

# STRUCTURAL - GENERAL NOTES

## GENERAL REQUIREMENTS

**GOVERNING CODE:** The design and construction new construction for this project is governed by the "Seattle Building Code (SBC)", 2009 Edition, hereafter referred to as the SBC, as adopted by the City of Seattle, WA understood to be the Authority Having Jurisdiction (AHJ). The structural portions of this re-roofing project consist of repair of deteriorated roof deck, installation of new mechanical units and fall restraints, and limited seismic upgrades. The seismic upgrades included in this project are intended to be part of a future, undefined overall seismic upgrade, but are being done now as they are best performed prior to re-roofing. The upgrades would not be required by the work currently being performed, and should be categorized as Voluntary Seismic Upgrades.

**REFERENCE STANDARDS:** Refer to Chapter 35 of 2009 IBC. Where other Standards are noted in the drawings, use the latest edition of the standard unless a specific date is indicated. Reference to a specific section in a code does not relieve the contractor from compliance with the entire standard.

**OTHER DRAWINGS:** Refer to the architectural, mechanical, and electrical, drawings for additional information including but not limited to: dimensions, elevations, slopes, finishes, drains, waterproofing, railings mechanical unit locations, and other nonstructural items.

**COORDINATION:** The Contractor is responsible for coordinating details and accuracy of the work; for confirming and correlating all quantities and dimensions; for selecting fabrication processes; for techniques of assembly; and for performing work in a safe and secure manner.

**MEANS, METHODS and SAFETY REQUIREMENTS:** The contractor is responsible for the means and methods of construction and all job related safety standards such as OSHA and DOSH (Department of Occupational Safety and Health).

**TEMPORARY SHORING, BRACING:** The contractor is responsible for the strength, safety, and stability of the new and existing structure during construction and shall provide temporary shoring, bracing and other elements required to maintain stability until the structure is complete. It is the contractor's responsibility to be familiar with the work required in the construction documents and the requirements for executing it properly. The contractor shall at his discretion employ an SSE, a registered professional engineer for the design of any temporary bracing and shoring.

**CONSTRUCTION LOADS:** Loads on the structure during construction shall not exceed the design loads as noted in DESIGN CRITERIA & LOADS below or the capacity of partially completed construction as determined by the Contractor's SSE for Bracing/Shoring.

**NOTE PRIORITIES:** Plan and detail notes and specific loading data provided on individual plans and detail drawings supplements information in the Structural General Notes.

**DISCREPANCIES:** In case of discrepancies between the General Notes, Specifications Plan/details or Reference Standards, the Architect/Engineer shall determine which shall govern. Discrepancies shall be brought to the attention of the Architect/Engineer before proceeding with the work. Should any discrepancy be found in the Contract Documents, the Contractor will be deemed to have included in the price the most expensive way of completing the work, unless prior to the submission of the price, the Contractor asks for a decision from the Architect as to which shall govern. Accordingly, any conflict in or between the Contract Documents shall not be a basis for adjustment in the Contract Price.

**SITE VERIFICATION:** The contractor shall verify all dimensions and conditions at the site. Conflicts between the drawings and actual site conditions shall be brought to the attention of the Architect/Engineer before proceeding with the work.

**ALTERNATES:** Alternate products of similar strength, nature and form for specified items may be submitted with adequate technical documentation to the Architect/Engineer for review. Alternate materials that are submitted without adequate technical documentation or that significantly deviate from the design intent of materials specified may be returned without review. Alternates that require substantial effort to review will not be reviewed unless authorized by the Owner.

## DESIGN CRITERIA AND LOADS

SEISMIC UPGRADES ARE VOLUNTARY SEISMIC UPGRADES THAT DO NOT CONFORM TO THE 2009 SBC

<b>OCCUPANCY:</b>	Occupancy Category of Building per 2009 IBC Table 1604.5 =	II
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SEISMIC DESIGN:	Seismic Design Category:	SDC =	D
	Basic Structural System		N/A
	Seismic Force Resisting System		Steel Braced Frame and Steel Frame with Masonry Infill
	Site Classification per IBC Table 1613.5.2	Site Class =	D
	Seismic Importance Factor per ASCE 7-05 Table 11.5-1	I <sub>e</sub> =	1.0
	Spectral Response Acceleration (Short Period)	S <sub>s</sub> =	1.243 g
	Spectral Response Acceleration (1-Second Period)	S <sub>1</sub> =	0.425 g
	Spectral Design Response Coefficient (Short Period)	S <sub>DS</sub> =	0.831 g
	Spectral Design Response Coefficient (1-Second Period)	S <sub>D1</sub> =	0.446 g
	Base shear governed by:		seismic
	Base shear governed by:		seismic

