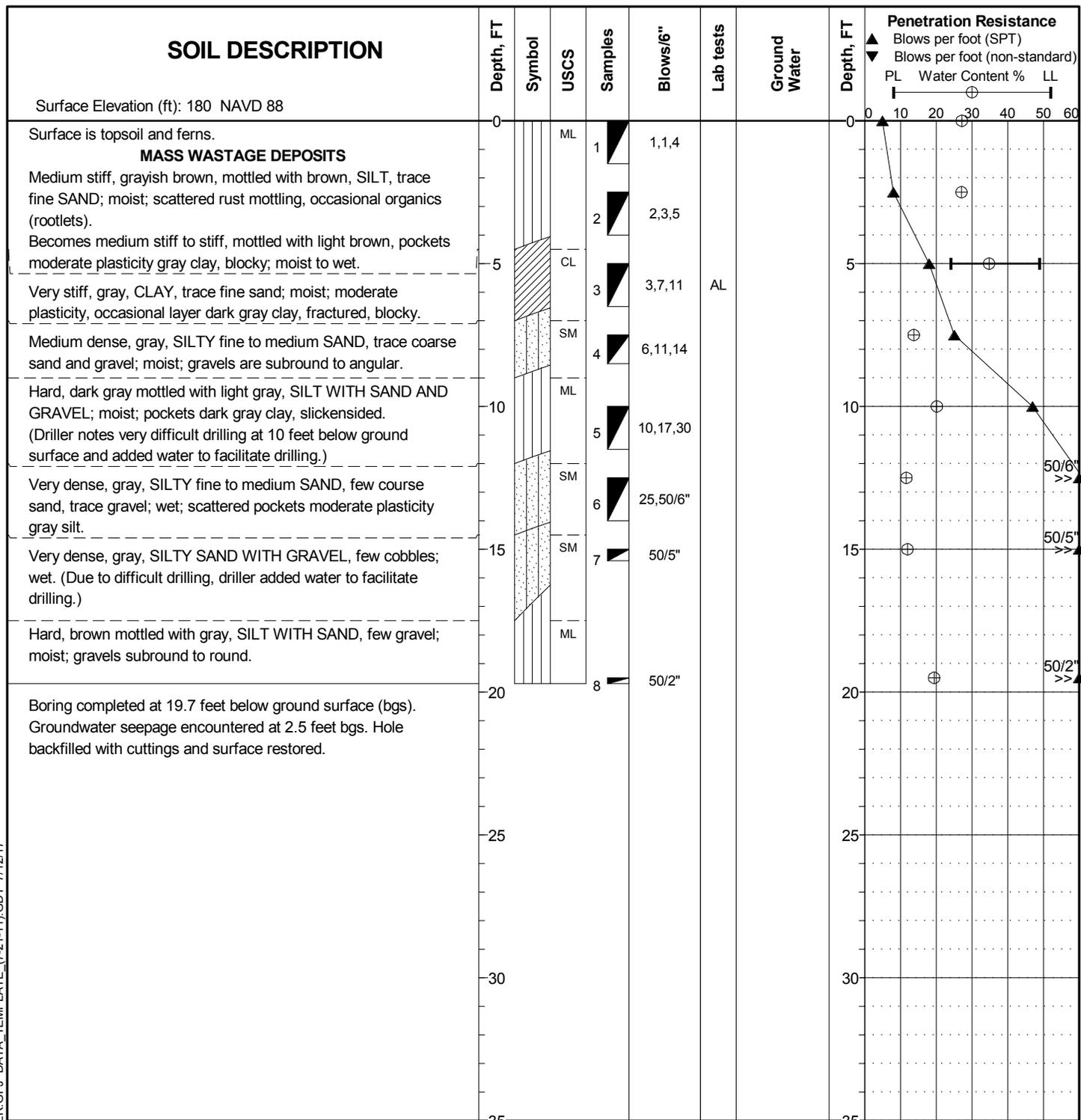
In addition to the explorations and laboratory test results presented in Appendices A and B, respectively, historical explorations completed by others or completed by us for other projects were reviewed to gain an understanding of the subsurface conditions at the site. Figure 1 shows the approximate locations of the historical explorations in the vicinity of the project site.

The following historical exploration logs and laboratory test results were relevant in our analysis, in terms of depth and location, and are presented in this appendix.

- Seattle Public Utilities Geotechnical Engineering
 - Boring B-101, completed May 31, 2017
 - Hand Auger HA-101, completed May 31, 2017
 - Hand Auger HA-102, completed May 31, 2017
 - Atterberg Limits Results
- Seattle Public Utilities Geotechnical Engineering
 - Boring B-101, completed September 25, 2014
 - Boring B-102, completed September 25, 2014
 - Boring B-103, completed September 25, 2014
 - Atterberg Limits Results
- Golder Associates
 - Boring B-1, completed September 21, 1994
 - Boring B-2, completed September 25, 1994
 - Boring B-3, completed September 26, 1994
 - Boring B-4, completion date not reported
 - Boring B-5, completed September 21, 1994
 - Atterberg Limits Results (2 pages)
 - Grain Size Determination Results (4 pages)

These logs and laboratory tests are presented for reference only.

LOG OF BORING - (7/20/2009) LAKERIDGE PARK SEWER.GPJ DATA_TEMPLATE_(7-21-11).GDT 7/12/17



Date Completed: 5/31/2017
 Driller: CN Drilling
 Equipment: Acker
 Drilling Method: 2-1/4 inch ID HSA
 Hammer System: Cathead

Approximate Location: In Lakeridge Park, approximately 9 feet west of the trail centerline and approximately 40 feet south of the first footbridge from the north trail entrance. (N: 187333 E: 1290937)