SNOW LOAD (1)	Flat Roof Snow Load, (PSF)	p <sub>s</sub> =	25 (2)
	Snow Drift Loading required by Authority Having Jurisdiction?		No
	Snow Load Importance Factor	I <sub>s</sub> =	1.0 (3)

- Snow Load is un-reducible and includes 5 psf rain-on-snow surcharge where ground snow load is greater than zero and 20 psf or less per ASCE 7-05 Chap 7.
- Snow Load based on WABO/SEAW White Paper in Low-Lying Puget Sound Basin, ASCE Fig 7-1.
- Snow Load Importance Factor per ASCE 7-05 Table 7-4.

## SUBMITTALS

**SUBMIT FOR REVIEW:** SUBMITTALS of shop drawings, product data are required for items noted in the individual materials sections and for bidder designed elements.

**SUBMITTAL REVIEW PERIOD:** Submittals shall be made in time to provide a minimum of TWO WEEKS for review by the Architect/Engineer prior to the onset of fabrication.

**GENERAL CONTRACTOR'S PRIOR REVIEW:** Prior to submission to the Architect/Engineer, the Contractor shall review the submittal for completeness. Dimensions and quantities are not reviewed by the SER, and therefore, must be verified by the General Contractor. Contractor shall provide any necessary dimensional details requested by the Detailer and provide the Contractor's review stamp and signature before forwarding to the Architect/Engineer.

**SHOP DRAWING REVIEW:** Once the contractor has completed his review, the SER will review the submittal for general conformance with the design concept and the contract documents of the building and will stamp the submittal accordingly. Markings or comments shall not be construed as relieving the contractor from compliance with the project plans and specifications, nor departures there from.

**SHOP DRAWING DEVIATIONS:** When shop drawings (component design drawings) differ from or add to the requirements of the structural drawings they shall be designed and stamped by the responsible SSE.

## TESTS AND INSPECTIONS

**INSPECTIONS:** Special Inspections and Testing shall be done in accordance the STATEMENT OF SPECIAL INSPECTIONS per IBC Sections 1704, 1705, as applicable.

**SPECIAL INSPECTORS:** Special Inspectors shall be employed by the Owner to provide Special Inspections for the project. Special Inspectors shall be qualified persons who are WABO approved and demonstrate competence to the satisfaction of the Authority Having Jurisdiction per 1704.1.

**STATEMENT OF SPECIAL INSPECTIONS per 1704 and 1705:** Special Inspections and Testing are required by 1704, 1706, 1707 and 1708 for the following:

**STRUCTURAL STEEL per IBC Section 1704.3 and Table 1704.3 at the site,** shall be done in accordance with the following requirements:

- Periodic inspection required:
  - During welding of Single-pass Fillet Welds NOT exceeding 5/16" size as noted in IBC Table 1704.3.

**WOOD CONSTRUCTION per IBC Section 1704.6 & 1707.3:**

- Periodic inspection required for verification of:
  - Diaphragms: blocking, strap connections, boundary edge and panel shear nailing size & spacing

**POST-INSTALLED ANCHORS TO CONCRETE AND MASONRY:** shall comply with IBC Section 1703. Inspections shall be in accordance with the requirements set forth in the approved ICC Evaluation Report and as indicated by the design requirements specified on the drawings. Refer to the POST INSTALLED ANCHORS section of these notes for anchors that are the basis of the design. Special inspector shall verify anchors are as specified in the POST INSTALLED ANCHORS section of these notes or as otherwise specified on the drawings. Substitutions require approval by the SER and require substantiating calculations and current 2009 IBC recognized ICC Evaluation Services (ES) Report. Special Inspector shall document in their Special Inspection Report compliance with each of the elements required within the applicable ICC Evaluation Services (ES) Report.

**INSPECTION SUBMITTALS:** Special inspection reports shall be provided on a weekly basis. Final special inspection reports will be required by each special inspection firm per IBC 1704.1.2. Submit copies of all inspection reports to the Architect/Engineer and the Authority Having Jurisdiction for review.

**CONTRACTOR RESPONSIBILITY:** Prior to issuance of the building permit, the Contractor is required to provide the Authority Having Jurisdiction a signed, written acknowledgement of the Contractor's responsibilities associated with the above Statement of Special Inspections addressing the requirements listed in IBC Section 1709. Contractor is referred to IBC Chapter 1707.6 and 1707.7 for architectural and MEP building systems that may be subject to additional inspections (based on the building's designated Seismic Design Category listed in the CRITERIA), including anchorage of HVAC ductwork containing hazardous materials, piping systems and mechanical units containing flammable, combustible or highly toxic materials, electrical equipment used for emergency or standby power, exterior wall panels and suspended ceiling systems.

## POST-INSTALLED ANCHORS (INTO CONCRETE AND MASONRY)

**POST-INSTALLED ANCHORS:** Install only where specifically shown in the details or allowed by SER. All post-installed anchors types and locations shall be approved by the SER and shall have a current ICC-Evaluation Service Report that provides relevant design values necessary to validate the available strength exceeds the required strength. Submit current manufacturer's data and ICC ESR report to SER for approval regardless of whether or not it is a pre-approved anchor. Anchors shall be installed in strict accordance to ICC-ESR and manufacturers instructions. No reinforcing bars shall be damaged during installation of post-installed anchors. Special inspection shall be per the TESTS and INSPECTIONS section. Anchor type, diameter and embedment shall be as indicated on drawings.

- ADHESIVE ANCHORS:** The following Adhesive-type anchoring systems have been used in the design and shall be used for anchorage to MASONRY, as applicable and in accordance with corresponding current ICC ESR report. Drilled-in anchor embedment lengths shall be as shown on drawings, or not less than 7 times the anchor nominal diameter (7D).
  - HILTI "HIT-HY 70" - ICC ESR-2322 for anchorage to MASONRY with mesh web

## STRUCTURAL STEEL

### DESIGN STANDARDS:

Structural Steel for this project is designed in accordance with American Institute of Steel Construction (AISC) Specifications.

Structural Steel for this project is designed per:

- AISC - Manual of Steel Construction, Thirteenth Edition (2005).

### REFERENCE STANDARDS:

- IBC 2009, Chapter 22 - Steel, hereafter referenced as IBC.
- ANSI/AISC 360-05 - Specification for Structural Steel Buildings, hereafter referenced as AISC 360.
- AWS D1.1-04 - Structural Welding Code - Steel, hereafter referenced as AWS D1.1.

### SUBMITTALS:

- Shop drawings shall be prepared in accordance with AISC 360 Section M.1 and AISC 303 Section 4.
- Submit welder's certificates verifying qualification within past 120 months.

### MATERIALS:

Channel (C) & Angle (L) Shapes ..... ASTM A36, Fy = 36 ksi  
 Structural Bars & Plates (PL) ..... ASTM A36, Fy = 36 ksi  
 Mild Threaded Rods ..... ASTM A36, Fy = 36 ksi  
 Threaded Rods (Anchor Bolts) ..... ASTM A307, Fy = 35 ksi

Welding Electrodes ..... E70XX, E71TXX unless noted otherwise with a minimum toughness of 20 ft-lbs at 40 degrees Fahrenheit.

### WELDING:

- Welding shall conform to AWS D1.1 and visually conform to AWS Section 6 and Table 6.1. Fabrication/erection inspections by the Contractor per AWS D1.1 Section 6, shall be by associate/certified inspectors (AWI/CWI) per AWS QC1 or AWS B5.1. Special Inspections (verification inspections) shall be by a certified Welding Inspector (WI) or Senior Welding Inspector (SWI) per AWS B5.1.
- Welders shall be qualified for the specific prequalified joints required by the design and certified in accordance with WABO, requirements.
- Welding shall be done in accordance with appropriate Weld Procedure Specifications (WPS's). Welders shall be familiar with the applicable WPS's.
- Welding shall be done with AWS Prequalified Welding Processes unless otherwise approved.
- Welder qualifications and WPS's shall be maintained at the site of the work and shall be readily available for inspection upon request, both in the shop and in the field.
- Use E70 or E71T, 70 ksi strength electrodes appropriate for the process selected.

### FABRICATION:

- Conform to AISC 303, Section 8 and AISC 360 Section M2 and M5.
- Structural Welding and qualifications shall conform to the AWS D1.1.