LAKERIDGE SEWER PROJECT
Seattle, Washington

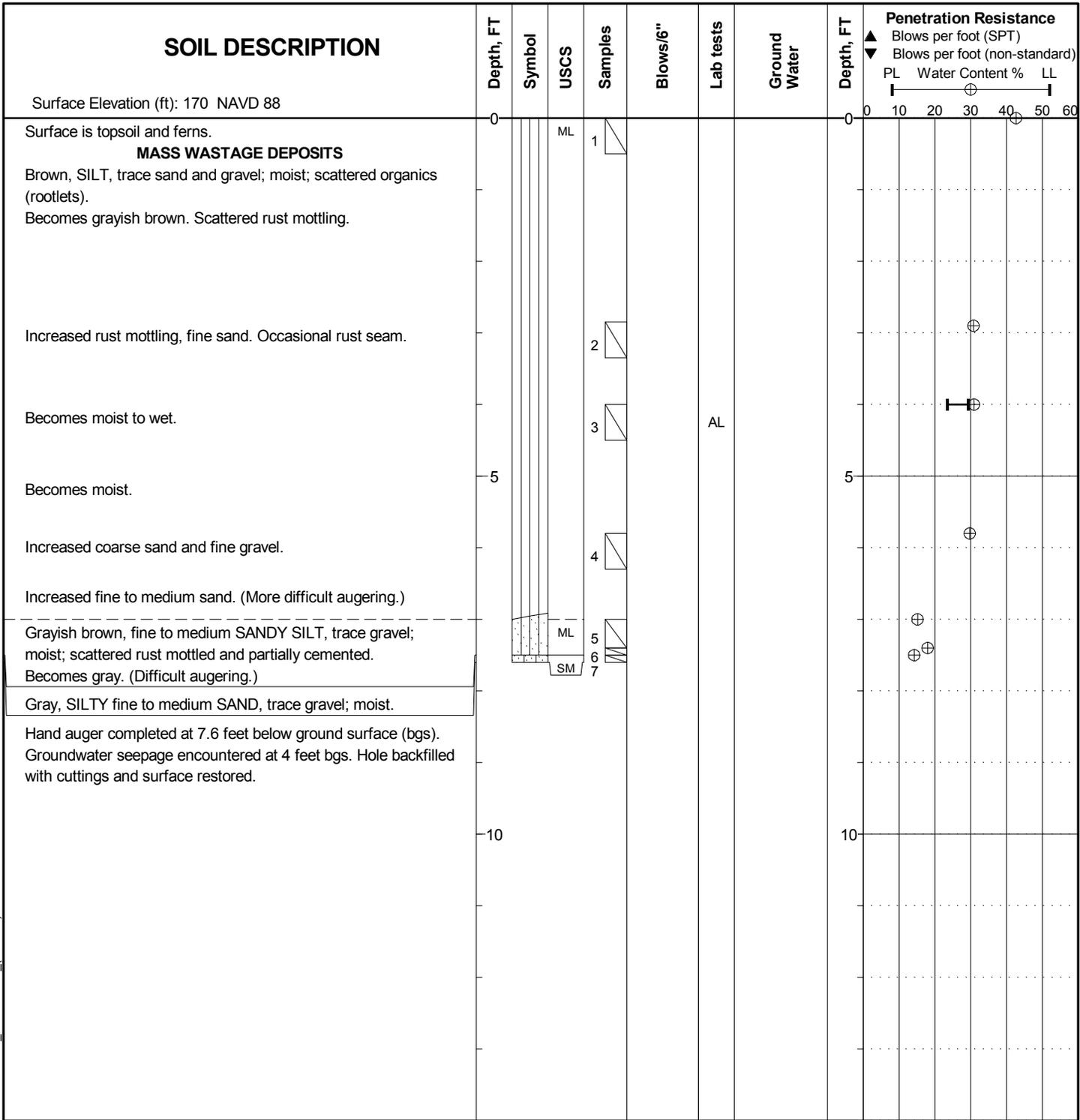
LOG OF BORING B-101



E314059 Z 110 MG3

FIGURE A-2

LOG OF BORING - (7/20/2009) LAKERIDGE PARK SEWER.GPJ DATA_TEMPLATE_(7-21-11).GDT 7/12/17



Date Completed: 5/31/2017
 Driller: N/A
 Equipment: Hand Auger
 Drilling Method: 2 3/4-inch ID Hand Auger
 Hammer System: N/A

Approximate Location: In Lakeridge Park, approximately 19 feet east of the trail centerline and approximately 50 feet south of the first footbridge from the north trail entrance. (N: 187348 E: 1290962)

LAKERIDGE SEWER PROJECT
Seattle, Washington

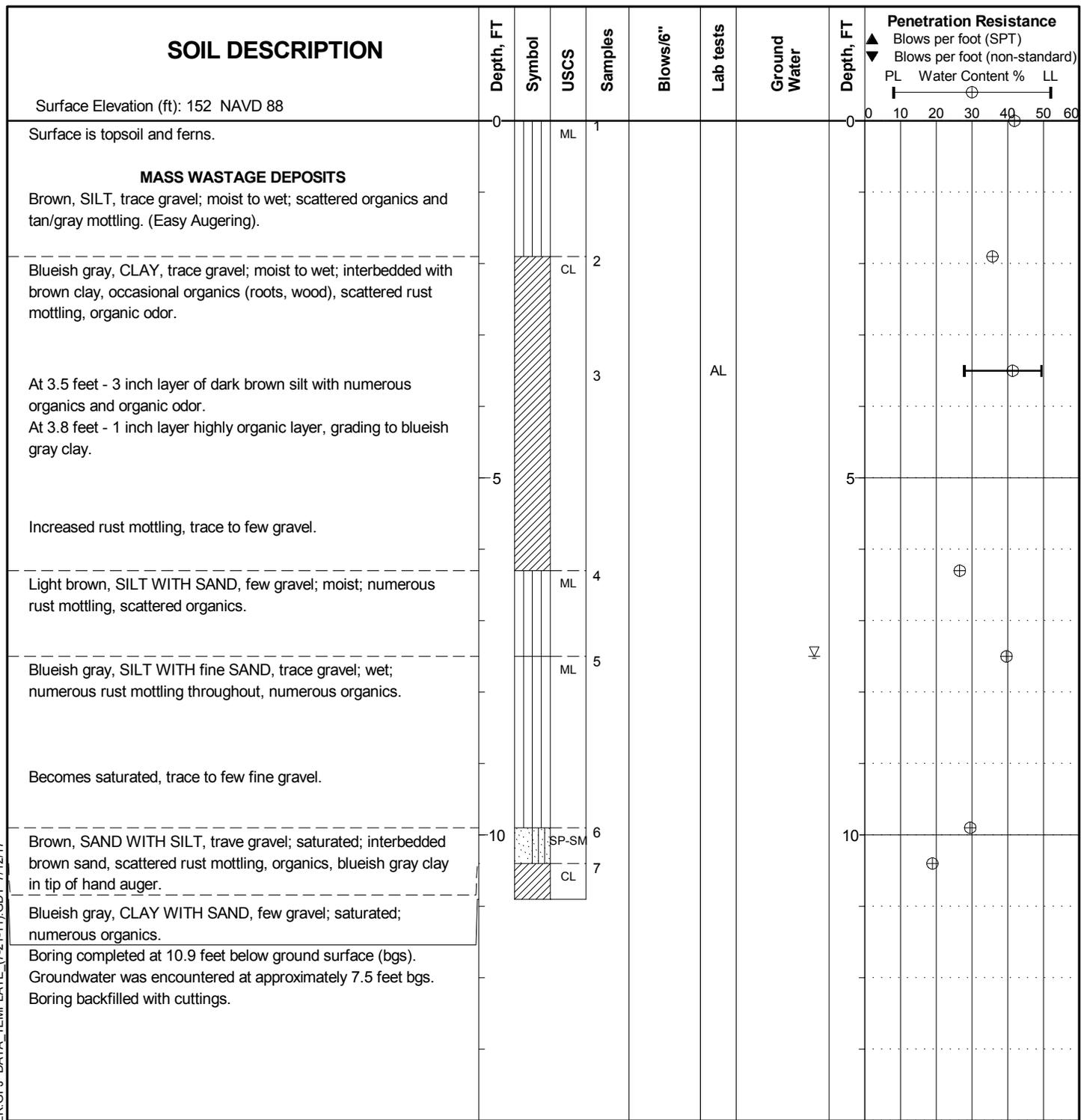
LOG OF BORING HA-101



E314059 Z 110 MG3

FIGURE A-3

LOG OF BORING - (7/20/2009) LAKERIDGE PARK SEWER.GPJ DATA_TEMPLATE_(7-21-11).GDT 7/12/17



Date Completed: 5/31/2017
 Driller: N/A
 Equipment: Hand Auger
 Drilling Method: 2 3/4-inch ID Hand Auger
 Hammer System: N/A

Approximate Location: In Lakeridge Park, approximately 45 feet east of the trail centerline and approximately 40 feet south of the first footbridge from the north trail entrance. (N: 187372 E: 1290978)