### VERIFICATION INSPECTION:

- Structural Welding inspections and qualifications shall conform to the AWS D1.1. See WELDING notes and SPECIAL INSPECTIONS for Structural Steel.
- Special Inspector shall review the procedures for completeness and adequacy relative the Code and the Work. Further shop Special Inspections may be waived if the Fabricator is "AISC Certified" or otherwise "Approved" by the Authority Having Jurisdiction per IBC Section 1704.2.2. See SPECIAL INSPECTIONS for Structural Steel.

### ERECTION:

- Conform to AISC 303, Section 7 "Erection", Section 8 "Quality Assurance," and AISC 360, Section M4.
- The Erector shall maintain detailed fabrication & erection quality control procedures that ensure that the work is performed in accordance with AISC 360 Section M1, AISC 303, and the Contract Documents.
- Steel work shall be carried up true and plumb within the limits defined in AISC 303 Section 7.13.
- Structural Welding to conform to the AWS D1.1 and applicable WELDING notes above.
- Special Inspector shall inspect the steel framing to verify compliance with the details shown on the Contract Documents including member size, location, bracing and the application of proper joint details at each connection.

**BRACING and SAFETY PROTECTION:** The contractor shall provide temporary bracing and safety protection required by AISC 360 Section M4.2 and AISC 303 Section 7.10 and 7.11.

### PROTECTIVE COATING REQUIREMENTS:

- SHOP PAINTING: Conform to AISC 360 Section M3 and AISC 303 Section 6.5 unless a multi-coat system is required per the project specifications.
  - Unless noted otherwise, **do not paint** steel surfaces to be,
    - Welded; if area requires painting, do not paint until after weld inspections and Non-destructive testing requirement, if any, are satisfied.
  - Interior steel, exposed to view, shall be painted with one coat of shop primer unless otherwise indicated in the project specifications. Field touch-ups to match the finish coat or as otherwise indicated in the project specifications

## WOOD FRAMING

### REFERENCE STANDARDS:

- Conform to:
- IBC Chapter 23 "WOOD"
  - NDS - "2005 National Design Specification (NDS) for Wood Construction"
  - APA PDS-97 Plywood Design Specification (revised 1998)

**IDENTIFICATION:** All sawn lumber and pre-manufactured wood products shall be identified by the grade mark or a certificate of inspection issued by the certifying agency.

### MATERIALS:

- Sawn Lumber: Conform to grading rules of WMPA, WCLIB or NLGA and Table below. Finger jointed studs acceptable at interior walls only.

### TABLE OF SOLID SAWN LUMBER

Member Use	Size	Species	Grade
Floor or Roof Joist	2x6 through 2x12	Doug Fir Larch	No. 2
Beam	4x8 through 4x12	Doug Fir Larch	No. 2
Beam	6x8 through 6x12	Doug Fir Larch	No. 1

### TABLE OF T&G DECKING

Member Use	Size	Species	Grade
T&G Decking	2x6 Solid	Doug-Fir Larch	Commercial

**Wood Structural Sheathing (Plywood):** Wood APA-rated structural sheathing includes: all veneer plywood. Architect will disallow OSB. Conform to "Construction and Industrial Plywood" based on Product Standard PS 1-07 by the U.S. Dept. of Commerce, and "Performance Standard for Wood-Based Structural-Use Panels" based on Product Standard PS 2-04 by the U.S. Dept. of Commerce and "Plywood Design Specification (revised 1998)" based on APA PDS-97 by the American Plywood Association. Unless noted otherwise, sheathing shall comply with the following table:

### TABLE OF SHEATHING - Use, Minimum Thickness and Minimum APA Rating

Location	Thickness	Span Rating	Plywood Grade	Exposure
Roof	15/32"	32/16	C-D	1

- Unless noted otherwise on drawings, install roof and floor panels with long dimension across supports and with panel continuous over two or more spans. End joints shall occur over supports. [HUD UM406 grade marking of sheathing certification is required.]

**Timber Connectors:** Shall be "Strong Tie" by Simpson Company as specified in their latest catalog. Alternate connectors by other manufacturers may be substituted provided they have current ICC approval for equivalent or greater load capacities and are reviewed and approved by the SER prior to ordering. Connectors shall be installed per the manufacturer's instructions. Where connector straps connect two members, place one-half of the nails or bolts in each member. Where straps are used as hold-downs, nail straps to wood framing just prior to drywall application, as late as possible in the framing process to allow the wood to shrink and the building to settle. Premature nailing of the strap may lead to strap buckling and potential finish damage.

Where connectors are in exposed exterior applications in contact with preservative treated wood (PT) other than CCA, connectors shall be either batch hot-dipped galvanized (HDG), mechanically galvanized (ASTM B695, Class 40 or greater) stainless steel, or provided with 1.85 oz/sf of zinc galvanizing equal to or better than Simpson ZMAX finish.

**Fasteners** (nails, bolts, screws, etc) attaching timber connectors (joist hangers, post caps and bases, etc) to PT wood shall have similar corrosion resistance properties (matching protective treatments) as the protected connector. Fasteners (nails, bolts, screws, etc) attaching sawn timber members or sheathing (shear walls) to PT wood be corrosion resistant; nails and lag bolts shall be either HDG (ASTM A153) or stainless steel. Verify the suitability of the fastener protection/coating with the wood treatment chemical manufacturer/supplier.

Provide washers under the heads and nuts of all bolts and lag screws bearing on wood. All nails 12d and smaller shall be full length common unless noted otherwise. 16d nails may be 16d sinkers unless noted otherwise. Nail straps to wood framing as late as possible in the framing process to allow the wood to shrink and the building to settle. Premature nailing of the strap may lead to strap buckling and potential finish damage.

**Fasteners:** Conform to IBC Section 2304.9 "Connections and fasteners." Unless noted on plans, nail per Table 2304.9.1. Unless noted otherwise all nails shall be common. Alternate nails may be used but are subject to review and approval by the Structural Engineer. Substitution of staples for the nailing of rated sheathing is subject to review by the structural engineer prior to construction.

**NAILING REQUIREMENTS:** Provide minimum nailing in accordance with IBC Table 2304.9.1. "Fastening Schedule" except as noted on the drawings. Nailing for roof/door diaphragms/shear walls shall be per drawings. Nails shall be driven flush and shall not fracture the surface of sheathing.

**STANDARD LIGHT-FRAME CONSTRUCTION:** Unless noted on the plans, construction shall conform to IBC Section 2308 "Conventional Light-Frame Construction."

**PRESERVATIVE TREATMENT (PT):** Wood materials are required to be "treated wood" in accordance with IBC Section 2304.11. "Decay and Termite Protection" shall conform to the appropriate standards of the American Wood-Preservers Association (AWPA) for sawn lumber, glued laminated timber, round poles, wood piles and marine piles. Follow American Lumber Standards Committee (ALSC) quality assurance procedures. Products shall bear the appropriate mark.

If using sill plates other than CCA or sodium borate, fasteners must be hot dipped galvanized or stainless steel. Fasteners (nails, bolts, screws, washers & lag screws) attaching timber connectors (joist hangers, post caps and bases, etc) to PT wood shall have similar corrosion resistance properties (matching protective treatments) as the protected connector; that is, use hot dipped galvanized or stainless steel fasteners. Fasteners (nails, bolts, screws, washers & lag screws) attaching sawn timber members or sheathing (shear walls) to Pressure Treated wood be corrosion resistant (not dipped galvanized or stainless steel).

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NOT LESS THAN 2 OR MORE THAN 10 BUSINESS DAYS PRIOR TO COMMENCING EXCAVATION OR DEMOLITION, SECURE THE SERVICES OF A COMMERCIAL UNDERGROUND UTILITIES LOCATOR SERVICE TO IDENTIFY BELOW-GROUND IMPROVEMENTS THAT MAY NOT BE INDICATED ON THE DRAWINGS. ADDITIONALLY, TO REQUEST ON-SITE UNDERGROUND UTILITY SYSTEMS LOCATES, CALL SPR INSPECTION REQUEST LINE @ (206) 884-7834. REFER TO SECTIONS 02100 & 02120 OF THE SPECIFICATIONS FOR INSTRUCTIONS FOR HOW TO PROCEED.

5			
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3			
2	PERMIT	07-29-13	
1	65% SUBMITTAL	08-20-13	
NO.	REVISION - AS BUILT	DATE	

REVIEWED BY: PARK ENGINEER DATE

All work shall be done in accordance with the City of Seattle Standard Plans and Specifications in effect on the date shown above, and supplemented by the Special Provisions.