LAKERIDGE SEWER PROJECT
Seattle, Washington

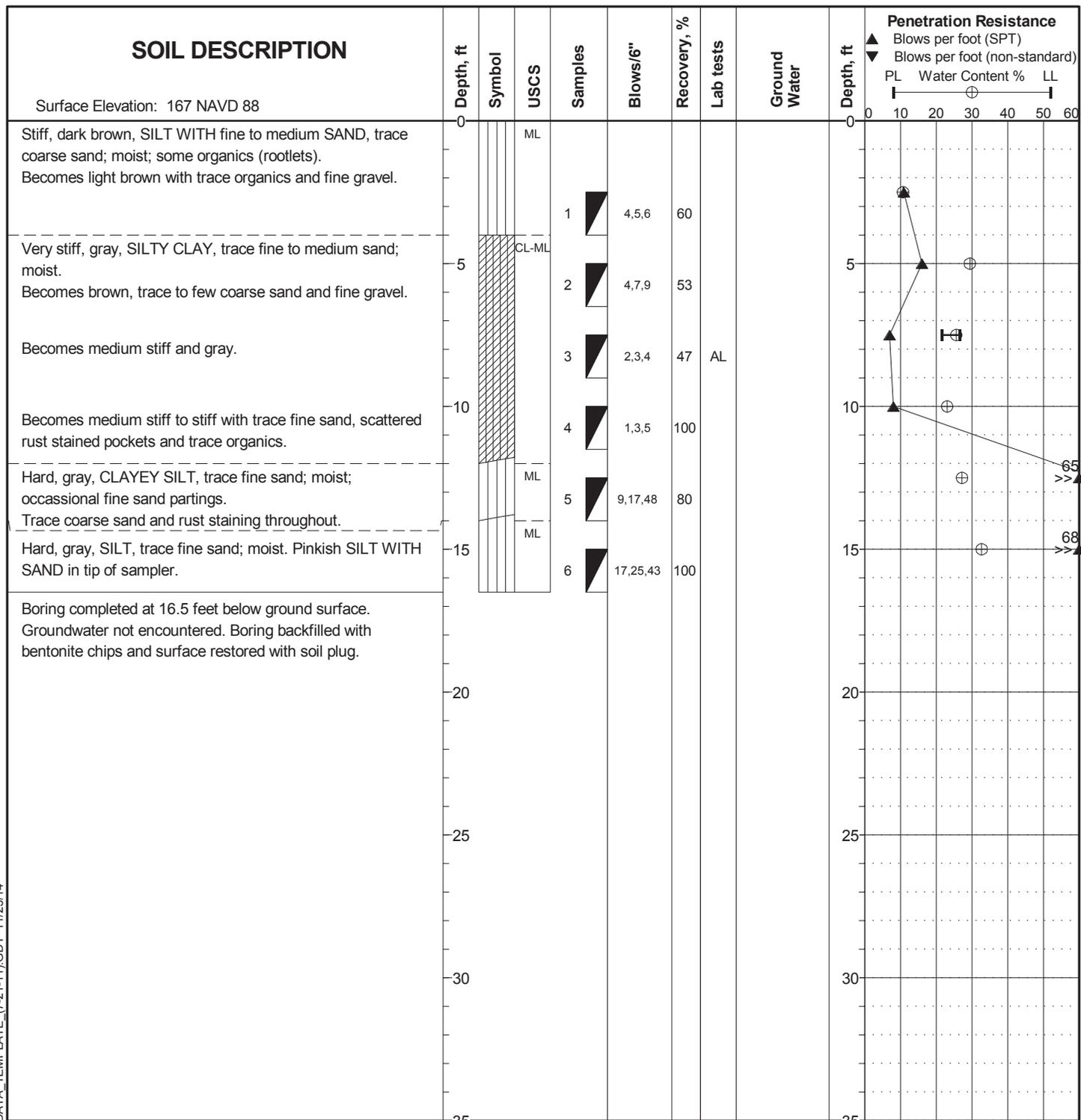
LOG OF BORING HA-102



E314059 Z 110 MG3

FIGURE A-4

LOG OF BORING (2/1/11) DEADHORSE CANYON.GPJ_DATA_TEMPLATE_(7-21-11).GDT 11/25/14



Date Completed: 9/25/2014
 Driller: CN Drilling
 Equipment: Acker
 Drilling Method: 4-1/2 inch OD HSA
 Hammer System: Cathead

Approximate Location: 100 feet N of large bridge crossing the bridge, 52 feet E of center of trail and 23 feet W of creek. (N: 187235.8 E: 1291095)

**Deadhorse Canyon Sewer
Seattle, Washington**

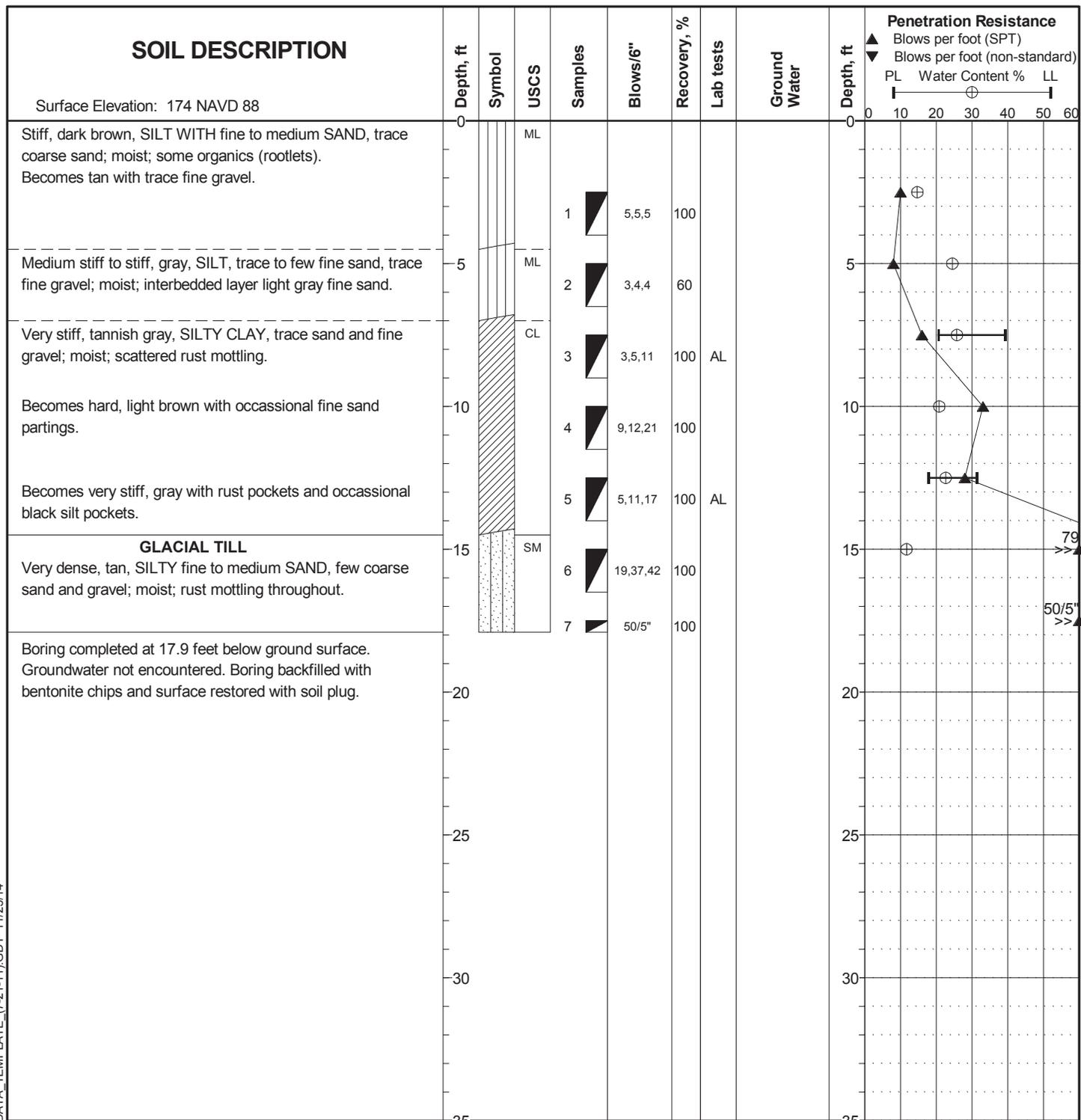
LOG OF BORING B-101



E314059

FIGURE A-2

LOG OF BORING (2/1/11) DEADHORSE CANYON.GPJ_DATA_TEMPLATE_(7-21-11).GDT 11/25/14



Date Completed: 9/25/2014
 Driller: CN Drilling
 Equipment: Acker
 Drilling Method: 4-1/2 inch OD HSA
 Hammer System: Cathead

Approximate Location: 100 feet N of large bridge crossing the bridge, 21 feet W of B-101 and 30 feet E of center of trail. (N: 187229.3 E: 1291080.8)