**WAGNER ARCHITECTURE**  
 PLANNING  
 1916 PIKE PLACE  
 SEATTLE WA 98101  
 2 0 6 - 4 4 8 - 2 5 2 8  
 F A X - 4 4 1 - 6 1 8 4  
 wagnerarchitects.com



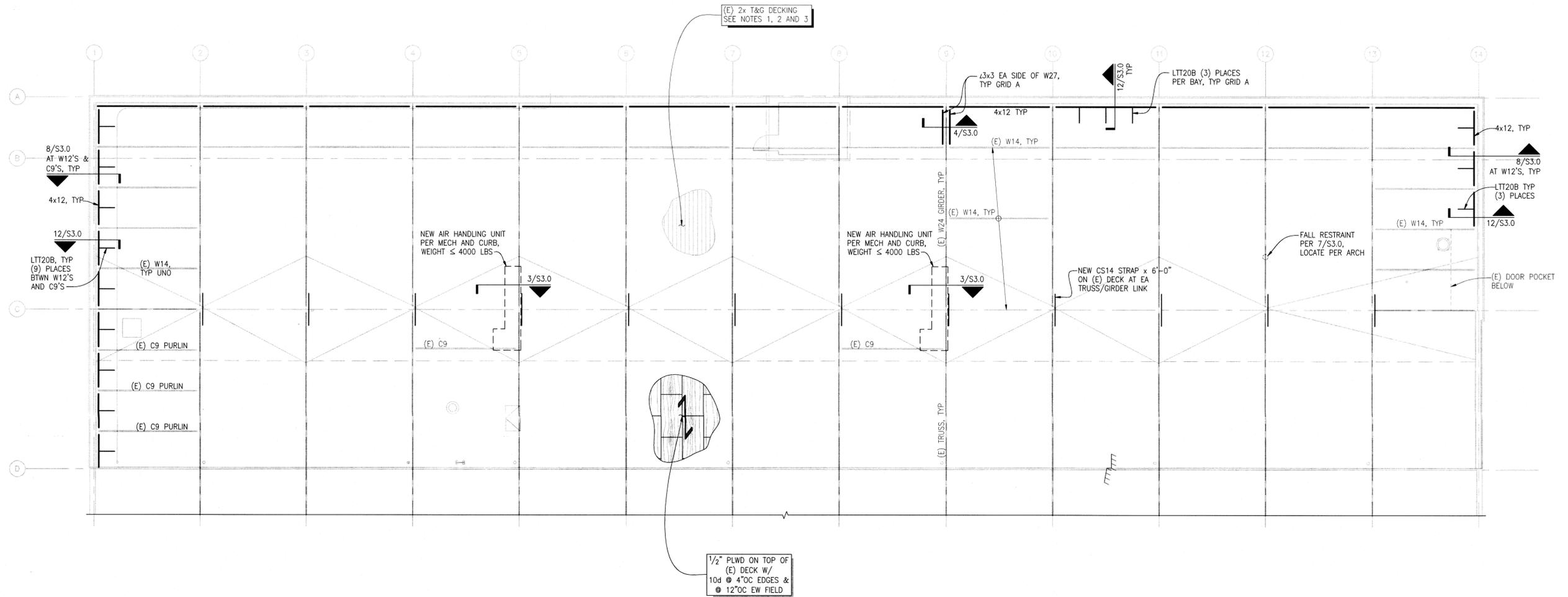
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**MAGNUSON BLDG #2**  
**PARTIAL ROOF & SEISMIC REPAIRS**  
**GENERAL NOTES**

DESIGNED	DH	DATE	07/30/2013
DRAWN	GR	SHEET	14 OF 26
CHECKED	DH		
ORDINANCE NO.	124058		S1.0
CONTRACT NO.	1862		
SCALE	NONE		