**Deadhorse Canyon Sewer
Seattle, Washington**

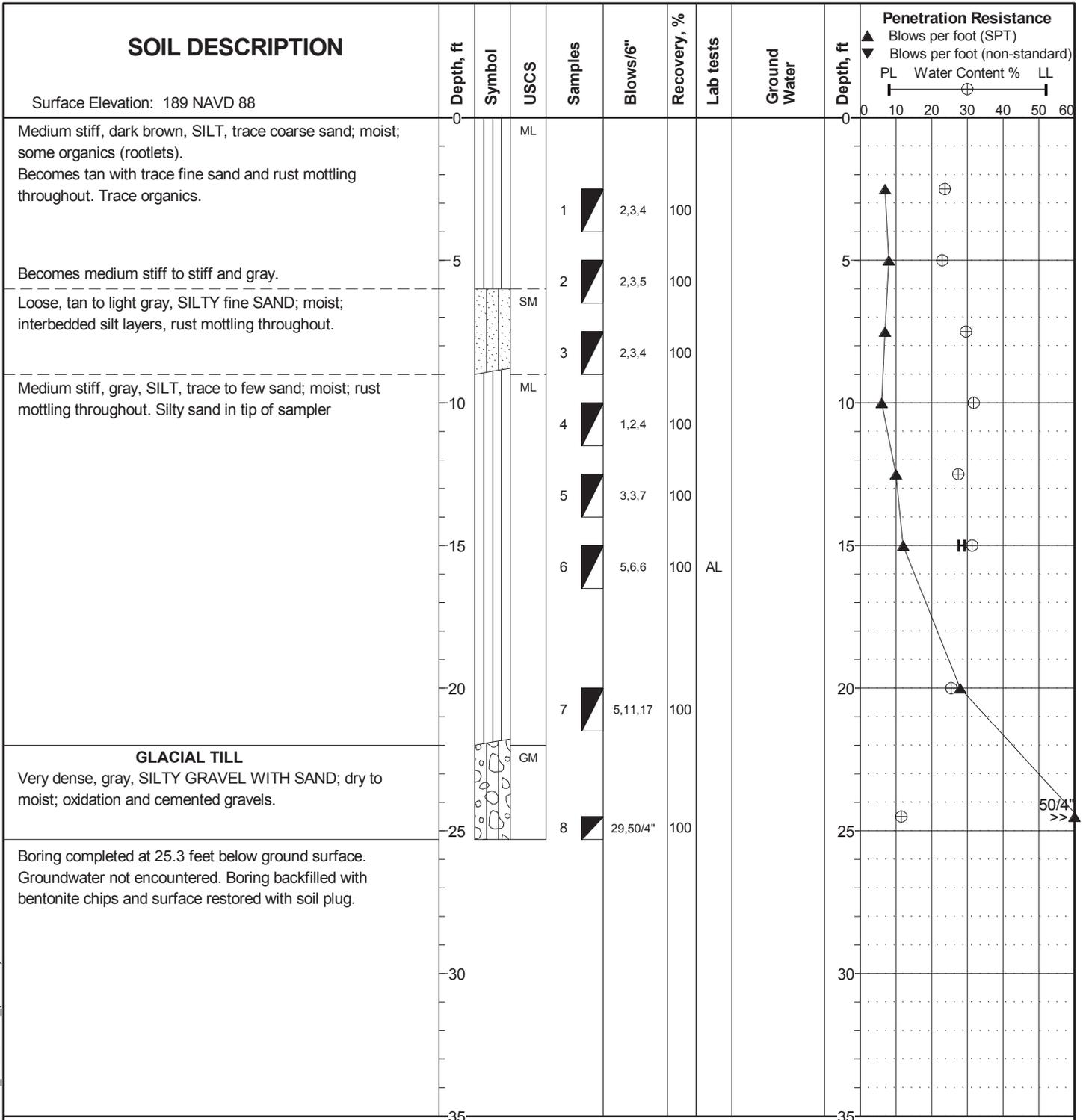
LOG OF BORING B-102



E314059

FIGURE A-3

LOG OF BORING (2/1/11) DEADHORSE CANYON.GPJ DATA_TEMPLATE_(7-21-11).GDT 11/25/14



Date Completed: 9/25/2014
 Driller: CN Drilling
 Equipment: Acker
 Drilling Method: 4-1/2 inch OD HSA
 Hammer System: Cathead

Approximate Location: 100 feet N of large bridge crossing the creek, 21.5 feet W of center of trail and 54.5 feet from B-102. Approximately 35 feet E of large cedar. (N: 187205.6 E: 1291030.4)

**Deadhorse Canyon Sewer
 Seattle, Washington**

LOG OF BORING B-103

E314059

FIGURE A-4



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-1

SHEET 1 OF 2

PROJECT NUMBER: 943-1626

BORING LOCATION: Crestwood Drive S.

DATUM: MSL

BORING DATE: 9/21/94

DEPTH FEET	BORING METHOD	SOIL PROFILE			SAMPLES				PENETRATION RESISTANCE		PIEZOMETER GRAPHIC			
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/ATT		BLOWS/FT	WATER CONTENT, PERCENT Wp — W — Wl	WATER LEVEL
0	4-inch I.D. HSA	Very dense, olive gray (SY 4/1) to olive black (SY 2/1), Silty fine to coarse SAND, trace to little gravel, trace cobble (TILL)		275.0										
10				SM	1	SS	50/5*	50+	4/5*					
20														
30				SM	2	SS	50/5*	50+	3/5*					
40														
50														
60														
65		Drilling ceases 64-65 feet												
70		Very dense, medium grey (N5), non-stratified CLAYEY SILT, trace fine sand												

Continued on page 2

DRILL RIG: B-61

DRILLING CONTRACTOR: RAM, Inc.

DRILLER: George Jewel

LOGGED: Richard Luerk

CHECKED: Jim Johnson

DATE: 10/31/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-1

SHEET 2 OF 2

PROJECT NUMBER: 943-1626

BORING LOCATION: Crestwood Drive S.

DATUM: MSL

BORING DATE: 9/22/94

DEPTH FEET	BORING METHOD	SOIL PROFILE				SAMPLES				PENETRATION RESISTANCE BLOWS/FT		PIEZOMETER GRAPHIC
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/ATT	WATER CONTENT, PERCENT Wp — W — Wi	
80	Mud Rotary	Very dense, medium gray (N5), non-stratified CLAYEY SILT, trace fine sand	CL		205.0 187.5 77.5	5	SS	100/6	50+	5/6	21.9%	
		Very dense, medium gray (N5), silty coarse to fine SAND, little gravel (TILL)	SM		185.0 90.0	6	SS	50/5*, 50/1*	50+	4/6		
		Very dense, medium gray (N5), fine to coarse GRAVEL, little to some silty sand	GW		169.0 108.0	7	SS	50/6*, 50/1*	50+	5/7		
		Very dense, brownish black (5YR 2/1), medium fibrous PEAT, some fine to medium sand	Pt		164.0 111.0	8	SS	4.3, 50/2*	50+	7/8		
110				159.0								
120		Boring terminated @ 116.0' Piezometer installed full depth.		118.0								
130												
140												
150												

DRILL RIG: 8-61

DRILLING CONTRACTOR: RAM, Inc.

DRILLER: George Jewel

LOGGED: Richard Luark

CHECKED: Jim Johnson

DATE: 11/1/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-2

SHEET 1 OF 3

DATUM: MSL

PROJECT NUMBER: 943-1626

BORING LOCATION: Lake Ridge Drive & Rustic Road

BORING DATE: 9/24-25/94

DEPTH FEET	BORING METHOD	SOIL PROFILE			SAMPLES				PENETRATION RESISTANCE		PIEZOMETER GRAPHIC	
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/ATT		BLOWS/FT
0	4-inch I.D. HSA	Very dense, moderate yellow brown (10YR 5/4), fine to medium SAND, some silt, little fine gravel (WEATHERED TILL)			285.0							
15.0		SM		1	SS	100/5*	50+	3/5*				
235.0		ML	Very dense, medium gray (N5), non-stratified SILT, some fine to coarse gravel, little sand (TILL)			235.0	2	SS	50/4*	50+	3/4*	
30.0												
37-40		Sand lense from 37-40' (based on drilling action)										
45.0	Mud Rotary	No recovery @S-3 gravel & cobbles			220.0	3	SS	22-38-50	88	0/18*		
45.0												
205.0		CL	Very dense, dusky yellow green (5GY 5/2), non-stratified SILT and SILT CLAY, little sand, trace gravel (TILL)			205.0	4	SS	18-45-50/5*	50+	17/17*	26.8%
60.0												
70		Hard, medium gray (N5), SILTY CLAY to CLAY, trace sand, trace gravel (LAGUSTRINE DEPOSIT)										

Continued on page 2

DRILL RIG: B-61

DRILLING CONTRACTOR: RAM, Inc.

DRILLER: Dan Troxa

LOGGED: Scott Huisell

CHECKED: Jim Johnson

DATE: 11/1/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-2

SHEET 2 OF 3

DATUM: MSL

PROJECT NUMBER: 943-1626

BORING LOCATION: Lake Ridge Drive & Rustic Road

BORING DATE: 9/24-25/94

DEPTH FEET	BORING METHOD	SOIL PROFILE			SAMPLES				PENETRATION RESISTANCE BLOWS/FT			PIEZOMETER GRAPHIC						
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/ATT	10		20	30	40	50	WATER LEVEL	
75.0 - 190.0	Mud Rotary	Hard, medium gray (N5), SILTY CLAY to CLAY, trace sand, trace gravel (LACUSTRINE DEPOSIT)	CL		190.0 75.0	5	SS	50/6"-50/4"	50+	6/10"						22%	30%	47%
90.0 - 175.0				CH		175.0 90.0	8	SS	21-50/5.5"	50+	5/11.5"							
105.0 - 160.0			No recovery on S-7	--		160.0 105.0	7	SS	50/3.5"	50+	0/3.5"							
120.0 - 145.0			Very dense, light olive gray (5Y 6/1) to very light gray (N8), fine to medium SAND, trace silt (weakly stratified)	SP		145.0 120.0	8	SS	50/5"	50+	5/5"							
130.0 - 135.0			Hard, grayish black (N2) stratified, CLAYEY ORGANIC SILT	OH		130.0 135.0	9	SS	50/6"-50/5.5"	50+	11/11.5"							
140.0 - 145.0			Hard, olive gray (5Y 4/1) stratified to non-stratified, CLAYEY SILT becoming CLAY with depth, trace fine sand, little organic silt in sample at 145 feet	MH		125.0 140.0	10	SS	50/5"-50/2"	50+	11/11.5"							
150.0 - 120.0			MH		120.0 145.0	11	SS	50/3"-50/1"	50+	11/11.5"								

DRILL RIG: B-61

LOGGED: Richard Luark

DRILLING CONTRACTOR: RAM, Inc.