**ROOF FRAMING PLAN NOTES:**

1. DETERIORATED EXISTING DECKING SHALL BE REMOVED AND REPLACED IN KIND BY REMOVING BOTTOM OF GROOVE. DECKING TO BE REPLACED FULL LENGTH BETWEEN SUPPORTS AT WIDE FLANGE BEAMS. TOE NAIL ADJACENT DECKING WITH 10d NAILS AT 24"OC.
2. EXTENT OF DETERIORATED DECKING SHALL BE DETERMINED BY CONSULTING STRUCTURAL ENGINEER. CONTRACTOR SHALL NOTIFY ARCHITECT OF ADDITIONAL AREAS OF DETERIORATED DECKING THAT MAY BE INDICATED DURING CONSTRUCTION FOR DETERMINATION BY OWNER'S REPRESENTATIVE. REFERENCE PROJECT SPECIFICATIONS FOR ASSUMED PERCENTAGE OF DECK REQUIRING REPLACEMENT FOR BID PURPOSES.
3. TYPE OF REPLACEMENT TO BE AT DIRECTION OF CONSULTING STRUCTURAL ENGINEER.
4. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 AND S1.2.
5. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECT'S DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
6. ALL DUCTS, CHASES AND PIPES SHALL BE PER MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS. STAIR DETAILS AND GUARDRAILS PER ARCHITECTURAL DRAWINGS.
7. ROOF SHEATHING PER PLAN AND STRUCTURAL GENERAL NOTES. SHEATHING SHALL BE NAILED TO ROOF FRAMING WITH 0.131"x2 1/2" NAILS @ 4"OC AT PANEL EDGES AND @ 12"OC FIELD. UNO. LAY SHEATHING WITH FACE GRAIN (LONG DIRECTION) PERPENDICULAR TO SUPPORTS AND STAGGER PANEL END JOINTS. ALLOW 1/8" SPACE BETWEEN PANEL ENDS AND EDGES.



**ROOF PLAN**  
SCALE: 3/32" = 1'-0"



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NOT LESS THAN 2 OR MORE THAN 10 BUSINESS DAYS PRIOR TO COMMENCING EXCAVATION OR DEMOLITION, SECURE THE SERVICES OF A COMMERCIAL UNDERGROUND UTILITIES LOCATOR SERVICE TO IDENTIFY BELOW-GROUND IMPROVEMENTS THAT MAY NOT BE INDICATED ON THE DRAWINGS. ADDITIONALLY, TO REQUEST ON-SITE UNDERGROUND UTILITY SYSTEMS LOCATES, CALL **SPR INSPECTION REQUEST LINE @ (206) 884-7034**. REFER TO SECTIONS 02100 & 02120 OF THE SPECIFICATIONS FOR INSTRUCTIONS FOR HOW TO PROCEED.

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2	PERMIT	07-29-13
1	65% SUBMITTAL	06-20-13
NO.	REVISION - AS BUILT	DATE

REVIEWED BY: PARK ENGINEER DATE \_\_\_\_\_  
All work shall be done in accordance with the City of Seattle Standard Plans and Specifications in effect on the date shown above, and supplemented by the Special Provisions.

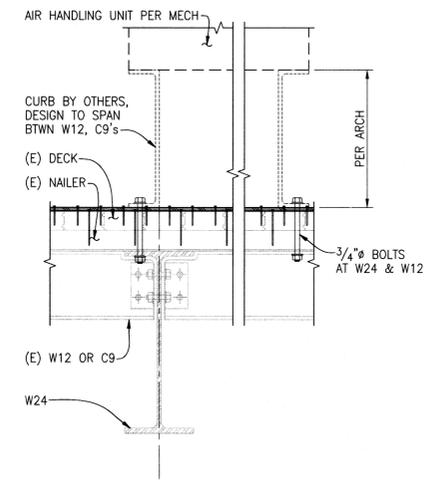


**WAGNER ARCHITECTURE**  
PLANNING  
1916 PIKE PLACE  
SEATTLE WA 98101  
206-448-2528  
FAX 441-6184  
wagnerarchitects.com



**MAGNUSON BLDG #2**  
**PARTIAL ROOF & SEISMIC REPAIRS**  
**ROOF PLAN**

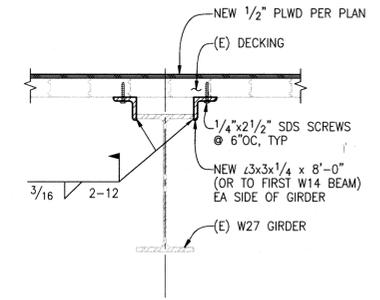
DESIGNED	DH	DATE	07/30/2013
DRAWN	GR	SHEET	15 OF 26
CHECKED	DH		
ORDINANCE NO.	124058		S2.0
CONTRACT NO.	1862		
SCALE	3/32" = 1'-0"		



SECTION 3

SCALE: 1"=1'-0"