CHECKED: Jim Johnson

DRILLER: George Jewel

DATE: 11/1/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-2

SHEET 3 OF 3

PROJECT NUMBER: 943-1626

BORING LOCATION: Lake Ridge & Rustic Road

DATUM: MSL

BORING DATE: 9/24-25/94

DEPTH FEET	BORING METHOD	SOIL PROFILE				SAMPLES				PENETRATION RESISTANCE BLOWS/FT ■		PIEZOMETER GRAPHIC													
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/INT	10		20	30	40	50	WATER CONTENT, PERCENT Wp ← W → Wl	WATER LEVEL							
150	Mud Rotary	Hard, olive gray (5Y 4/1), stratified to non-stratified, CLAYEY SILT becoming CLAY with depth, trace fine sand, little clayey organic silt in sample at 145 feet	-		115.0	12	SS	50/1*-5/0.5	50+	0/1.5"															
			150.0																						
			CL		110.0						13	SS	50/0.5-50/0.5-50/0.5	50+	25/1.5"										
			155.0																						
160			CH		105.0	14	SS	50/5*-50/4*	50+	5/9"															
				100.0																					
		Boring terminated @160.75 feet Backfilled with bentonite to surface			160.75																				
170																									
180																									
190																									
200																									
210																									
220																									

DRILL RIG: B-61

DRILLING CONTRACTOR: RAM, Inc.

DRILLER: Dan Troxa

LOGGED: Scott Hutsell

CHECKED: Jim Johnson

DATE: 10/31/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-3

SHEET 1 OF 3

PROJECT NUMBER: 943-1626

BORING LOCATION: Rustic Rd. & 108th St.

DATUM: MSL

BORING DATE: 9/26/94

DEPTH FEET	BORING METHOD	SOIL PROFILE				SAMPLES				PENETRATION RESISTANCE BLOWS/FT ■		PIEZOMETER GRAPHIC
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/ATT	WATER CONTENT PERCENT Wp — W — Wi	
0		Very dense, light olive gray (5Y 5/2), SILT, some fine to medium sand, trace gravel (TLL)			240.0							
10			SM		227.0 19.0	1	SS	16-29-42	50+	18/18		
20			Very dense, dark yellowish brown (10YR 4/2), fine to medium SAND, trace silt, trace gravel (ADVANCE OUTWASH)			212.0						
30				SP		28.0	2	SS	32-40-50/5"	50+	17/17	
40					220.0							
50			SP		45.0	3	SS	15-36-46	50+	18/18		
60					182.0							
70		Hard, medium gray (N5), SILTY CLAY, trace fine sand (PROGLACIAL LACUSTRINE CLAY)	SP		58.0	4	SS	50/4"-50/2.5"	50+	4/6.5"		

Continued on page 2

DRILL RIG: B-61

DRILLING CONTRACTOR: RAM, Inc.

DRILLER: Mike Jordan

LOGGED: Scott Hutsell

CHECKED: Jim Johnson

DATE: 11/1/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-3

SHEET 2 OF 3

PROJECT NUMBER: 943-1626

BORING LOCATION: Rustic Rd. & 108th St.

DATUM: MSL

BORING DATE: 9/26/94

DEPTH FEET	BORING METHOD	SOIL PROFILE				SAMPLES				PENETRATION RESISTANCE		PIEZOMETER GRAPHIC	
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/ATT	BLOWS/FT		WATER CONTENT PERCENT
											10 20 30 40 50	Wp W WI	WATER LEVEL
80		Hard, medium gray (N5), SILTY CLAY, trace fine sand (PROGLACIAL LACUSTRINE CLAY)	CL		165.0 75.0	5	SS	50/5*	50+	4/5*			
90			CL		150.0 90.0	6	SS	50/3*-50/2*	50+	4/5*			
110		Sample S-7 becomes CLAYEY SILT, little organics (wood chips)	MH		135.0 105.0	7	SS	50/2*-50/1*	50+	2/3			
120		Hard, brownish black (5YR 2/1), fibrous PEAT and ORGANIC CLAYEY SILT	Pt		120.0 120.0	8	SS	100/3*	50+	3/3			
130		Hard, light olive gray (5Y 6/1), laminated CLAY (PROGLACIAL LACUSTRINE CLAY)	CH		105.0 135.0	9	SS	100/3*	50+	3/3			
140		1-1/4" gravel jammed in sample shoe, no recovery	-		95.0 145.0	10	SS	150/1*	50+	6/1			
150		Continued on page 3											

DRILL RIG: B-61

DRILLING CONTRACTOR: RAM, Inc.

DRILLER: Mike Jordan

LOGGED: Scott Hutsell

CHECKED: Jim Johnson

DATE: 11/1/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-3

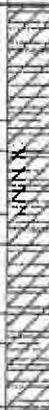
SHEET 3 OF 3

PROJECT NUMBER: 943-1626

BORING LOCATION: Rustic Rd. & 108th St.

DATUM: MSL

BORING DATE: 9/26/94

DEPTH FEET	BORING METHOD	SOIL PROFILE			SAMPLES				PENETRATION RESISTANCE BLOWS/FT				PIEZOMETER GRAPHIC WATER LEVEL								
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REC/INT	10	20		30	40	50					
150		Very dense, medium dark gray (N4), SILTY CLAY and fine to medium SAND, trace gravel (TILL)	CL		85.0	11	SS	100/2"	50+	2/2											
	155.0																				
160					80.0	12	SS	100/2"	50+	2/2											
	160.0																				
			CL		75.0	13	SS	100/2"	50+	2/2											
					165.0																
170			CL		70.0	14	SS	100/2"	50+	2/2											
					170.0																
		Boring terminated @ 170.2 feet Backfilled with bentonite to surface			170.2																
180																					
190																					
200																					
210																					
220																					

DRILL RIG: B-61

DRILLING CONTRACTOR: RAM, inc.

DRILLER: Mike Jordan

LOGGED: Scott Hutsell

CHECKED: Jim Johnson

DATE: 11/1/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-4

SHEET 1 OF 2

PROJECT NUMBER: 943-1626

BORING LOCATION: Bangor & Waters Street

DATUM: MSL

BORING DATE:

DEPTH, FEET	BORING METHOD	SOIL PROFILE			SAMPLES				PENETRATION RESISTANCE BLOWS/FT		PIEZOMETER GRAPHIC	
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV. DEPTH	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	RECI/ATT		WATER CONTENT, PERCENT Wp — W — WI
0	4-inch I.D. HSA	Very dense, brownish gray (SYR 4/1) fine to medium SAND, little gravel, cobbles, trace silt (OUTWASH)			230.0							
15			SP		215 15	1	SS	40-60/2"	50+	6/7		
20												
30			Hard, medium dark gray (N4) CLAY, trace sand, trace gravel, becomes gravelly SILTY CLAY, little sand in S-3 (TILL)	CL		200.0 30.0	2	SS	7-27-50/3.5	50+	2/15	
40			Boulder encountered @ 40 feet in re-drill attempt.									
45			CL		185.0 45.0	3	SS	50/3"-50/1"	50+	1/4		
50												
60		Hard medium gray (N5), non-stratified SILT to SILTY CLAY, to CLAY (PROGLACIAL LACUSTRINE CLAY)	ML		170 60	4	SS	26-50/4"	50+	9/1		
70												

Continued on page 2

DRILL RIG: B-61

DRILLING CONTRACTOR: RAM, Inc.

DRILLER: Rodney

LOGGED: Scott Hutsell

CHECKED: Jim Johnson

DATE: 11/1/94



PROJECT: Seattle/Lakewood Sewer

RECORD OF BOREHOLE B-5

SHEET 1 OF 1

DATUM: MSL

PROJECT NUMBER: 943-1626

BORING LOCATION: Holyoke Way

BORING DATE: 9/21/94

DEPTH FEET	BORING METHOD	SOIL PROFILE				SAMPLES				PENETRATION RESISTANCE		PIEZOMETER GRAPHIC	
		DESCRIPTION	USCS	GRAPHIC LOG	ELEV.	NUMBER	TYPE	BLOWS / 6 IN. 140 lb. hammer 30 inch drop	N	REMARKS	WATER CONTENT, PERCENT		
					DEPTH						Wp		WI
0	4-inch I.D. HSA	Compact, medium gray (N5), silty, fine to medium SAND, trace to some gravel, wood debris (FILL)			50.0								
2.5		SM		47.5	1	SS	10-10-8	18	18/18				
7.5		-	Sample 2 driven on wood debris, possibly a log		42.5	2	SS	50/6"	50+	2/6			
12.5		SM		37.5	3	SS	9-7-8	15	18/18				
17.5		CL	Very dense, medium gray (N6), silty fine to medium SAND, trace gravel		32.5	4	SS	8-21-36	57	18/18	36%		
22.5		SM	Lense of CLAY, little coarse to fine sand in sample 4.		27.5	5	SS	50/3.5"	50+	12/35			
27.5		SM			22.5	6	SS	50-50/2	50+	2/8			
28.2			Boring terminated @28'2" Completed with piezometer		28.2								

DRILL RIG: B-61
 DRILLING CONTRACTOR: RAM, Inc.
 DRILLER: George Jewel

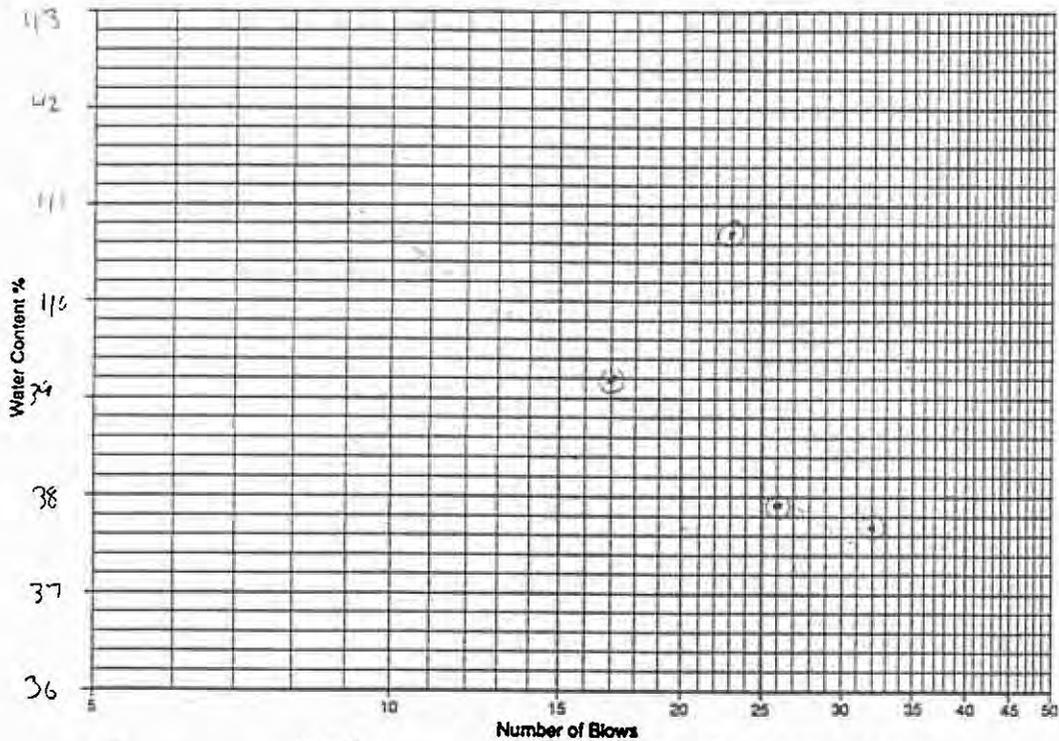
LOGGED: Richard Luark
 CHECKED: Jim Johnson
 DATE: 11/1/94



Type of Test	LL	LL	LL	LL	Nat. MC
Container #	T-71	T-78	T-87	T-90	30
Number of blows	22	23	17	26	-
Weight of sample wet + tare	36.57	38.19	39.49	38.03	50.03
Weight of sample dry + tare	32.43	33.31	34.33	33.31	44.87
Weight of water					
Tare	21.46	21.31	21.18	20.78	25.00
Weight of dry soil					
Water content %	37.7	40.7	39.2	37.7	26.0

Type of Test	PL	PL
Container #	T-119	T-120
Weight of sample wet + tare	36.26	35.96
Weight of sample dry + tare	34.85	34.56
Weight of water		
Tare	28.65	28.96
Weight of dry soil		
Water content %	22.7	25.0

Borehole #	B-4
Sample #	S-5
Depth	75'
Liquid Limit	38
Plastic Limit	24
Plasticity Index	14
Moisture Content	26.0
Liquidity Index	0.14



Sample Description Medium gray (N/S), CLAYEY SILT,
little F gravel, trace F sand, (CL).

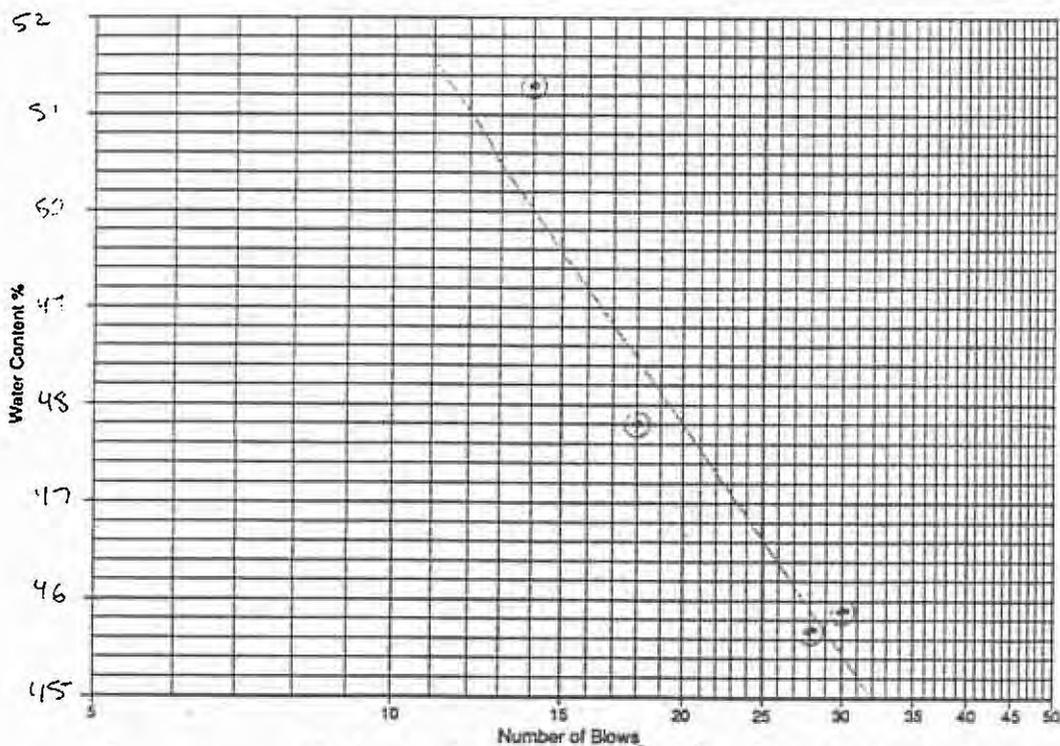
ATTERBERG LIMITS, ASTM D4318

Project Seattle / Lakewood Tunnel / WA
 Project No. 443-1626 Date 11-29-92 Tested By MSB Checked By DPO  Golder Associates

Type of Test	LL	LL	LL	LL	Nat. MC
Container #	T-65	T-88	T-96	T-110	29
Number of blows	70	14	18	28	-
Weight of sample wet + tare	38.82	40.56	38.65	40.18	40.45
Weight of sample dry + tare	33.31	33.83	32.91	34.12	36.83
Weight of water					
Tare	21.31	20.70	20.91	20.86	24.75
Weight of dry soil					
Water content %	45.9	51.3	47.8	45.7	30.0

Type of Test	PL	PL
Container #	T-113	T-117
Weight of sample wet + tare	35.50	36.34
Weight of sample dry + tare	34.29	34.91
Weight of water		
Tare	28.81	28.58
Weight of dry soil		
Water content %	22.1	22.6

Borehole #	B-2
Sample #	S-5
Depth	90'
Liquid Limit	47
Plastic Limit	22
Plasticity Index	25
Moisture Content	30.0
Liquidity Index	0.32