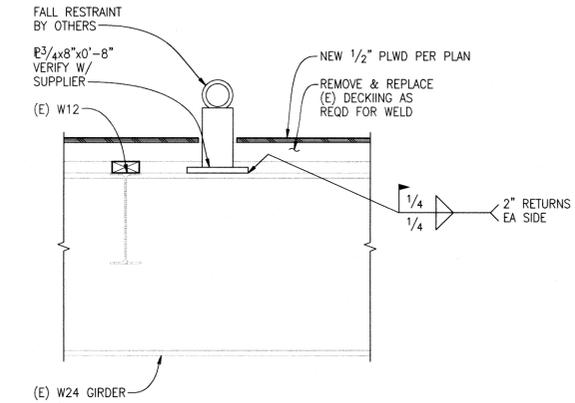
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SECTION 4

SCALE: 1"=1'-0"

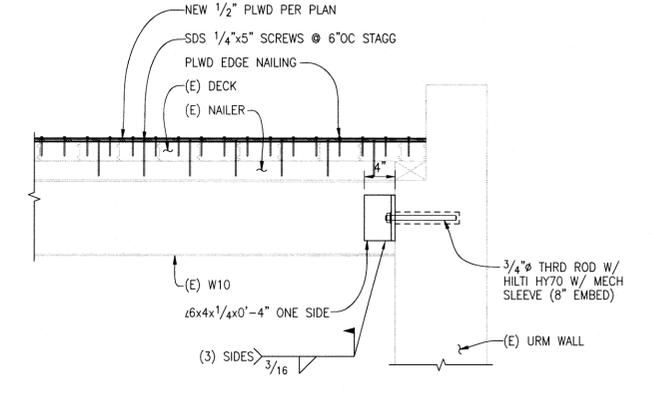
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FALL RESTRAINT (OPTION)

SCALE: 1"=1'-0"

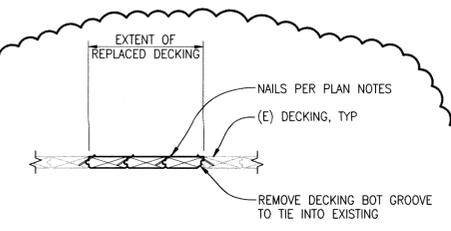
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SECTION 8

SCALE: 1"=1'-0"

8

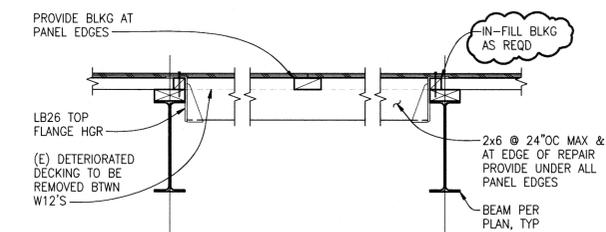


NOTE:  
REPLACE DECKING WITH TYPE ONE OR TYPE TWO AT THE DIRECTION OF THE CONSULTING STRUCTURAL ENGINEER.

TYPE ONE DECKING REPAIR AND REPLACEMENT

SCALE: 1"=1'-0"

10

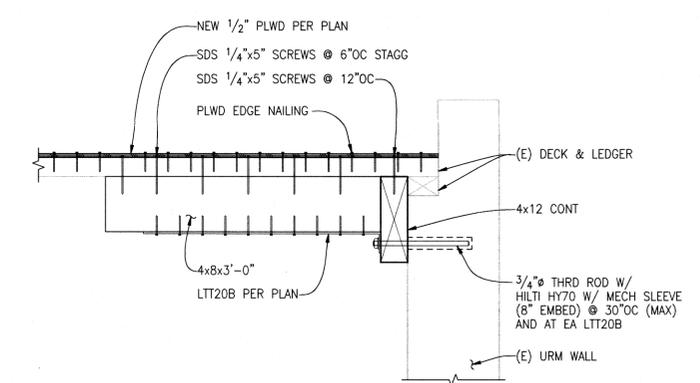


NOTE:  
REPLACE DECKING WITH TYPE ONE OR TYPE TWO AT THE DIRECTION OF THE CONSULTING STRUCTURAL ENGINEER.

TYPE TWO DECKING REPLACEMENT FOR 2'-0" OR WIDER AREAS OF DECKING

SCALE: 1"=1'-0"

11



SECTION 12

SCALE: 1"=1'-0"

12

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**MAGNUSON BLDG #2**  
**PARTIAL ROOF & SEISMIC REPAIRS**  
**STRUCTURAL DETAILS**

DESIGNED	DH	DATE	07/30/2013
DRAWN	GR	SHEET	16 OF 26
CHECKED	DH		
ORDINANCE NO.	124058		S3.0
CONTRACT NO.	1862		
SCALE	1" = 1'-0"		