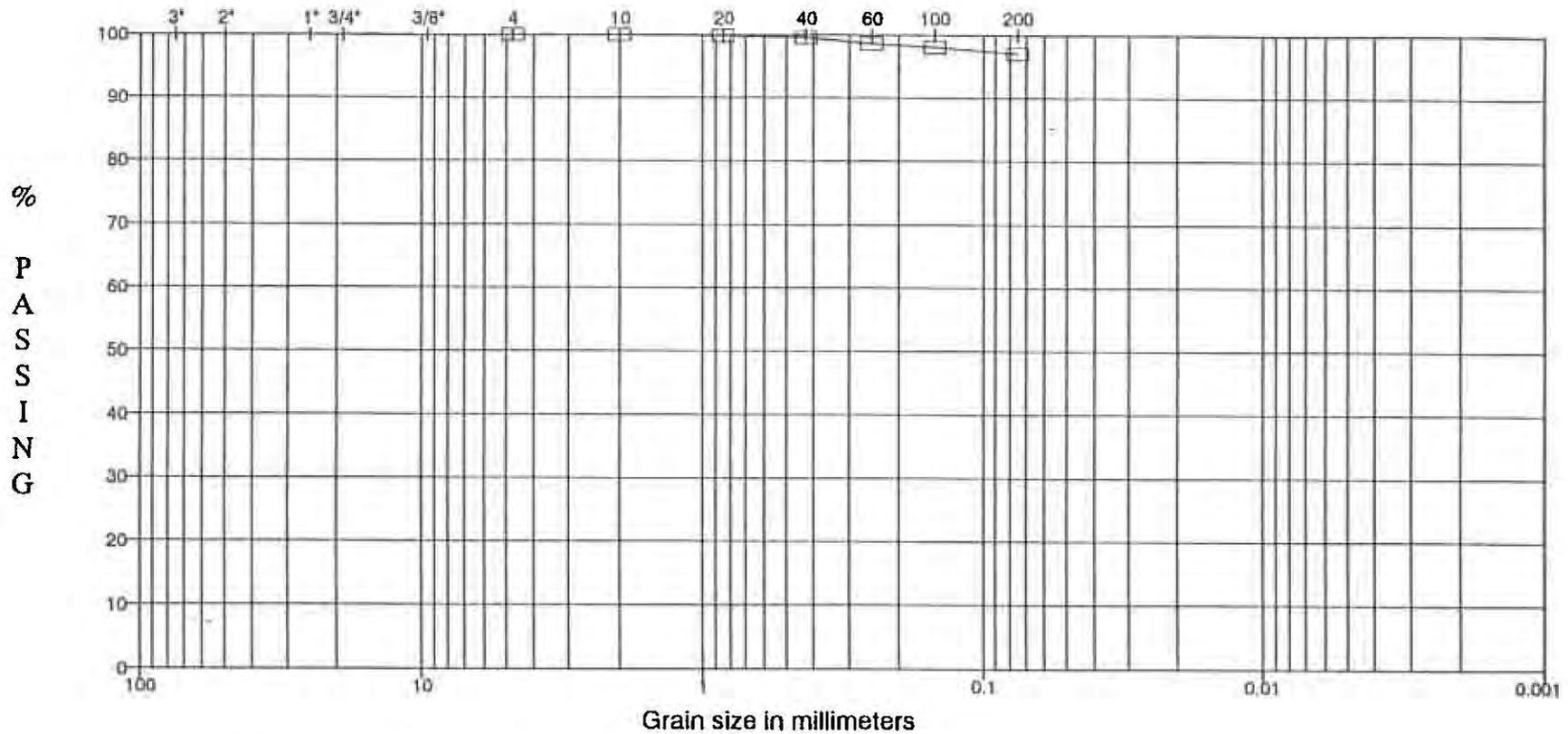


Sample Description Medium dark gray (N4), SILTY CLAY,
some f gravel, little m-f sand, (CL).

ATTERBERG LIMITS, ASTM D4318

Project Seattle / Lakewood Tunnel / WA
 Project No. 943-1626 Date 11-29-94 Tested By MSB Checked By [Signature]  Golder Associates

**PARTICLE SIZE DISTRIBUTION
US STANDARD SIEVE OPENING SIZES**



Coarse	Fine	C	Med	Fine	FINES (Silt or Clay)
GRAVEL		SAND			

SAMPLE ID	DEPTH	W%	LL	PL	PI	USCS	DESCRIPTION
B-1 S-5	77.5-78'	21.9				CL	Medium gray (N5), CLAYEY SILT, trace m-f sand, (CL).

PROJECT: SEATTLE / LAKEWOOD TUNNEL / WA

PROJECT NO.: 943-1626

DATE: 11-30-94

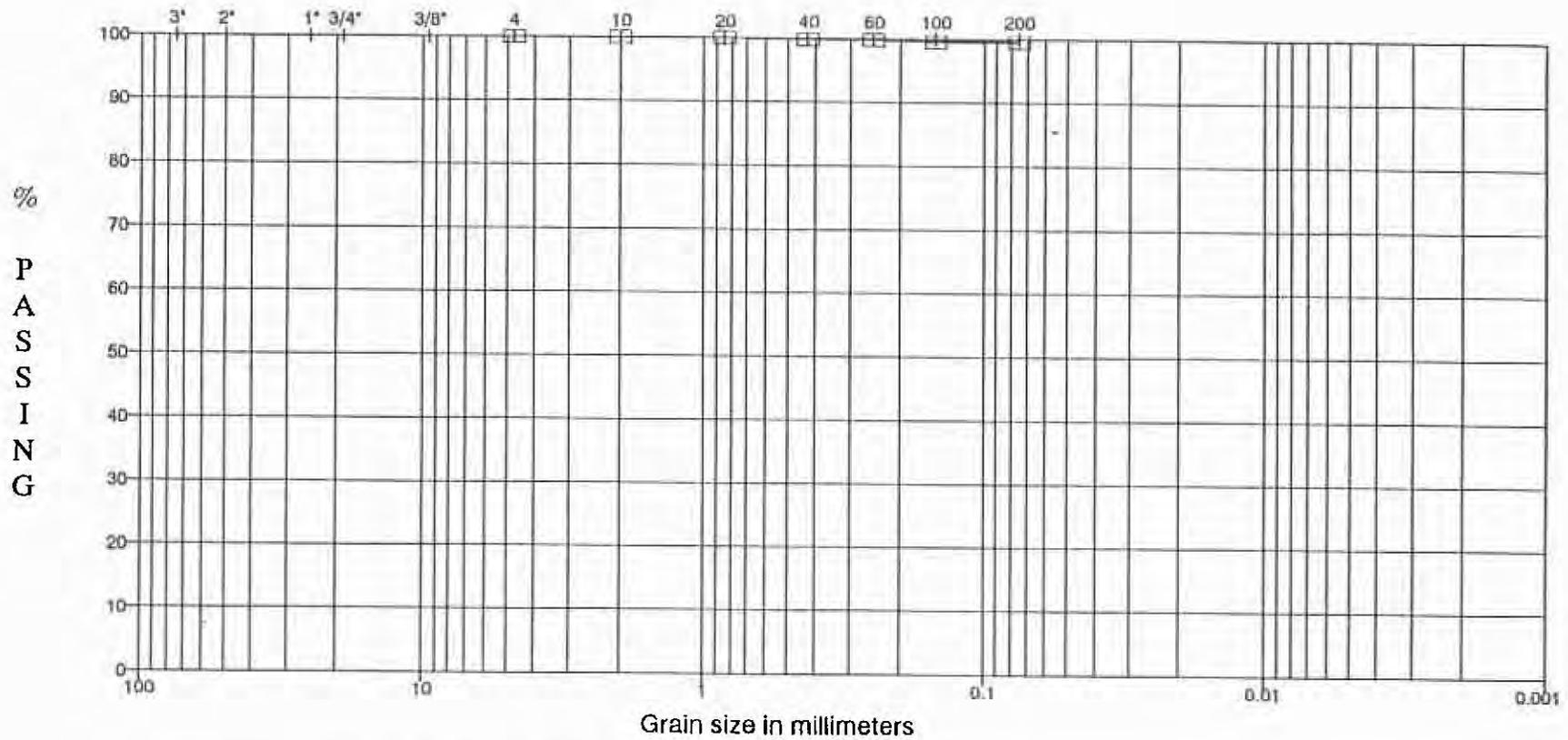
TECH: MSB

REVIEW: DPO

GOLDER ASSOCIATES INC.

REDMOND, WA

**PARTICLE SIZE DISTRIBUTION
US STANDARD SIEVE OPENING SIZES**



Coarse	Fine	C	Med	Fine	FINES (Silt or Clay)
GRAVEL		SAND			

SAMPLE ID	DEPTH	W%	LL	PL	PI	USCS	DESCRIPTION
B-2 S-4	75'	26.6				CL	Medium gray (N5), SILTY CLAY, trace m-f sand, (CL).

PROJECT: SEATTLE / LAKEWOOD TUNNEL / WA

PROJECT NO.: 943-1626

DATE: 11-30-94

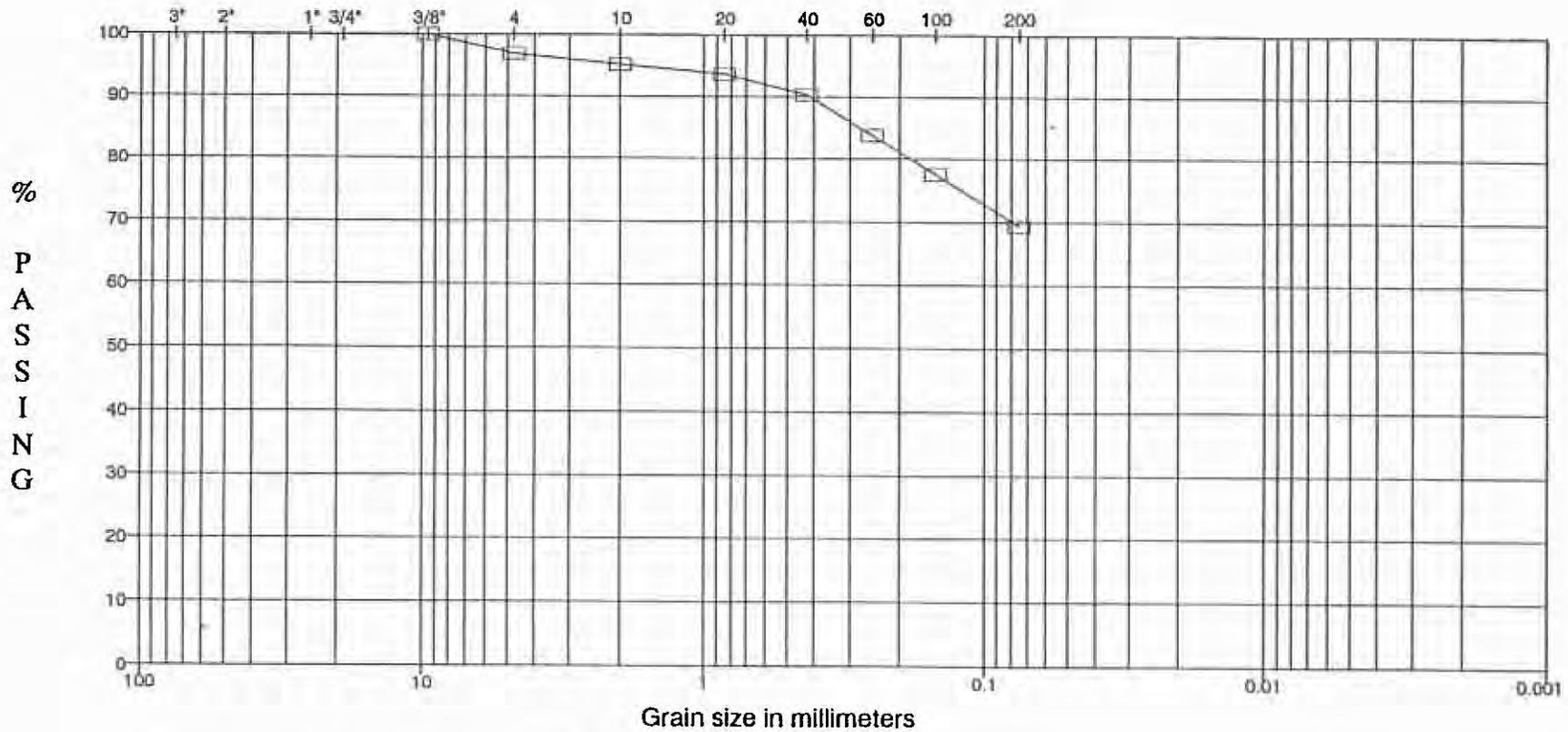
TECH: MSB

REVIEW: DPO

GOLDER ASSOCIATES INC.

REDMOND, WA

**PARTICLE SIZE DISTRIBUTION
US STANDARD SIEVE OPENING SIZES**



Coarse	Fine	C	Med	Fine	FINES (Silt or Clay)
GRAVEL		SAND			

SAMPLE ID	DEPTH	W%	LL	PL	PI	USCS	DESCRIPTION
B-3 S-11-14	155-170'	10.1				CL	Medium dark gray (N4), SILTY CLAY, some m-f sand, trace f gravel, (CL).

PROJECT: SEATTLE / LAKEWOOD TUNNEL / WA

PROJECT NO.: 943-1626

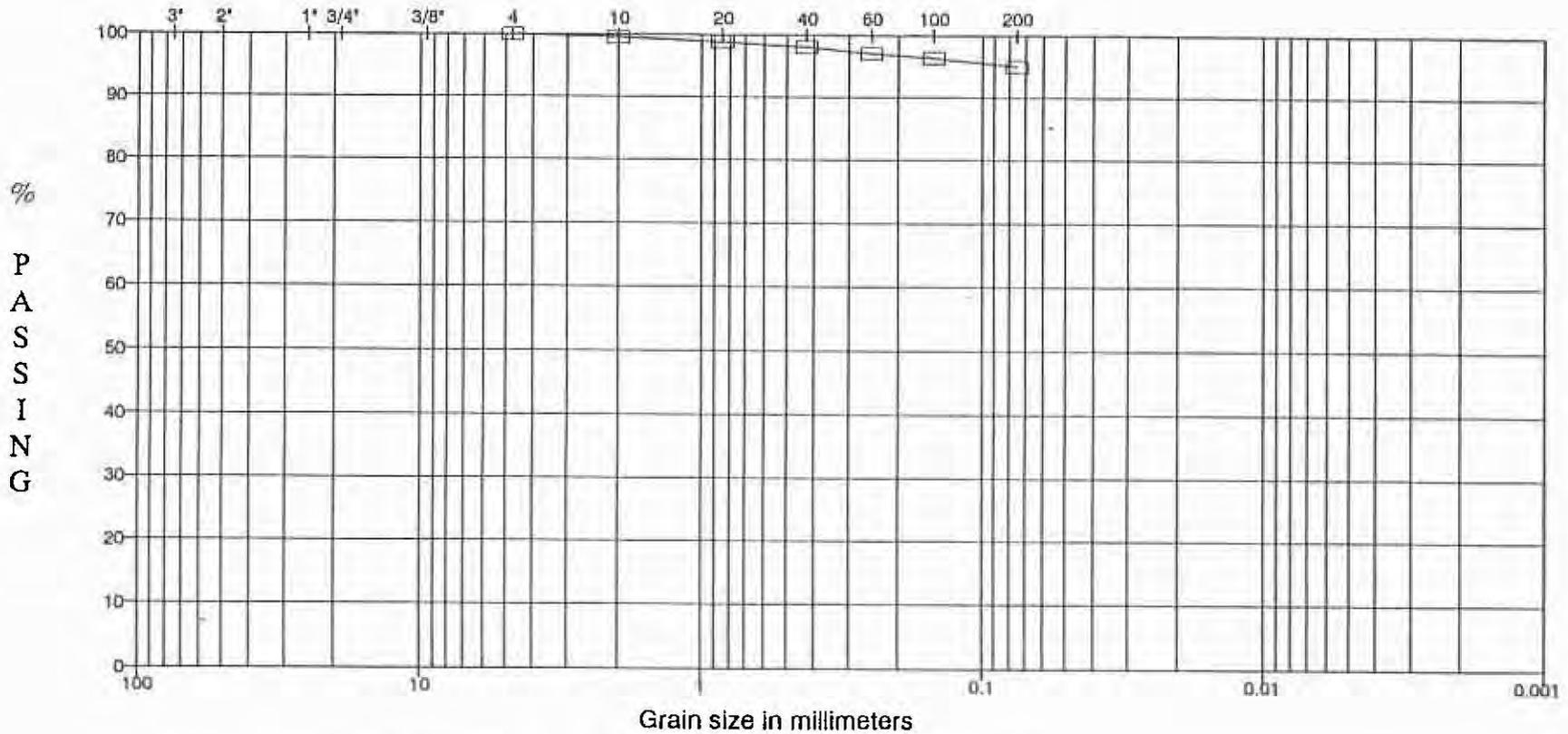
DATE: 11-30-94

TECH: MSB

REVIEW: DPO

GOLDER ASSOCIATES INC.
REDMOND, WA

**PARTICLE SIZE DISTRIBUTION
US STANDARD SIEVE OPENING SIZES**



Coarse	Fine	C	Med	Fine	FINES (Silt or Clay)
GRAVEL		SAND			

SAMPLE ID	DEPTH	W%	LL	PL	PI	USCS	DESCRIPTION
B-5 S-4	17.5-19'	36.2				CH	Medium gray (N5), CLAY, little c-f sand, (CH).

PROJECT: SEATTLE / LAKEWOOD TUNNEL / WA

PROJECT NO.: 943-1626

DATE: 11-30-94

TECH: MSB

REVIEW: DPO

GOLDER ASSOCIATES INC.

REDMOND, WA